DRAINAGE REPORT

ALLEN & MAJOR
ASSOCIATES, INC.

200-400 Quannapowitt Parkway
Wakefield, MA

APPLICANT:
Cabot, Cabot & Forbes
185 Dartmouth Street
Boston, MA 02116

PREPARED BY:
Allen & Major Associates, Inc.
100 Commerce Way, Suite 5
Woburn, Massachusetts 01801

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ISSUED:
March 16, 2021

REVISED:
March 31, 2021

A&M PROJECT NO.:
1623-11

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SECTION 1.0 -
DRAINAGE REPORT

Introduction

The purpose of this drainage report is to provide an overview of the proposed stormwater management system for the proposed development of 200-400 Quannapowitt Parkway, Wakefield, MA, (the site). The report will show by means of narrative, calculations and exhibits that there is no increase in the peak rate of runoff from the Site at all of the study points for each of the required (2-, 10- and 100-year) design storm events.

The proposed mixed-use redevelopment includes razing the existing building, and constructing three detached multi-story buildings with approximately 485 multifamily units and retail space. Other improvements to the site include renovation and reconstruction of all surface parking, landscaping, and underground utilities servicing the redevelopment. The stormwater management system (SMS) will be enhanced to maximize treatment and infiltration of stormwater on site.

The SMS incorporates structural and non-structural Best Management Practices (BMPs) to provide stormwater quality treatment and conveyance. The SMS includes deep-sump hooded catch basins, drain manholes, underground piping, underground infiltration chambers, bioretention cells, surface infiltration basins vegetated filter strip with stone diaphragm, roof drains, and an Operation & Maintenance Plan.

Site Categorization for Stormwater Regulations

According to the Massachusetts Stormwater Handbook the proposed site improvements at 200 Quannapowitt Parkway are considered "redevelopment" due to the reduction in impervious area. Under proposed conditions, the project will increase the amount of green (pervious) surfaces by approximately 41,000 s.f. (0.95 acres). A "redevelopment" project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions. See the discussion of Stormwater Management Standards that follows.

There is work proposed within MA DEP jurisdictional areas. Therefore, approval under the Massachusetts Wetlands Protection Act is required and a Notice of Intent (NOI) filing has been prepared and submitted as part of this project.

Site Location and Access

The site is located entirely within the Town of Wakefield, Massachusetts and is situated between Interstate-95 to the west and Lake Quannapowitt to the east. Access to

Quannapowitt Parkway is provided off Lowell Street to the north and North Ave to the south.

Existing Site Conditions

The majority of the existing building is currently vacant and previously used as office space with a small percentage dedicated to a data center. The Site is generally flat, varying 0-5 feet in elevation, with a low point around the perimeter of the property and a high point at the center in the general location of the existing building. The Site is surrounded by wetlands as shown on the accompanying plans. These wetlands are fed primarily by surface runoff from the existing roof and parking lot. The majority of this runoff flows unmitigated with no treatment prior to discharge at the resource areas. These wetlands are also connected to Lake Quannapowitt by two underground culverts located at the southern and eastern corners of the property.

All runoff from the Site was analyzed at three study points. Study Point #1 is a summation of all runoff towards the existing culvert to the south connecting to the lake. Study Point #2 is a summation of all site runoff from the site into the lake. Study Point #3 is a summation of all runoff towards the existing culvert to the east connecting to the lake. All Site runoff eventually discharges to Lake Quannapowitt through the two culverts described above, direct entry from roof leaders, or overland flow from the grassed area bordering the lake. Copies of the Watershed plans (Existing and Proposed), providing the boundaries and contributing areas are provided in the back pocket of this report.

Existing Soil Conditions

The on-site soils were identified using the USDA Natural Resources Conservation Services (NRCS) Soil Survey for Middlesex County. The site is primarily Soil Type 656 – Udorthents-Urban Land Complex. Urban Land consists of areas where the soil has been altered or obscured by buildings or paved areas. These structures cover 75 percent or more of the surface area. A copy of the soil mapping is included in the Appendix of this report.

Haley & Aldrich performed some preliminary subsurface explorations on the property and described their findings in a Memorandum titled "Due Diligence Geotechnical Investigation". It's estimated that a 2-6' surficial layer of fill exists across the site. The fill consists primarily of reworked natural granular soils placed during previous site development. The memorandum went on to describe the groundwater conditions as follows:

"Groundwater observation wells were not installed as part of this investigation. Previous explorations on adjacent parcels, as well as water levels in the adjacent Lake Quannapowitt, suggest that groundwater is likely present about 3 to 5 feet below

existing site grades. This is consistent with where groundwater levels were interpreted in the recent CPT explorations. The Lake is dam controlled and reported to be maintained at approximately El. 79."

A&M has taken this information into consideration during the design but additional test pits will need to be performed to confirm actual site conditions at the proposed stormwater management locations. For purposes of this analysis and because of the poor drainage of the existing soils on site, all soils were assumed to be Hydrologic Soil Group "C" and "D" in wetland areas.

FEMA Floodplain/Environmental Due Diligence

The Site borders and has a hydraulic connection to FEMA *Zone "AE"* elevation 83.0. *Zone "AE" areas* are areas of 1% annual chance (or 100-year) flood area. The official Flood Insurance Rate Map (FIRM) for the site is dated June 4, 2010 and shown on FEMA panel 25017C0314E. A copy of the FEMA FIRM is included in the appendix of this report.

Environmentally Sensitive Zones

The Commonwealth of Massachusetts asserts control over numerous protected and regulated areas including: Areas of Critical Environmental Concern (ACEC); Outstanding Resource Waters (ORWs); Priority and Protected Habitat for rare and endangered species, and areas protected under the Wetlands Protection Act. The subject property is not located within any of these regulated areas.

Drainage Analysis Methodology

A peak rate of runoff will be determined using techniques and data found in the following:

1. Urban Hydrology for Small Watersheds – Technical Release 55 by the United States Department of Agriculture Soils Conservation Service, June 1986. Runoff curve numbers and 24-hour precipitation values were obtained from this reference.
2. HydroCAD © Stormwater Modeling System by HydroCAD Software Solutions LLC, version 10.00-24. The HydroCAD program was used to generate the runoff hydrographs for the watershed areas, to determine discharge/ stage/storage characteristics for the stormwater BMPs, to perform drainage routing and to combine the results of the runoff hydrographs. HydroCAD uses the TR-20 methodology of the SCS Unit Hydrograph procedure (SCS-UH).

Proposed Conditions – Peak Rate of Runoff

The stormwater runoff analysis of the existing and proposed conditions includes an estimate of the peak rate of runoff from various rainfall events. Peak runoff rates were developed using TR55 Urban Hydrology for Small Watersheds, developed by the U.S.

Department of Commerce, Engineering Division and the HydroCAD computer program. Further, the analysis has been prepared in accordance with the MassDEP and the Town of Wakefield requirements and standard engineering practices. The peak rate of runoff has been estimated for each watershed during the 2, 10, and 100-year storm events.

The stormwater runoff model demonstrates that the overall peak rates of runoff into Lake Quannapowitt will not be increased under proposed conditions during all storm events. There will be a negligible increase (2%) to the peak rate of runoff at Study Point #3 due to the revised grading within the parkway. The eastern corner of the parkway has been raised to accommodate the new driveway entrance servicing the surface lots and also improve drainage within the parkway where it is currently very flat. The following tables provide a summary of the estimated peak rates of runoff for each of the design storm events. The HydroCAD worksheets are included in Section 3 and 4 of this report.

STUDY POINT #1 (Flow to South Culvert)			
	2-Year	10-Year	100-Year
Existing Flow (CFS)	12.45	21.29	34.16
Proposed Flow (CFS)	6.10	17.65	31.08
Decrease (CFS)	**6.35 (51%)**	**3.64 (17%)**	**3.08 (9%)**
Existing Volume (CF)	82,247	132,199	205,156
Proposed Volume (CF)	58,579	107,632	180,591
Decrease (CF)	**23,668 (29%)**	**24,567 (19%)**	**24,565 (12%)**

STUDY POINT #2 (Flow to Lake Quannapowitt)			
	2-Year	10-Year	100-Year
Existing Flow (CFS)	20.98	36.17	59.49
Proposed Flow (CFS)	10.98	28.50	54.84
Decrease (CFS)	**10.00 (48%)**	**7.67 (21%)**	**4.65 (8%)**
Existing Volume (CF)	147,693	252,264	410,892
Proposed Volume (CF)	99,650	200,127	354,864
Decrease (CF)	**48,043 (33%)**	**52,137 (21%)**	**56,028 (14%)**

STUDY POINT #3 (Flow to East Culvert)			
	2-Year	10-Year	100-Year
Existing Flow (CFS)	4.73	9.92	18.15
Proposed Flow (CFS)	4.82	10.10	18.48
Increase (CFS)	**0.09 (2%)**	**0.18 (2%)**	**0.33 (2%)**
Existing Volume (CF)	34,345	68,701	124,528
Proposed Volume (CF)	34,968	69,949	126,790
Increase (CF)	**623 (2%)**	**1,248 (2%)**	**2,262 (2%)**

MASSDEP Stormwater Performance Standards

The MA DEP Stormwater Management Policy was developed to improve water quality by implementing performance standards for stormwater management. The intent is to implement the stormwater management standards through the review of Notice of Intent filings by the issuing authority (Conservation Commission or DEP). The following section outlines how the proposed Stormwater Management System meets the standards set forth by the Policy.

BMP's implemented in the design include –

- Deep Sump Catch Basins
- Vegetated Filter Strip & Stone Diaphragm
- Subsurface Structures
- Bioretention Areas
- Surface Infiltration Basin
- Specific Maintenance Schedule

Stormwater Best Management Practices (BMP's) have been incorporated into the design of the project to mitigate the anticipated pollutant loading. An Operations and Maintenance Plan has been developed for the project, which addresses the long-term maintenance requirements of the proposed system.

Temporary erosion and sedimentation controls will be incorporated into the construction phase of the project. These temporary controls may include straw bale and/or silt fence barriers, inlet sediment traps, slope stabilization, and stabilized construction entrances.

The Massachusetts Department of Environmental Protection has established ten (10) Stormwater Management Standards. A project that meets or exceeds the standards is presumed to satisfy the regulatory requirements regarding stormwater management. The Standards are enumerated below as well as descriptions and supporting calculations as to how the Project will comply with the Standards:

1. *No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.*

   The proposed development will not introduce any new outfalls with direct discharge to a wetland area or waters of the Commonwealth of Massachusetts. All discharges will be treated for water quality and the rate will not be increased over existing conditions.

2. *Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard*

may be waived for discharges to land subject to coastal storm flowage as defined in 310 CMR 10.04.

The proposed development has been designed so that the post-development peak discharge rates do not exceed the predevelopment peak discharge rates. A summary of the existing and proposed discharge rates is included within this document.

3. *Loss of annual recharge to groundwater shall be eliminated or minimized through the use of infiltration measures including environmentally sensitive site design, low impact development techniques, stormwater best management practices, and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil type. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.*

   The proposed site improvements are classified as a "redevelopment" under the MA DEP Stormwater Management Standards based upon a reduction in impervious area. Consequently, compliance with Standard #3 is required only to the maximum extent practicable. This reduction is achieved with the installation of landscape areas.
   Existing impervious area = 9.77 ± acres
   Proposed impervious area = 8.82 ± acres
   Change in impervious area = -0.95 ± acres

   See the appendix located in section 6 of this report for stormwater recharge calculations.

4. *Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). This standard is met when:*
   - *Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan, and thereafter are implemented and maintained;*
   - *Structural stormwater best management practices are sized to capture the required water quality volume determined in accordance with the Massachusetts Stormwater Handbook; and*
   - *Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.*

Standard #4 is met when structural stormwater best management practices are sized to capture and treat the required water quality volume and pretreatment is provided in accordance with the Massachusetts Stormwater Handbook. Standard #4 also requires that suitable source control measures are identified in the Long-term Pollution Prevention Plan. The water quality volume for the site redevelopment is captured and treated using the BMPs listed above.

The implemented BMPs have been designed to treat the contributing water quality volume. These water quality calculations can be seen within the appendix of this report.

The proposed stormwater management system has been designed to remove 80% of the average annual post-construction load for each treatment train. The TSS removal calculations can be seen within the appendix of this report.

The TSS removal efficiencies for the proprietary separator are based on the values assigned under the Technology Acceptance and Reciprocity Partnership (TARP) testing protocol. The TARP is a workgroup of the Environmental Council of States that was originally comprised of California, Illinois, Maryland, Massachusetts, New Jersey, New York, Pennsylvania and Virginia. TARP is recognized in the MA DEP Stormwater Management Handbook as a valid source for assigning TSS removal efficiencies for proprietary separators.

5. *For land uses with higher potential pollutant loads, source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable. If through source control and/or pollution prevention all land uses with higher potential pollutant loads cannot be completely protected from exposure to rain, snow, snow melt, and stormwater runoff, the proponent shall use the specific structural stormwater BMPs determined by the Department to be suitable for such uses as provided in the Massachusetts Stormwater Handbook. Stormwater discharges from land uses with higher potential pollutant loads shall also comply with the requirements of the Massachusetts Clean Waters Act, M.G.L. c. 21, §§ 26-53 and the regulations promulgated thereunder at 314 CMR 3.00, 314 CMR 4.00 and 314 CMR 5.00.*

   The proposed redevelopment is considered a source of higher potential pollutant loads due to 1,000 or more expected vehicle trips per day. The SMS will be designed to treat 1" water quality volume.

6. *Stormwater discharges within the Zone II or Interim Wellhead Protection Area of a public water supply, and stormwater discharges near or to any other critical area, require the use of the specific source control and pollution prevention measures and the specific structural stormwater best management practices determined by the Department to be suitable for managing discharges to such areas, as provided in the Massachusetts Stormwater Handbook. A discharge is near a critical area if there is a strong likelihood of a significant impact occurring to said area, taking into account site-specific factors. Stormwater discharges to Outstanding Resource Waters and Special Resource Waters shall be removed and set back from the receiving water or wetland and receive the highest and best practical method of treatment. A "storm water discharge" as defined in 314 CMR 3.04(2)(a)1 or (b) to an Outstanding Resource Water or Special Resource Water shall comply with 314 CMR 3.00 and 314 CMR 4.00. Stormwater discharges to a Zone I or Zone A are prohibited unless essential to the operation of a public water supply.*

   The project site does not discharge stormwater within a Zone II or Interim Wellhead Protection Area or near a critical area. Critical Areas are Outstanding Resource Waters as designated in 314 CMR 4.00, Special Resource Waters as designated in 314 CMR 4.00, recharge areas for public water supplies as defined in 310 CMR 22.02, bathing beaches as defined in 105 CMR 445.000, cold-water fisheries as defined in 314 CMR 9.02 and 310 CMR 10.04, and shellfish growing areas as defined in 314 CMR 9.02 and 310 CMR 10.04.

7. *A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.*

   The proposed project is considered a redevelopment under the MA DEP Stormwater Management Standards as there is a decrease in the amount of total impervious area.

8. *A plan to control construction-related impacts including erosion, sedimentation and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented.*

A plan to control construction-related impacts, including erosion, sedimentation and other pollutant sources during construction has been developed. A detailed Erosion and Sedimentation Control Plan is included in the Permit Drawings. The proponent will prepare and submit a Stormwater Pollution Prevention Plan (SWPPP) prior to commencement of construction activities that will result in the disturbance of one acre of land or more.

9. *A long-term operation and maintenance plan shall be developed and implemented to ensure that stormwater management systems function as designed.*

   A Long-Term Operation & Maintenance (O&M) Plan has been developed for the proposed stormwater management system and is included within this document. See Section 2.0 of this report.

10. *All illicit discharges to the stormwater management system are prohibited.*

    There are no expected illicit discharges to the stormwater management system. The applicant will submit the Illicit Discharge Compliance Statement prior to the discharge of stormwater runoff to the post-construction stormwater best management practices and prior to the issuance of a Certificate of Compliance.

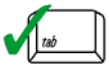
See the next page for the MassDEP Stormwater Checklist.

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.[1] This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8[2]
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

[1] The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

[2] For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

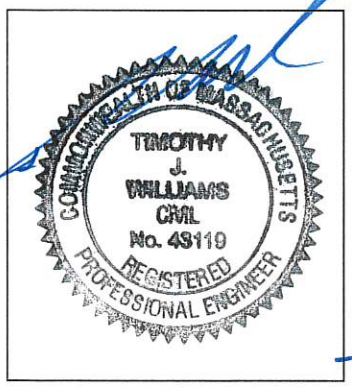
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



Signature and Date

3/16/21

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

☐ New development

☒ Redevelopment

☐ Mix of New Development and Redevelopment

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

☒ No disturbance to any Wetland Resource Areas

☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)

☒ Reduced Impervious Area (Redevelopment Only)

☒ Minimizing disturbance to existing trees and shrubs

☐ LID Site Design Credit Requested:

- ☐ Credit 1
- ☐ Credit 2
- ☐ Credit 3

☒ Use of "country drainage" versus curb and gutter conveyance and pipe

☒ Bioretention Cells (includes Rain Gardens)

☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)

☐ Treebox Filter

☐ Water Quality Swale

☐ Grass Channel

☐ Green Roof

☐ Other (describe): _____

Standard 1: No New Untreated Discharges

☒ No new untreated discharges

☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth

☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.

☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

☒ Soil Analysis provided.

☒ Required Recharge Volume calculation provided.

☐ Required Recharge volume reduced through use of the LID site Design Credits.

☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.

☒ Static ☐ Simple Dynamic ☐ Dynamic Field[1]

☐ Runoff from all impervious areas at the site discharging to the infiltration BMP.

☒ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:

- ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
- ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
- ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.

☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

[1] 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

☒ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.

☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.

☒ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:

- ☐ is within the Zone II or Interim Wellhead Protection Area
- ☐ is near or to other critical areas
- ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
- ☒ involves runoff from land uses with higher potential pollutant loads.

☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.

☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

☒ The BMP is sized (and calculations provided) based on:

- ☒ The ½" or 1" Water Quality Volume or
- ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.

☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.

☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.

☒ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted ***prior to*** the discharge of stormwater to the post-construction stormwater BMPs.

☐ The NPDES Multi-Sector General Permit does ***not*** cover the land use.

☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.

☐ All exposure has been eliminated.

☐ All exposure has ***not*** been eliminated and all BMPs selected are on MassDEP LUHPPL list.

☒ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.

☐ Critical areas and BMPs are identified in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

☒ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- ☐ Limited Project
- ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- ☐ Bike Path and/or Foot Path
- ☒ Redevelopment Project
- ☐ Redevelopment portion of mix of new and redevelopment.

☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☒ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has ***not*** been included in the Stormwater Report but will be submitted ***before*** land disturbance begins.

☐ The project is ***not*** covered by a NPDES Construction General Permit.

☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.

☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:

- ☒ Name of the stormwater management system owners;
- ☒ Party responsible for operation and maintenance;
- ☒ Schedule for implementation of routine and non-routine maintenance tasks;
- ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
- ☒ Description and delineation of public safety features;
- ☒ Estimated operation and maintenance budget; and
- ☒ Operation and Maintenance Log Form.

☐ The responsible party is ***not*** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:

- ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
- ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;

☐ An Illicit Discharge Compliance Statement is attached;

☒ NO Illicit Discharge Compliance Statement is attached but will be submitted ***prior to*** the discharge of any stormwater to post-construction BMPs.

SECTION 2.0 -
OPERATION & MAINTENANCE PLAN

Introduction

In accordance with the standards set forth by the Stormwater Management Policy issued by the Massachusetts Department of Environmental Protection (MassDEP), Allen & Major Associates, Inc. has prepared the following Operations & Maintenance (O&M) Plan for the existing development at 200 Quannapowitt Parkway, Wakefield, MA.

The plan is broken down into three major sections. The first section describes construction-related erosion and sedimentation controls (Demolition & Construction Maintenance Plan). The second section describes the long-term pollution prevention measures (Long Term Pollution Prevention Plan). The third section is a post-construction operation and maintenance plan designed to address the long-term maintenance needs of the stormwater management system (Long-Term Maintenance Plan – Facilities Description).

Notification Procedures for Change of Responsibility for O&M

The Stormwater Management System (SMS) for this project is owned by Cabot, Cabot & Forbes (owner). The owner shall be legally responsible for the long-term operation and maintenance of this SMS as outlined in this Operation and Maintenance Plan.

The owner shall submit an annual summary report and the completed Operation & Maintenance Schedule & Checklist to the Conservation Commission (via email or print copy), highlighting inspection and maintenance activities including performances of BMPs. Should ownership of the SMS change, the owner will continue to be responsible until the succeeding owner shall notify the Commission that the succeeding owner has assumed such responsibility. Upon subsequent transfers, the responsibility shall continue to be that of transferring owner until the transferee owner notifies the Commission of its assumption of responsibility.

In the event the SMS will serve multiple lots/owners, such as the subdivision of the existing parcel or creation of lease areas, the owner(s) shall establish an association on other legally enforceable arrangements under which the association or a single party shall have legal responsibility for the operation and maintenance of the entire SMS. The legal instrument creating such responsibility shall be recorded with the Registry of Deeds and promptly following its recording, a copy thereof shall be furnished to the Commission.

Contact Information

Stormwater Management System Owner:	Cabot, Cabot & Forbes 185 Dartmouth Street Boston, MA 02116 Phone: 617-603-4000

Emergency Contact Information:

Allen & Major Associates, Inc. (Site Civil Engineer)	Phone: (781) 935-6889
Wakefield Department of Public Works	Phone: 781-246-6301
Wakefield Conservation Commission	Phone: 781-224-5015
Wakefield Fire Department (non-emergency line)	Phone: 781-246-6435
MassDEP Emergency Response	Phone: (888) 304-1133
Clean Harbors Inc (24-Hour Line)	Phone: (800) 645-8265

Demolition & Construction Maintenance Plan

1. Call Digsafe: 1-888-344-7233
2. Contact the Town of Wakefield at least three (3) days prior to start of demolition and/or construction activities.
3. Install Erosion Control measures as shown on the Plans prepared by A&M. The Town of Wakefield shall review the installation of straw bales and silt fencing prior to the start of any site demolition work. Install Construction fencing if determined to be necessary at the commencement of construction.
4. Install construction entrances, straw bales, and silt fence at the locations shown on the Erosion Control Plan prepared by A&M.
5. Site access shall be achieved only from the designated construction entrances.
6. Cut and clear trees in construction areas only (within the limit of work; see plans).
7. Stockpiles of materials subject to erosion shall be stabilized with erosion control matting or temporary seeding whenever practicable, but in no case more than 14 days after the construction activity in that portion of the site has temporarily or permanently ceased.
8. Install silt sacks and straw bales around each drain inlet prior to any demolition and or construction activities.
9. All erosion control measures shall be inspected weekly and after every rainfall event. Records of these inspections shall be kept on-site for review.

10. All erosion control measures shall be maintained, repaired, or replaced as required or at the direction of the owner's engineer or the Town of Wakefield.
11. Sediment accumulation up-gradient of the straw bales, silt fence, and stone check dams greater than 6" in depth shall be removed and disposed of in accordance with all applicable regulations.
12. If it appears that sediment is exiting the site, silt sacks shall be installed in all catch basins adjacent to the site. Sediment accumulation on all adjacent catch basin inlets shall be removed and the silt sack replaced if torn or damaged.
13. Install stone check dams on-site during construction as needed. Refer to the erosion control details. Temporary sediment basins combined with stone check damns shall be installed on-site during construction to control and collect runoff from upland areas of this site during demolition and construction activities.
14. The contractor shall comply with the Sedimentation and Erosion Control Notes as shown on the Site Development Plans and Specifications.
15. The stabilized construction entrances shall be inspected weekly and records of inspections kept. The entrances shall be maintained by adding additional clean, angular, durable stone to remove the soil from the construction vehicle's tires when exiting the site. If soil is still leaving the site via the construction vehicle tires, adjacent roadways shall be kept clean by street sweeping.
16. Dust pollution shall be controlled using on-site water trucks and/or an approved soil stabilization product.
17. During demolition and construction activities, Status Reports on compliance with this O&M Document shall be submitted weekly. The report shall document any deficiencies and corrective actions taken by the applicant.

Long-Term Pollution Prevention Plan

Standard #4 from the MassDEP Stormwater Management Handbook requires that a Long-Term Pollution Prevention Plan (LTPPP) be prepared and incorporated as part of the Operation and Maintenance Plan of the Stormwater Management System. The purpose of the LTPPP is to identify potential sources of pollution that may affect the quality of stormwater discharges, and to describe the implementation of practices to reduce the pollutants in stormwater discharges. The following items describe the source control and proper procedures of the LTPPP.

- Housekeeping

The existing development has been designed to maintain a high level of water quality treatment for all stormwater discharge to the wetland areas. An Operation and Maintenance (O&M) plan has been prepared and is included in this section of the report. The owner (or its designee) is responsible for adherence to the O&M plan in a strict and complete manner.

- Storing of Materials & Water Products
  The trash and waste program for the site includes exterior dumpsters. There is a trash contractor used to pick up the waste material in the dumpsters. The stormwater drainage system has water quality inlets designed to capture trash and debris.
- Vehicle Washing
  Outdoor vehicle washing has the potential to result in high loads of nutrients, metals, and hydrocarbons during dry weather conditions, as the detergent-rich water used to wash the grime off the vehicle enters the stormwater drainage system. The existing development does not include any designated vehicle washing areas, nor is it expected that any vehicle washing will take place on-site.
- Spill Prevention & Response
  Sources of potential spill hazards include vehicle fluids, liquid fuels, pesticides, paints, solvents, and liquid cleaning products. The majority of the spill hazards would likely occur within the buildings and would not enter the stormwater drainage system. However, there are spill hazards from vehicle fluids or liquid fuels located outside of the buildings. These exterior spill hazards have the potential to enter the stormwater drainage system and are to be addressed as follows:

  1. Spill hazards of pesticides, paints, and solvents shall be remediated using the Manufacturers' recommended spill cleanup protocol.
  2. Vehicle fluids and liquid fuel spill shall be remediated according to the local and state regulations governing fuel spills.
  3. The owner shall have the following equipment and materials on hand to address a spill clean-up: brooms, dust pans, mops, rags, gloves, absorptive material, sand, sawdust, plastic and metal trash containers.
  4. All spills shall be cleaned up immediately after discovery.
  5. Spills of toxic or hazardous material shall be reported, regardless of size, to the Massachusetts Department of Environmental Protection at (888) 304-1333.

6. Should a spill occur, the pollution prevention plan will be adjusted to include measures to prevent another spill of a similar nature. A description of the spill, along with the causes and cleanup measures will be included in the updated pollution prevention plan.

- Maintenance of Lawns, Gardens, and Other Landscaped Areas
  It should be recognized that this is a general guideline towards achieving high quality and well-groomed landscaped areas. The grounds staff/landscape contractor must recognize the shortcomings of a general maintenance plan such as this, and modify and/or augment it based on weekly, monthly, and yearly observations. In order to assure the highest quality conditions, the staff must also recognize and appreciate the need to be aware of the constantly changing conditions of the landscaping and be able to respond to them on a proactive basis. No trees shall be planted over the drain lines or recharge area, and that only shallow rooted plants and shrubs will be allowed.

  - Fertilizer
    Maintenance practices should be aimed at reducing environmental, mechanical and pest stresses to promote healthy and vigorous growth. When necessary, pest outbreaks should be treated with the most sensitive control measure available. Synthetic chemical controls should be used only as a last resort to organic and biological control methods. Fertilizer, synthetic chemical controls and pest management applications (when necessary) shall be performed only by licensed applicators in accordance with the manufacturer's label instructions when environmental conditions are conducive to controlled product application.

    Only slow-release organic fertilizers should be used in the planting and mulch areas to limit the amount of nutrients that could enter downstream resource areas. Fertilization of the planting and mulch areas will be performed within manufacturers labeling instructions and shall not exceed an NPK ration of 1:1:1 (i.e. Triple 10 fertilizer mix), considered a low nitrogen mixture. Fertilizers approved for the use under this O&M Plan are as follows:

    Type: LESCO® 28-0-12 (Lawn Fertilizer)
    MERIT® 0.2 Plus Turf Fertilizer
    MOMENTUM™ Force Weed & Feed

  - Suggested Aeration Program
    In-season aeration of lawn areas is good cultural practice, and is recommended whenever feasible. It should be accomplished with a solid thin tine aeration method to reduce disruption to the use of the area. The

depth of solid tine aeration is similar to core type, but should be performed when the soil is somewhat drier for a greater overall effect.

Depending on the intensity of use, it can be expected that all landscaped lawn areas will need aeration to reduce compaction at least once per year. The first operation should occur in late May following the spring season. Methods of reducing compaction will vary based on the nature of the compaction. Compaction on newly established landscaped areas is generally limited to the top 2-3" and can be alleviated using hollow core or thin tine aeration methods.

The spring aeration should consist of two passes at opposite directions with 1/4" hollow core tines penetrating 3-5" into the soil profile. Aeration should occur when the soil is moist but not saturated. The soil cores should be shattered in place and dragged or swept back into the turf to control thatch. If desired the cores may also be removed and the area top-dressed with sand or sandy loam. If the area drains on average too slowly, the topdressing should contain a higher percentage of sand. If it is draining on average too quickly, the top dressing should contain a higher percentage of soil and organic matter.

- Landscape Maintenance Program Practices:
  - Lawn
    1. Mow a minimum of once a week in spring, to a height of 2" to 2 1/2" high. Mowing should be frequent enough so that no more than 1/3 of grass blade is removed at each mowing. The top growth supports the roots; the shorter the grass is cute, the less the roots will grow. Short cutting also dries out the soil and encourages weeds to germinate.
    2. Mow approximately once every two weeks from July 1st to August 15th depending on lawn growth.
    3. Mow on a ten-day cycle in fall, when growth is stimulated by cooler nights and increased moisture.
    4. Do not remove grass clippings after mowing.
    5. Keep mower blades sharp to prevent ragged cuts on grass leaves, which cause a brownish appearance and increase the chance for disease to enter a leaf.
  - Shrubs
    1. Mulch not more than 3" depth with shredded pine or fir bark.

2. Hand prune annually, immediately after blooming, to remove 1/3 of the above-ground biomass (older stems). Stem removals are to occur within 6" of the ground to open up shrub and maintain two-year wood (the blooming wood).
3. Hand-prune evergreen shrubs only as needed to remove dead and damaged wood and to maintain the naturalistic form of the shrub. Never mechanically shear evergreen shrubs.

- Trees
  1. Provide aftercare of new tree plantings for the first three years.
  2. Do not fertilize trees, it artificially stimulates them (unless tree health warrants).
  3. Water once a week for the first year; twice a month for the second; once a month for the third year.
  4. Prune trees on a four-year cycle.
- Invasive Species
  1. Inform the Conservation Commission Agent prior to the removal of invasive species proposed either through hand work or through chemical removal.

- Storage and Use of Herbicides and Pesticides

  Integrated Pest Management is the combination of all methods (of pest control) which may prevent, reduce, suppress, eliminate, or repel an insect population. The main requirements necessary to support any pest population are food, shelter and water, and any upset of the balance of these will assist in controlling a pest population. Scientific pest management is the knowledgeable use of all pest control methods (sanitation, mechanical, chemical) to benefit mankind's health, welfare, comfort, property and food. A Pest Management Professional (PMP) should be retained who is licensed with the Commonwealth of Massachusetts Executive Office of Energy and Environmental Affairs, Department of Agricultural Resources.

  The site manager will be provided with approved bulletin before entering into or renewing an agreement to apply pesticides for the control of indoor household or structural pests, refer to 333 CMR 13.08.

  Before beginning each application, the applicator must post a Department approved notice on all of the entrances to the treated room or area. The applicator must leave such notices posted after the application. The notice will be posted at conspicuous point(s) of access to the area treated. The location and

number of signs will be determined by the configuration of the area to be treated based on the applicator's best judgment. It is intended to give sufficient notice so that no one comes into an area being treated unaware that the applicator is working and pesticides are being applied. However, if the contracting entity does not want the signs posted, he/she may sign a Department approved waiver indicating this.

The applicator or employer will provide to any person upon their request the following information on previously conducted applications:

1. Name and phone number of pest control company;
2. Date and time of the application;
3. Name and license number of the applicator;
4. Target pests; and
5. Name and EPA Registration Number of pesticide products applied.

- Pet Waste Management
  The owner's landscape crew (or designee) shall remove any obvious pet waste that has been left behind by pet owners within the development. The pet waste shall be disposed of in accordance with local and state regulations.
- Operations and Management of Septic Systems
  There are no proposed septic systems within the limits of the project.
- Management of Deicing Chemicals and Snow
  Snow will be stockpiled on site until the accumulated snow becomes a hazard to the daily operations of the site. It will be the responsibility of the snow removal contractor to properly dispose of transported snow according to MassDEP, Bureau of Resource Protection – Snow Disposal Guideline #BRPG01-01, governing the proper disposal of snow. It will be the responsibility of the snow removal contractor to follow these guidelines and all applicable laws and regulations

  The owner's maintenance staff (or its designee) will be responsible for the clearing of the sidewalk and building entrances. The owner may be required to use a de-icing agent such as potassium chloride to maintain a safe walking surface. If used, the de-icing agent for the walkways and building entrances will be kept within the storage rooms located within the building. If used, de-icing agents will not be stored outside. The owner's maintenance staff will limit the application of sand.

Long-Term Maintenance Plan – Facilities Description

A maintenance log will be kept (i.e. report) summarizing inspections, maintenance, and any corrective actions taken. The log will include the date on which each inspection or

maintenance task was performed, a description of the inspection findings or maintenance completed, and the name of the inspector or maintenance personnel performing the task. If a maintenance task requires the clean-out of any sediments or debris, the location where the sediment and debris was disposed after removal will be indicated. The log will be made accessible to department staff and a copy provided to the department upon request.

The following is a description of the Stormwater Management System for the project site.

Stormwater Collection System – On-Site:

The stormwater collection system is a series of inlets located at low points within the limits of the paved area. All of the proposed on-site catch basins incorporate a deep sump and hooded outlet. The catch basins are connected by a closed gravity pipe network routed to an isolator row within the underground detention chambers.

Roof runoff discharges directly to the underground chambers, bioretention area, or surface infiltration basin. All remaining runoff along the perimeter of the site and within the parkway, sheet flows through vegetated filter strips equipped with a stone diaphragm before entering the wetlands and/or drainage channel.

Pretreatment BMPs: Regular maintenance of these BMPs is especially critical because they typically receive the highest concentration of suspended solids during the first flush of a storm event.

- Deep Sump Catch Basin:
  Precast structure equipped with grated inlet and 4' sump to allow sediment to settle out.
- Isolator Row:
  Single row of underground chambers wrapped in geotextile to filter out sediment. Equipped with overflow into remaining chambers.
- Vegetated Filter Strip:
  Uniformly graded vegetated surfaces that receive runoff from adjacent impervious surfaces via sheet flow.

Treatment BMPs:

- Exfiltrating Bioretention Area:
  Shallow depressions filled with sandy soil topped with a thick layer of mulch and planted with dense native vegetation. Equipped with overflow and underdrain.

Infiltration BMPs:

- Subsurface Structures
  Underground chambers surrounded by stone used to store large volumes of stormwater and allow for infiltration into the groundwater.
- Infiltration Basin:
  Stormwater runoff impoundments that are constructed over permeable soils.

Other Maintenance Activity:

- Mosquito Control - Both above ground and underground stormwater BMPs have the potential to serve as mosquito breeding areas. Good design, proper operation and maintenance, and treatment with larvicides can minimize this potential. See the supplemental information for Mosquito Control in Stormwater Management Practices, and the Operation and Maintenance Plan Schedule for inspection schedule.
- Street Sweeping - Clear accumulations of winter sand in parking lots and along roadways at least once a year, preferably in the spring. Accumulations on pavement may be removed by pavement sweeping. Accumulations of sand along road shoulders may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader.

Inspection and Maintenance Frequency and Corrective Measures
In accordance with MA DEP Stormwater Handbook: Volume 2, Chapter 2; the previously described BMPs will be inspected and the identified deficiencies will be corrected. Clean-out must include the removal and legal disposal of any accumulated sediments, trash, and debris. In any and all cases, operations, inspections, and maintenance activities shall utilize best practical measures to avoid and minimize impacts to wetland resource areas outside the footprint of the SMS.

Supplemental Information

- Operation & Maintenance Plan Schedule
- Massachusetts Stormwater Handbook, Chapter 5, Miscellaneous Stormwater Topics, Mosquito Control in Stormwater Management Practices.
- MassDEP Bureau of Water Resources Snow Disposal Guidance
- Stormtech Isolator Row O&M Manual

OPERATION AND MAINTENANCE PLAN SCHEDULE **Date: March 16, 2021**

ALLEN & MAJOR ASSOCIATES, INC.

Project: 200 Quannapowitt Parkway
Project Address: 200 Quannapowitt Parkway Wakefield, MA

Responsible for O&M Plan: Cabot, Cabot & Forbes
Address: 185 Dartmouth Street Boston, MA

All information within table is derived from Massachussetts Stormwater Handbook: Volume 2, Chapter 2

BMP CATEGORY	BMP OR MAINTENANCE ACTIVITY	SCHEDULE/ FREQUENCY	NOTES	ESTIMATED ANNUAL MAINTENANCE COST	INSPECTION PERFORMED	
					DATE:	BY:
STRUCTURAL PRETREATMENT BMPs	DEEP SUMP CATCH BASIN	Four times per year (quarterly).	Inspect and clean catch basin units whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin.	$1,000		
	VEGETATED FILTER STRIPS	Inspect every six months during the first year and annually thereafter. Mow and remove sediment as needed.	Inspect the vegetation for signs of erosion, bare spots, and overall health. Regularly mow the grass. Remove sediment from the toe of slope or level spreader and reseed bare spots.	$250		
TREATMENT BMPs	BIORETENTION AREA & RAIN GARDEN	Remove trash monthly. Remove and replace dead vegetation, prune and mulch annually.	Inspect & remove trash, Mulch, Remove dead vegetation, Replace dead vegetation, Prune, Replace entire media & all vegetation.	$3,000		
INFILTRATION BMPs	INFILTRATION BASIN	Inspect after every major storm during first 3 months of operation and twice a year thereafter. Clean pretreatment devices twice a year and after every major storm.	Inspect to ensure proper functioning. Mow the buffer area, side slopes, and basin bottom if grassed floor; rake if stone bottom; remove trash and debris; remove grass clippings and accumulated organic matter. Inspect and clean pretreatment devices.	$1,500		
	SUBSURFACE STRUCTURES	Inspect structure inlets at least twice a year. Remove debris that may clog the system as needed.	Because subsurface structures are installed underground, they are extremely difficult to maintain. Remove any debris that might clog the system.	$500		
OTHER MAINTENANCE ACTIVITY	MISQUITO CONTROL	Inspect BMPs as needed to ensure the system's drainage time is less than the maximum 72 hour period.	Massachusetts stormwater handbook requires all stormwater practices that are designed to drain do so within 72 hours to reduce the number of mosquitos that mature to adults since the aquatic stage of a mosquito is 7-10 days.	$100		
	SNOW STORAGE	Clear and remove snow to approved storage locations as necessary to ensure systems are working properly and are protected from meltwater pollutants.	Carefully select snow disposal sites before winter. Avoid dumping removed snow over catch basins, or in detention ponds, sediment forebays, rivers, wetlands, and flood plains. It is also prohibited to dump snow in the bioretention basins or gravel swales.	$500		
	STREET SWEEPING	Clear accumulations of winter sand in parking lots and along roadways at least once a year, preferably in the spring.	Sweep, power broom or vacuum paved areas. Submit information that confirms that all street sweepings have been completed in accordance with state and local requirements	$2,000		

Chapter 5
Miscellaneous Stormwater Topics

Mosquito Control in Stormwater Management Practices

Both aboveground and underground stormwater BMPs have the potential to serve as mosquito breeding areas. Good design, proper operation and maintenance and treatment with larvicides can minimize this potential.

EPA recommends that stormwater treatment practices dewater within 3 days (72 hours) to reduce the number of mosquitoes that mature to adults, since the aquatic stage of many mosquito species is 7 to 10 days. Massachusetts has had a 72-hour dewatering rule in its Stormwater Management Standards since 1996. The 2008 technical specifications for BMPs set forth in Volume 2, Chapter 2 of the Massachusetts Stormwater Handbook also concur with this practice by requiring that all stormwater practices designed to drain do so within 72 hours.

Some stormwater practices are designed to include permanent wet pools. These practices – if maintained properly – can limit mosquito breeding by providing habitat for mosquito predators. Additional measures that can be taken to reduce mosquito populations include increasing water circulation, attracting mosquito predators by adding suitable habitat, and applying larvicides.

The Massachusetts State Reclamation and Mosquito Control Board (SRMCB), through the Massachusetts Mosquito Control Districts, can undertake further mosquito control actions specifically for the purpose of mosquito control pursuant to Massachusetts General Law Chapter 252. The Mosquito Control Board, http://www.mass.gov/agr/mosquito/, describes mosquito control methods and is in the process of developing guidance documents that describe Best Management Practices for mosquito control projects.

The SRMCB and Mosquito Control Districts are not responsible for operating and maintaining stormwater BMPs to reduce mosquito populations. The owners of property that construct the stormwater BMPs or municipalities that "accept" them through local subdivision approval are responsible for their maintenance.[1] The SRMCB is composed of officials from MassDEP, Department of Agricultural Resources, and Department of Conservation and Recreation. The nine (9) Mosquito Control Districts overseen by the SRMCB are located throughout Massachusetts, covering 176 municipalities.

Construction Period Best Management Practices for Mosquito Control

To minimize mosquito breeding during construction, it is essential that the following actions be taken to minimize the creation of standing pools by taking the following actions:

- ***Minimize Land Disturbance:*** Minimizing land disturbance reduces the likelihood of mosquito breeding by reducing silt in runoff that will cause construction period controls to clog and retain standing pools of water for more than 72 hours.
- ***Catch Basin inlets:*** Inspect and refresh filter fabric, hay bales, filter socks or stone dams on a regular basis to ensure that any stormwater ponded at the inlet drains within 8 hours after precipitation stops. Shorter periods may be necessary to avoid hydroplaning in roads

[1] MassDEP and MassHighway understand that the numerous stormwater BMPs along state highways pose a unique challenge. To address this challenge, the 2004 MassHighway Stormwater Handbook will provide additional information on appropriate operation and maintenance practices for mosquito control when the Handbook is revised to reflect the 2008 changes to the Stormwater Management Standards..

caused by water ponded at the catch basin inlet. Treat catch basin sumps with larvicides such as *Bacillus sphaericus* (*Bs*) using a licensed pesticide applicator.

- ***Check Dams:*** If temporary check dams are used during the construction period to lag peak rate of runoff or pond runoff for exfiltration, inspect and repair the check dams on a regular basis to ensure that any stormwater ponded behind the check dam drains within 72 hours.
- ***Design construction period sediment traps*** to dewater within 72 hours after precipitation. Because these traps are subject to high silt loads and tend to clog, treat them with the larvicide *Bs* after it rains from June through October, until the first frost occurs.
- ***Construction period open conveyances:*** When temporary manmade ditches are used for channelizing construction period runoff, inspect them on a regular basis to remove any accumulated sediment to restore flow capacity to the temporary ditch.
- ***Revegetating Disturbed Surfaces:*** Revegetating disturbed surfaces reduces sediment in runoff that will cause construction period controls to clog and retain standing pools of water for greater than 72 hours.
- ***Sediment fences/hay bale barriers:*** When inspections find standing pools of water beyond the 24-hour period after a storm, take action to restore barrier to its normal function.

Post-Construction Stormwater Treatment Practices

- Mosquito control begins with the environmentally sensitive site design. Environmentally sensitive site design that minimizes impervious surfaces reduces the amount of stormwater runoff. Disconnecting runoff using the LID Site Design credits outlined in the Massachusetts Stormwater Handbook reduces the amount of stormwater that must be conveyed to a treatment practice. Utilizing green roofs minimizes runoff from smaller storms. Storage media must be designed to dewater within 72 hours after precipitation.
- Mosquito control continues with the selection of structural stormwater BMPs that are unlikely to become breeding grounds for mosquitoes, such as:
    - ***Bioretention Areas/Rain Gardens/Sand Filter:*** These practices tend not to result in mosquito breeding. If any level spreaders, weirs or sediment forebays are used as part of the design, inspect them and correct them as necessary to prevent standing pools of water for more than 72 hours.
    - ***Infiltration Trenches:*** This practice tends not to result in mosquito breeding. If any level spreaders, weirs, or sediment forebays are used as part of the design, inspect them and correct them as necessary to prevent standing pools of water for more than 72 hours.
- Another mosquito control strategy is to select BMPs that can become habitats for mosquito predators, such as:
    - ***Constructed Stormwater Wetlands:*** Habitat features can be incorporated in constructed stormwater wetlands to attract dragonflies, amphibians, turtles, birds, bats, and other natural predators of mosquitoes.
    - ***Wet Basins:*** Wet basins can be designed to incorporate fish habitat features, such as deep pools. Introduce fish in consultation with Massachusetts Division of Fisheries and Wildlife. Vegetation within wet basins designed as fish habitat must be properly managed to ensure that vegetation does not overtake the habitat. Proper design to ensure that no low circulation or "dead" zones are created may reduce the potential for mosquito breeding. Introducing bubblers may increase water circulation in the wet basin.

Effective mosquito controls require proponents to design structural BMPs to prevent ponding and facilitate maintenance and, if necessary, the application of larvicides. Examples of such design practices include the following:

- ***Basins:*** Provide perimeter access around wet basins, extended dry detention basins and dry detention basins for both larviciding and routine maintenance. Control vegetation to ensure that access pathways stay open.
- ***BMPs without a permanent pool of water:*** All structural BMPs that do not rely on a permanent pool of water must drain and completely dewater within 72 hours after precipitation. This includes dry detention basins, extended dry detention basins, infiltration basins, and dry water quality swales. Use underdrains at extended dry detention basins to drain the small pools that form due to accumulation of silts. Wallace indicates that extended dry extended detention basins may breed more mosquitoes than wet basins. It is, therefore, imperative to design outlets from extended dry detention basins to completely dewater within the 72-hour period.
- ***Energy Dissipators and Flow Spreaders:*** Currier and Moeller, 2000 indicate that shallow recesses in energy dissipators and flow spreaders trap water where mosquitoes breed. Set the riprap in grout to reduce the shallow recesses and minimize mosquito breeding.
- ***Outlet control structures:*** Debris trapped in small orifices or on trash racks of outlet control structures such as multiple stage outlet risers may clog the orifices or the trash rack, causing a standing pool of water. Optimize the orifice size or trash rack mesh size to provide required peak rate attenuation/water quality detention/retention time while minimizing clogging.
- ***Rain Barrels and Cisterns:*** Seal lids to reduce the likelihood of mosquitoes laying eggs in standing water. Install mosquito netting over inlets. The cistern system should be designed to ensure that all collected water is drained into it within 72 hours.
- ***Subsurface Structures, Deep Sump Catch Basins, Oil Grit Separators, and Leaching Catch Basins:*** Seal all manhole covers to reduce likelihood of mosquitoes laying eggs in standing water. Install mosquito netting over the outlet (CALTRANS 2004).

The Operation and Maintenance Plan should provide for mosquito prevention and control.

- ***Check dams:*** Inspect permanent check dams on the schedule set forth in the O&M Plan. Inspect check dams 72 hours after storms for standing water ponding behind the dam. Take corrective action if standing water is found.
- ***Cisterns:*** Apply *Bs* larvicide in the cistern if any evidence of mosquitoes is found. The Operation and Maintenance Plan shall specify how often larvicides should be applied to waters in the cistern.
- ***Water quality swales:*** Remove and properly dispose of any accumulated sediment as scheduled in the Operation and Maintenance Plan.
- ***Larvicide Treatment:*** The Operation and Maintenance Plan must include measures to minimize mosquito breeding, including larviciding.
- The party identified in the Operation and Maintenance Plan as responsible for maintenance shall see that larvicides are applied as necessary to the following stormwater treatment practices: catch basins, oil/grit separators, wet basins, wet water quality swales, dry extended detention basins, infiltration basins, and constructed stormwater wetlands. The Operation and Maintenance Plan must ensure that all larvicides are applied by a licensed pesticide applicator and in compliance with all pesticide label requirements.
- The Operation and Maintenance Plan should identify the appropriate larvicide and the time and method of application. For example, *Bacillus sphaericus (Bs*), the preferred

larvicide for stormwater BMPs, should be hand-broadcast.[2] Alternatively, Altosid, a Methopren product, may be used. Because some practices are designed to dewater between storms, such as dry extended detention and infiltration basins, the Operation and Maintenance Plan should provide that larviciding must be conducted during or immediately after wet weather, when the detention or infiltration basin has a standing pool of water, unless a product is used that can withstand extended dry periods.

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[2] *Bacillus thuringienis israelensis* or *Bti* is usually applied by helicopter to wetlands and floodplains

Roads and Stormwater BMPs

In general, the stormwater BMPs used for land development projects can also be used for new roadways and roadway improvement projects. However, for improvement of existing roads, there are often constraints that limit the choice of BMP. These constraints derive from the linear configuration of the road, the limited area within the existing right-of-way, the structural and safety requirements attendant to good roadway design, and the long-term maintainability of the roadway drainage systems. The MassHighway Handbook provides strategies for dealing with the constraints associated with providing stormwater BMPs for roadway redevelopment projects.

Roadway design can minimize impacts caused by stormwater. Reducing roadway width reduces the total and peak volume of runoff. Designing a road with country drainage (no road shoulders or curbs) disconnects roadway runoff. Disconnection of roadway runoff is eligible for the Low Impact Site Design Credit provided the drainage is disconnected in accordance with specifications outlined in Volume 3.

Like other parties, municipalities that work within wetlands jurisdictional areas and adjacent buffer zones must design and implement structural stormwater best management practices in accordance with the Stormwater Management Standards and the Stormwater Management Handbook. In addition, in municipalities and areas where state agencies operate stormwater systems, the DPWs (or other town or state agencies) must meet the "good housekeeping" requirement of the municipality's or agency's MS4 permit.

MassHighway has taken stormwater management one step further by working with MassDEP to develop the MassHighway Storm Water Handbook for Highways and Bridges. The purpose of the MassHighway Handbook is to provide guidance for persons involved in the design, permitting, review and implementation of state highway projects, especially those involving existing roadways where physical constraints often limit the stormwater management options available. These constraints, like those common to redevelopment sites, may make it difficult to comply precisely with the requirements of the Stormwater Management Standards and the Massachusetts Stormwater Handbook.[3] In response to these constraints, MassDEP and MHD developed specific design, permitting, review and implementation practices that meet the unique challenges of providing environmental protection for existing state roads. The information in the MassHighway Handbook may also aid in the planning and design of projects to build new highways and to add lanes to existing highways, since they may face similar difficulties in meeting the requirements of the Stormwater Management Standards.

Although it is very useful, the MassHighway Handbook does not allow MassHighway projects to proceed without individual review and approval by the issuing authority when subject to the Wetlands Protection Act Regulations, 310 CMR 10.00, or the 401 Water Quality Certification Regulations, 314 CMR 9.00. For example, MassHighway must provide a Conservation Commission with a project-specific Operation and Maintenance Plan in accordance with Standard 9 that documents how the project's post-construction BMPs will be operated and maintained.[4]

[3] The 2004 MassHighway Handbook outlines standardized methods for dealing with these constraints as they apply to highway redevelopment projects. MassDEP and MassHighway intend to work together to provide guidance for add a lane projects when the 2004 Handbook is revised to reflect the 2008 changes to the Stormwater Management Standards.

[4] The general permit for municipal separate storm sewer systems (the MS4 Permit) requires MassHighway to develop and implement procedures for the proper operation and maintenance of stormwater BMPs. To

Some municipalities have asked if the MassHighway Handbook governs municipal road projects. The answer is no.[5] The MassHighway Handbook was developed in response to the unique problems and challenges arising out of the management of the state highway system. Like other project proponents, cities and towns planning road or other projects in areas subject to jurisdiction under the Wetlands Protection Act must design and implement LID, non-structural and structural best management practices in accordance with the Stormwater Management Standards and the Massachusetts Stormwater Handbook.

avoid duplication of effort, MassHighway may be able rely on the same procedures to fulfill the operation and maintenance requirements of Standard 9 and the MS 4 Permit.

[5] Although the MassHighway Handbook does not govern municipal road projects, cities and towns may find some of the information presented in the Handbook useful.

Commonwealth of Massachusetts
Executive Office of Energy & Environmental Affairs

Department of Environmental Protection
One Winter Street Boston, MA 02108 • 617-292-5500

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Martin Suuberg
Commissioner

Massachusetts Department of Environmental Protection
Bureau of Water Resources
Snow Disposal Guidance

Effective Date: December 23, 2019

Applicability: Applies to all federal, state, regional and local agencies, as well as to private businesses.

Supersedes: Bureau of Resource Protection (BRP) Snow Disposal Guideline No. BRPG97-1 issued December 12, 1997 and BRPG01-01 issued March 8, 2001; Bureau of Water Resources (BWR) snow disposal guidance issued December 21, 2015 and December 12, 2018.

Approved by: Kathleen Baskin, Assistant Commissioner, Bureau of Water Resources

PURPOSE: To provide guidelines to all government agencies and private businesses regarding snow disposal site selection, site preparation and maintenance, and emergency snow disposal options that are protective of wetlands, drinking water, and water bodies, and are acceptable to the Massachusetts Department of Environmental Protection (MassDEP), Bureau of Water Resources.

APPLICABILITY: These Guidelines are issued by MassDEP's Bureau of Water Resources on behalf of all Bureau Programs (including Drinking Water Supply, Wetlands and Waterways, Wastewater Management, and Watershed Planning and Permitting). They apply to all federal agencies, state agencies, state authorities, municipal agencies and private businesses disposing of snow in the Commonwealth of Massachusetts.

INTRODUCTION

Finding a place to dispose of collected snow poses a challenge to municipalities and businesses as they clear roads, parking lots, bridges, and sidewalks. While MassDEP is aware of the threats to public safety caused by snow, collected snow that is contaminated with road salt, sand, litter, and automotive pollutants such as oil also threatens public health and the environment.

As snow melts, road salt, sand, litter, and other pollutants are transported into surface water or through the soil where they may eventually reach the groundwater. Road salt and other pollutants can contaminate water supplies and are toxic to aquatic life at certain levels. Sand washed into

waterbodies can create sand bars or fill in wetlands and ponds, impacting aquatic life, causing flooding, and affecting our use of these resources.

There are several steps that communities can take to minimize the impacts of snow disposal on public health and the environment. These steps will help communities avoid the costs of a contaminated water supply, degraded waterbodies, and flooding. Everything that occurs on the land has the potential to impact the Commonwealth's water resources. Given the authority of local government over the use of the land, municipal officials and staff have a critically important role to play in protecting our water resources.

The purpose of these guidelines is to help federal agencies, state agencies, state authorities, municipalities and businesses select, prepare, and maintain appropriate snow disposal sites before the snow begins to accumulate through the winter. Following these guidelines and obtaining the necessary approvals may also help municipalities in cases when seeking reimbursement for snow disposal costs from the Federal Emergency Management Agency is possible.

RECOMMENDED GUIDELINES

These snow disposal guidelines address: (1) site selection; (2) site preparation and maintenance; and (3) emergency snow disposal.

1. SITE SELECTION

The key to selecting effective snow disposal sites is to locate them adjacent to or on pervious surfaces in upland areas or upland locations on impervious surfaces away from water resources and drinking water wells. At these locations, the snow meltwater can filter into the soil, leaving behind sand and debris which can be removed in the spring. The following conditions should be followed:

- Within water supply Zone A and Zone II, avoid storage or disposal of snow and ice containing deicing chemicals that has been collected from streets located outside these zones. Municipalities may have a water supply protection land use control that prohibits the disposal of snow and ice containing deicing chemicals from outside the Zone A and Zone II, subject to the Massachusetts Drinking Water Regulations at 310 CMR 22.20C and 310 CMR 22.21(2).
- Avoid storage or disposal of snow or ice in Interim Wellhead Protection Areas (IWPA) of public water supply wells, and within 75 feet of a private well, where road salt may contaminate water supplies.
- Avoid dumping snow into any waterbody, including rivers, the ocean, reservoirs, ponds, or wetlands. In addition to water quality impacts and flooding, snow disposed of in open water can cause navigational hazards when it freezes into ice blocks.
- Avoid dumping snow on MassDEP-designated high and medium-yield aquifers where it may contaminate groundwater.
- Avoid dumping snow in sanitary landfills and gravel pits. Snow meltwater will create more contaminated leachate in landfills posing a greater risk to groundwater, and in gravel pits, there is little opportunity for pollutants to be filtered out of the meltwater because groundwater is close to the land surface.

- Avoid disposing of snow on top of storm drain catch basins or in stormwater drainage systems including detention basins, swales or ditches. Snow combined with sand and debris may block a stormwater drainage system, causing localized flooding. A high volume of sand, sediment, and litter released from melting snow also may be quickly transported through the system into surface water.

Recommended Site Selection Procedures

It is important that the municipal Department of Public Works or Highway Department, Conservation Commission, and Board of Health work together to select appropriate snow disposal sites. The following steps should be taken:

- Estimate how much snow disposal capacity may be needed for the season so that an adequate number of disposal sites can be selected and prepared.
- Identify sites that could potentially be used for snow disposal, such as municipal open space (e.g., parking lots or parks).
- Select sites located in upland locations that are not likely to impact sensitive environmental resources first.
- If more storage space is still needed, prioritize the sites with the least environmental impact (using the site selection criteria, and local or MassGIS maps as a guide).

Snow Disposal Mapping Assistance

MassDEP has an online mapping tool to assist in identifying possible locations to potentially dispose of snow. MassDEP encourages municipalities to use this tool to identify possible snow disposal options. The tool identifies wetland resource areas, public drinking water supplies and other sensitive locations where snow should not be disposed. The tool may be accessed through the Internet at the following web address: **https://maps.env.state.ma.us/dep/arcgis/js/templates/PSF/**.

2. SITE PREPARATION AND MAINTENANCE

In addition to carefully selecting disposal sites before the winter begins, it is important to prepare and maintain these sites to maximize their effectiveness. The following maintenance measures should be undertaken for all snow disposal sites:

- A silt fence or equivalent barrier should be placed securely on the downgradient side of the snow disposal site.
- Wherever possible maintain a 50-foot vegetated buffer between the disposal site and adjacent waterbodies to filter pollutants from the meltwater.
- Clear debris from the site prior to using the site for snow disposal.
- Clear debris from the site and properly dispose of it at the end of the snow season, and no later than May 15.

3. SNOW DISPOSAL APPROVALS

Proper snow disposal may be undertaken through one of the following approval procedures:

- Routine snow disposal – Minimal, if any, administrative review is required in these cases when upland and pervious snow disposal locations or upland locations on impervious surfaces that have functioning and maintained stormwater management systems have been identified, mapped, and used for snow disposal following ordinary snowfalls. Use of upland and pervious snow disposal sites avoids wetland resource areas and allows snow meltwater to recharge groundwater and will help filter pollutants, sand, and other debris. This process will address the majority of snow removal efforts until an entity exhausts all available upland snow disposal sites. The location and mapping of snow disposal sites will help facilitate each entity's routine snow management efforts.
- Emergency Certifications – If an entity demonstrates that there is no remaining capacity at upland snow disposal locations, local conservation commissions may issue an Emergency Certification under the Massachusetts Wetlands Protection regulations to authorize snow disposal in buffer zones to wetlands, certain open water areas, and certain wetland resource areas (i.e. within flood plains). Emergency Certifications can only be issued at the request of a public agency or by order of a public agency for the protection of the health or safety of citizens, and are limited to those activities necessary to abate the emergency. See 310 CMR 10.06(1)-(4). Use the following guidelines in these emergency situations:
    - Dispose of snow in open water with adequate flow and mixing to prevent ice dams from forming.
    - Do not dispose of snow in salt marshes, vegetated wetlands, certified vernal pools, shellfish beds, mudflats, drinking water reservoirs and their tributaries, Zone IIs or IWPAs of public water supply wells, Outstanding Resource Waters, or Areas of Critical Environmental Concern.
    - Do not dispose of snow where trucks may cause shoreline damage or erosion.
    - Consult with the municipal Conservation Commission to ensure that snow disposal in open water complies with local ordinances and bylaws.

- Severe Weather Emergency Declarations – In the event of a large-scale severe weather event, MassDEP may issue a broader Emergency Declaration under the Wetlands Protection Act which allows federal agencies, state agencies, state authorities, municipalities, and businesses greater flexibility in snow disposal practices. Emergency Declarations typically authorize greater snow disposal options while protecting especially sensitive resources such as public drinking water supplies, vernal pools, land containing shellfish, FEMA designated floodways, coastal dunes, and salt marsh. In the event of severe winter storm emergencies, the snow disposal site maps created by municipalities will enable MassDEP and the Massachusetts Emergency Management Agency (MEMA) in helping communities identify appropriate snow disposal locations.

  If upland disposal sites have been exhausted, the Emergency Declaration issued by MassDEP allows for snow disposal near water bodies. In these situations, a buffer of at

least 50 feet, preferably vegetated, should still be maintained between the site and the waterbody. Furthermore, it is essential that the other guidelines for preparing and maintaining snow disposal sites be followed to minimize the threat to adjacent waterbodies.

Under extraordinary conditions, when all land-based snow disposal options are exhausted, the Emergency Declaration issued by MassDEP may allow disposal of snow in certain waterbodies under certain conditions. ***A federal agency, state agency, state authority, municipality or business seeking to dispose of snow in a waterbody should take the following steps***:

- Call the emergency contact phone number [(888) 304-1133)] and notify the MEMA of the municipality's intent.
- MEMA will ask for some information about where the requested disposal will take place.
- MEMA will confirm that the disposal is consistent with MassDEP's Severe Weather Emergency Declaration and these guidelines and is therefore approved.

During declared statewide snow emergency events, MassDEP's website will also highlight the emergency contact phone number [(888) 304-1133)] for authorizations and inquiries. For further non-emergency information about this Guidance you may contact your MassDEP Regional Office Service Center:

Northeast Regional Office, Wilmington, 978-694-3246
Southeast Regional Office, Lakeville, 508-946-2714
Central Regional Office, Worcester, 508-792-7650
Western Regional Office, Springfield, 413-755-2114

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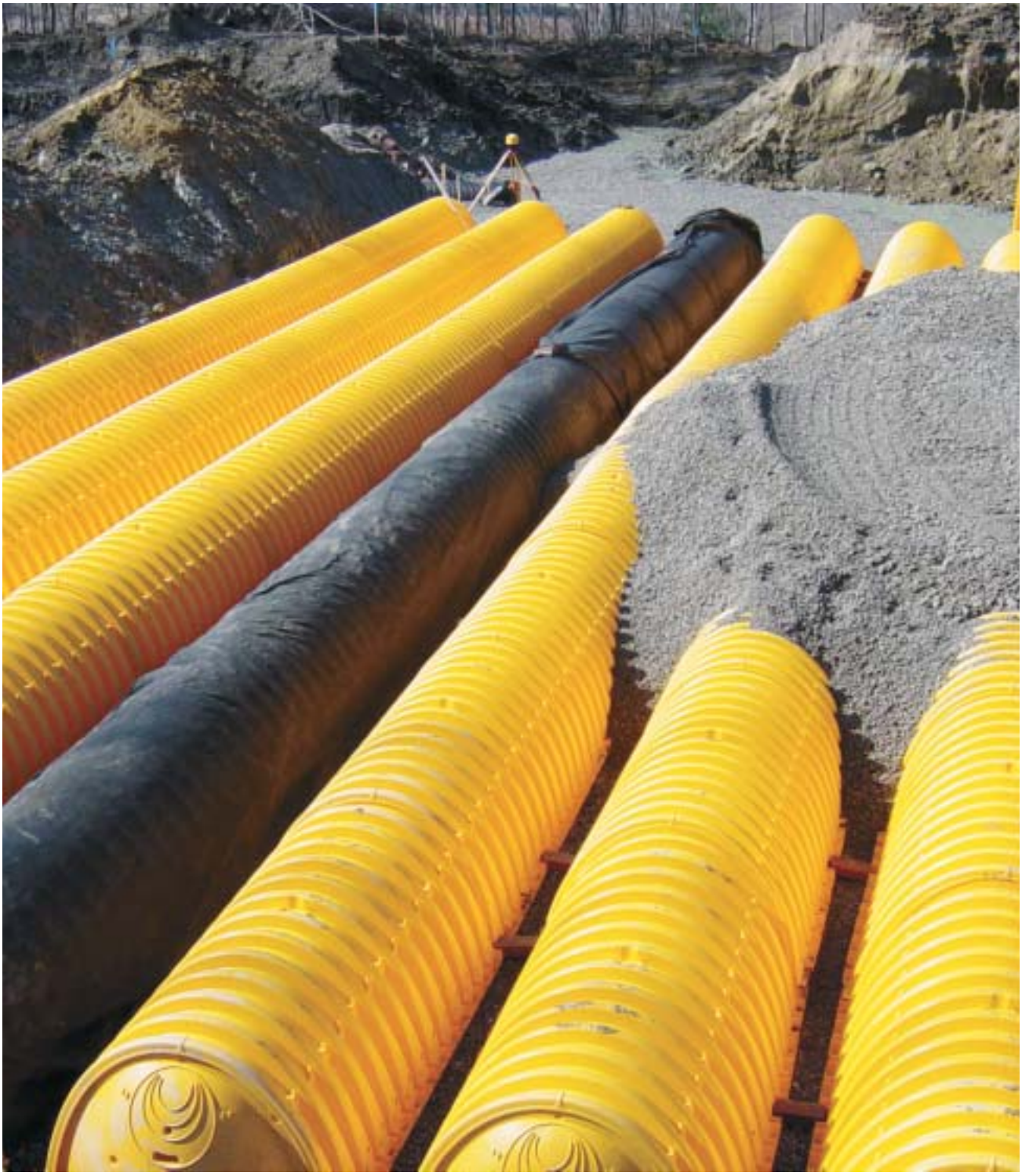


Isolator™ Row O&M Manual

StormTech® Chamber System for Stormwater Management

1.0 The Isolator™ Row

1.1 INTRODUCTION

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row is a patent pending technique to inexpensively enhance Total Suspended Solids (TSS) removal and provide easy access for inspection and maintenance.

Looking down the Isolator Row from the manhole opening, woven geotextile is shown between the chamber and stone base.

1.2 THE ISOLATOR™ ROW

The Isolator Row is a row of StormTech chambers, either SC-740 or SC-310 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for settling and filtration of sediment as storm water rises in the Isolator Row and ultimately passes through the filter fabric. The open bottom chambers and perforated sidewalls allow storm water to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row protecting the storage areas of the adjacent stone and chambers from sediment accumulation.

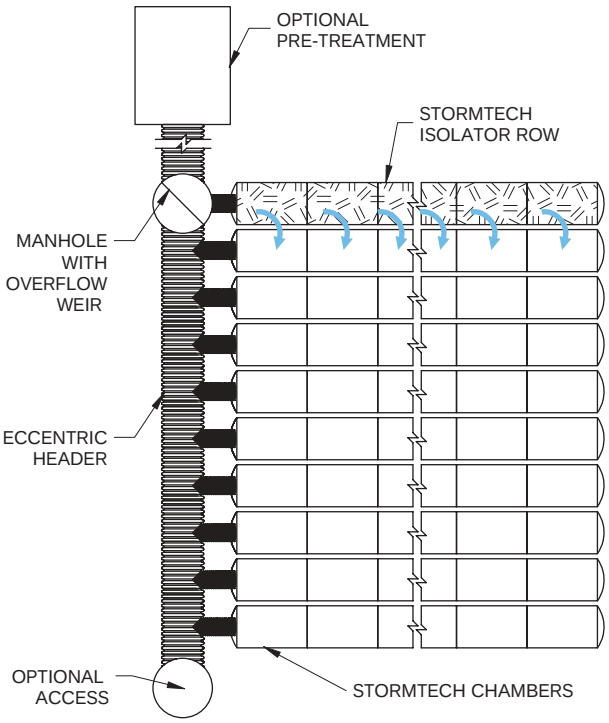
Two different fabrics are used for the Isolator Row. A woven geotextile fabric is placed between the stone and the Isolator Row chambers. The tough geotextile provides a media for storm water filtration and provides a durable surface for maintenance operations. It is also designed to prevent scour of the underlying stone and remain intact during high pressure jetting. A non-woven fabric is placed over the chambers to provide a filter media for flows passing through the perforations in the sidewall of the chamber.

The Isolator Row is typically designed to capture the "first flush" and offers the versatility to be sized on a volume basis or flow rate basis. An upstream manhole not only provides access to the Isolator Row but typically includes a high flow weir such that storm water flowrates or volumes that exceed the capacity of the Isolator Row overtop the over flow weir and discharge through a manifold to the other chambers.

The Isolator Row may also be part of a treatment train. By treating storm water prior to entry into the chamber system, the service life can be extended and pollutants such as hydrocarbons can be captured. Pre-treatment best management practices can be as simple as deep sump catch basins, oil-water separators or can be innovative storm water treatment devices. The design of the treatment train and selection of pretreatment devices by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, the Isolator Row is recommended by StormTech as an effective means to minimize maintenance requirements and maintenance costs.

Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row.

StormTech Isolator Row with Overflow Spillway
(not to scale)



2.0 Isolator Row Inspection/Maintenance

2.1 INSPECTION

The frequency of Inspection and Maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

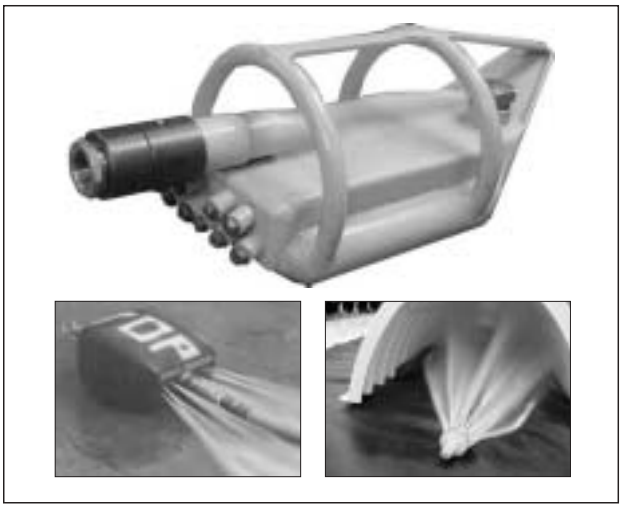
At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

The Isolator Row incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row, clean-out should be performed.

2.2 MAINTENANCE

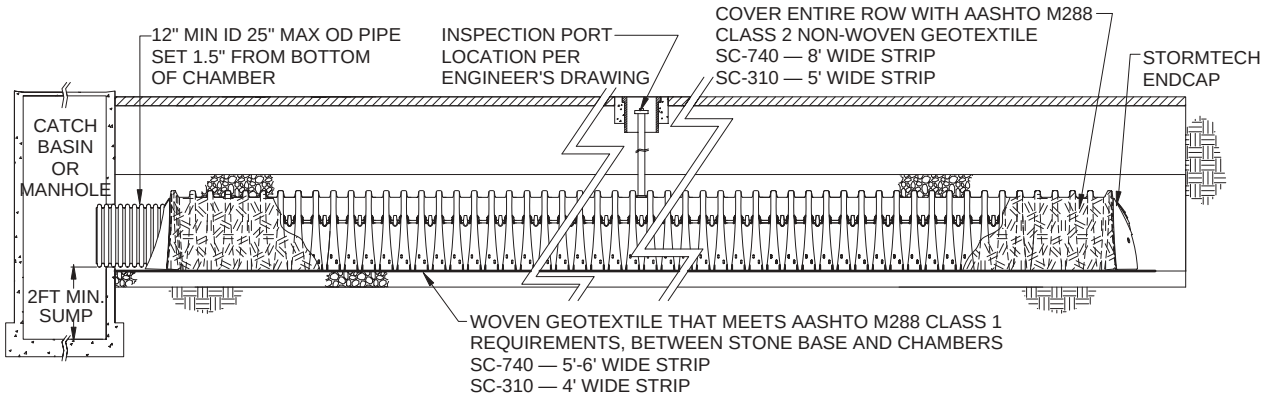
The Isolator Row was designed to reduce the cost of periodic maintenance. By “isolating” sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.



Examples of culvert cleaning nozzles appropriate for Isolator Row maintenance. (These are not StormTech products.)

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45” are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. **The JetVac process shall only be performed on StormTech Isolator Rows that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.**

StormTech Isolator Row (not to scale)



3.0 Isolator Row Step By Step Maintenance Procedures

Step 1) Inspect Isolator Row for sediment

A) Inspection ports (if present)

i. Remove lid from floor box frame

ii. Remove cap from inspection riser

iii. Using a flashlight and stadia rod, measure depth of sediment and record results on maintenance log.

iv. If sediment is at, or above, 3 inch depth proceed to Step 2. If not proceed to step 3.

B) All Isolator Rows

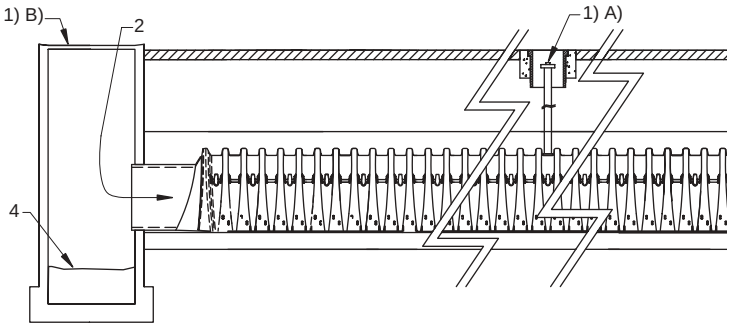
i. Remove cover from manhole at upstream end of Isolator Row

ii. Using a flashlight, inspect down Isolator Row through outlet pipe

1. Mirrors on poles or cameras may be used to avoid a confined space entry
2. Follow OSHA regulations for confined space entry if entering manhole

iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches) proceed to Step 2. If not proceed to Step 3.

StormTech Isolator Row (not to scale)



Step 2) Clean out Isolator Row using the JetVac process

A) A fixed culvert cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable

B) Apply multiple passes of JetVac until backflush water is clean

C) Vacuum manhole sump as required

Step 3) Replace all caps, lids and covers, record observations and actions

Step 4) Inspect & clean catch basins and manholes upstream of the StormTech system

Sample Maintenance Log

Date	Stadia Rod Readings		Sediment Depth (1) - (2)	Observations/Actions	Inspector
	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)			
3/15/01	6.3 ft.	none		New installation. Fixed point is CI frame at grade	djm
9/24/01		6.2	0.1 ft.	Some grit felt	sm
6/20/03		5.8	0.5 ft.	Mucky feel, debris visible in manhole and in Isolator row, maintenance due	rv
7/7/03	6.3 ft.		0	System jetted and vacuumed	djm

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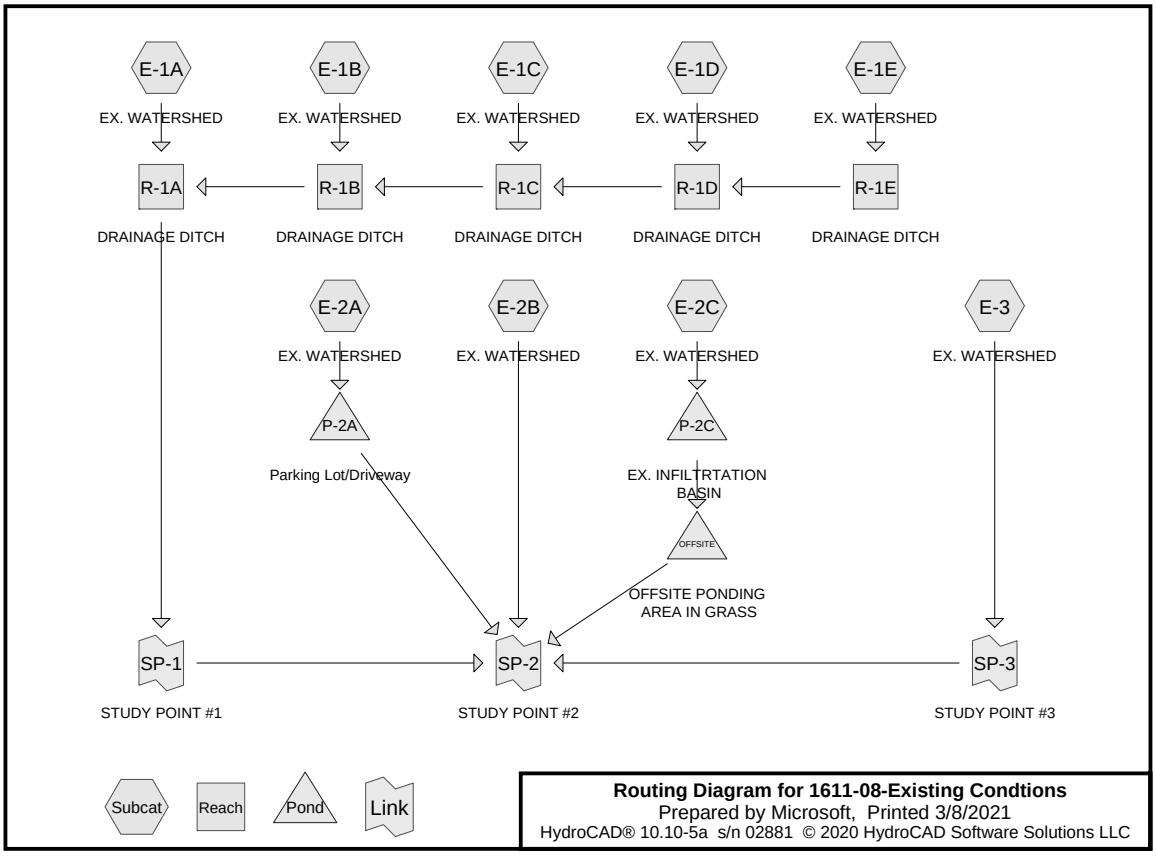
SECTION 3.0 -
EXISTING DRAINAGE ANALYSIS



Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	Type III 24-hr		Default	24.00	1	3.10	2
2	10-Year	Type III 24-hr		Default	24.00	1	4.50	2
3	100-Year	Type III 24-hr		Default	24.00	1	6.50	2

Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers
0	0	31,336	0	0	31,336	<50% Grass cover, Poor	E-1A, E-1B, E-1C, E-1D, E-1E, E-2A, E-2B, E-2C
0	0	128,745	0	0	128,745	>75% Grass cover, Good	E-1A, E-1B, E-1C, E-1D, E-1E, E-2B, E-2C
0	0	15,843	0	0	15,843	Gravel surface	E-1E
0	0	0	0	425,412	425,412	Impervious	E-1A, E-1B, E-1C, E-1D, E-1E, E-2A, E-2B, E-2C, E-3
0	0	159,797	237,023	0	396,820	Woods, Good	E-3
0	0	0	46,239	0	46,239	Woods/grass comb., Good	E-1A, E-1B, E-1C, E-1D
0	**0**	**335,721**	**283,262**	**425,412**	**1,044,395**	**TOTAL AREA**	

Summary for Subcatchment E-1A: EX. WATERSHED

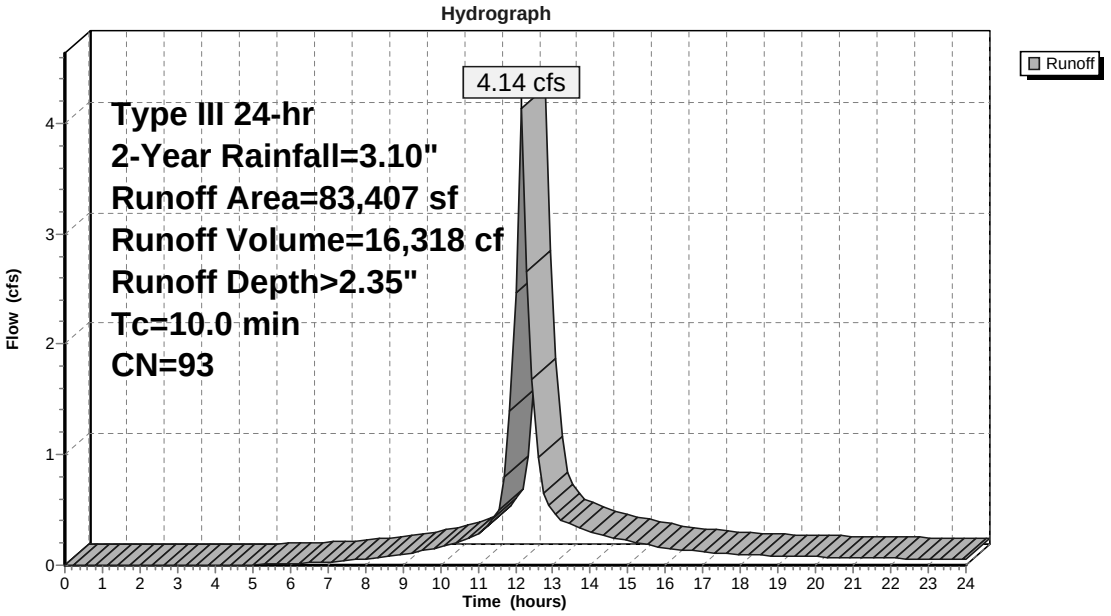
Runoff = 4.14 cfs @ 12.15 hrs, Volume= 16,318 cf, Depth> 2.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	6,343	86	<50% Grass cover, Poor, HSG C
	8,692	74	>75% Grass cover, Good, HSG C
	5,285	79	Woods/grass comb., Good, HSG D
*	63,087	98	Impervious
	83,407	93	Weighted Average
	20,320		24.36% Pervious Area
	63,087		75.64% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1A: EX. WATERSHED



Summary for Subcatchment E-1B: EX. WATERSHED

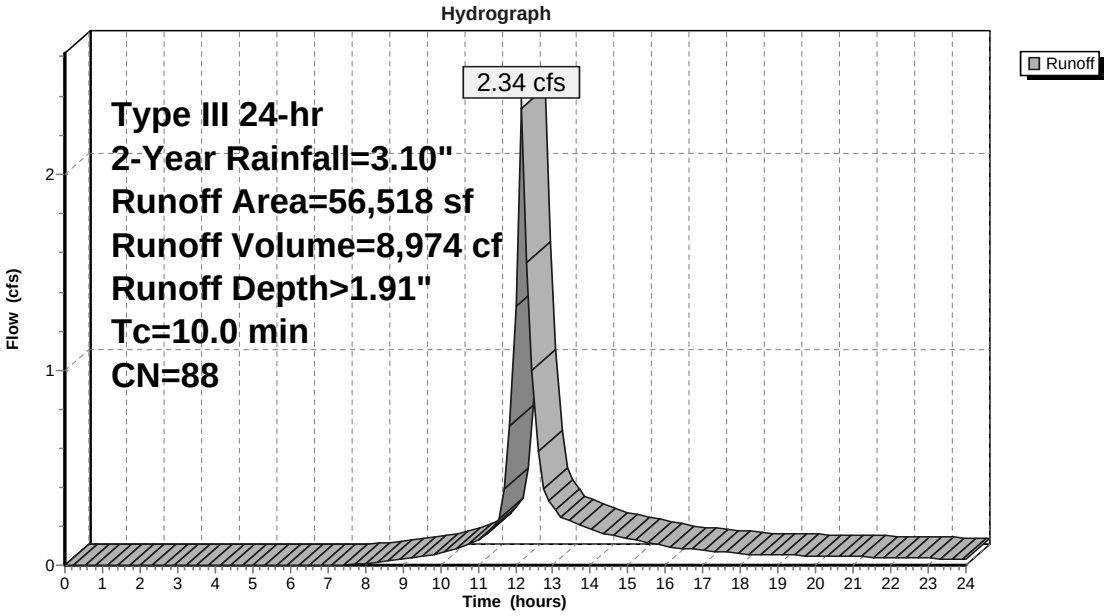
Runoff = 2.34 cfs @ 12.16 hrs, Volume= 8,974 cf, Depth> 1.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	3,722	86	<50% Grass cover, Poor, HSG C
	8,826	74	>75% Grass cover, Good, HSG C
	14,964	79	Woods/grass comb., Good, HSG D
*	29,006	98	Impervious
	56,518	88	Weighted Average
	27,512		48.68% Pervious Area
	29,006		51.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1B: EX. WATERSHED



Summary for Subcatchment E-1C: EX. WATERSHED

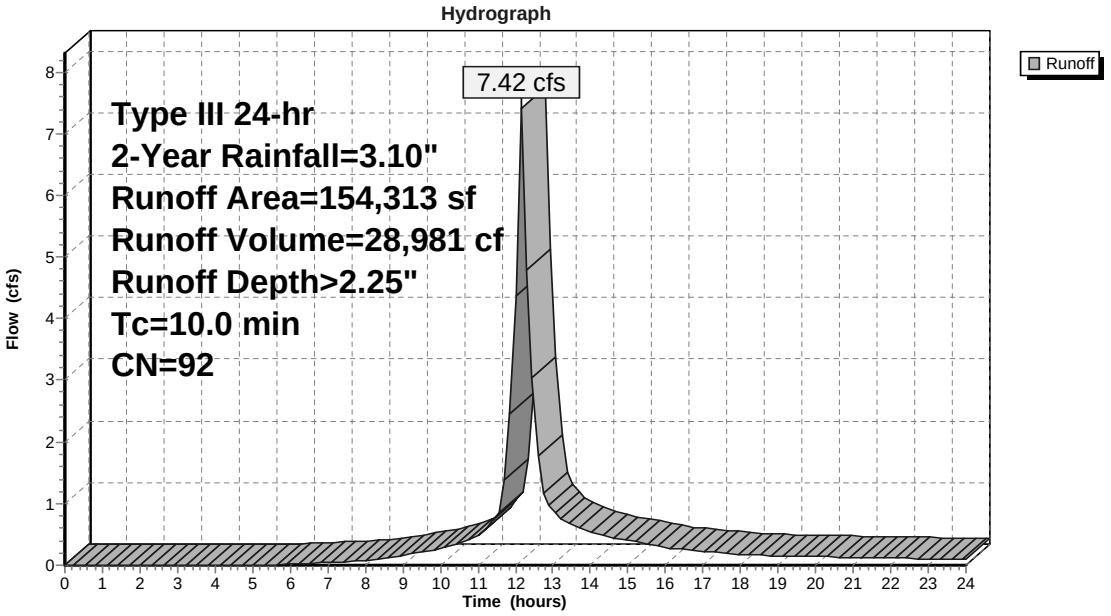
Runoff = 7.42 cfs @ 12.16 hrs, Volume= 28,981 cf, Depth> 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	3,840	86	<50% Grass cover, Poor, HSG C
	20,360	74	>75% Grass cover, Good, HSG C
	23,421	79	Woods/grass comb., Good, HSG D
*	106,692	98	Impervious
	154,313	92	Weighted Average
	47,621		30.86% Pervious Area
	106,692		69.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1C: EX. WATERSHED



Summary for Subcatchment E-1D: EX. WATERSHED

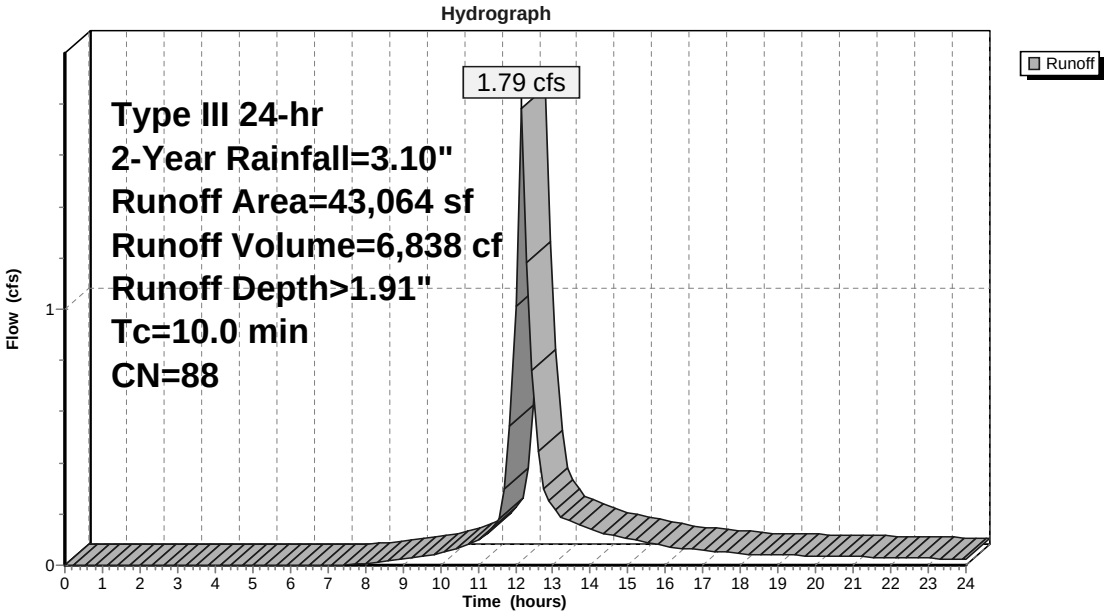
Runoff = 1.79 cfs @ 12.16 hrs, Volume= 6,838 cf, Depth> 1.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	5,705	86	<50% Grass cover, Poor, HSG C
	12,809	74	>75% Grass cover, Good, HSG C
	2,569	79	Woods/grass comb., Good, HSG D
*	21,981	98	Impervious
	43,064	88	Weighted Average
	21,083		48.96% Pervious Area
	21,981		51.04% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1D: EX. WATERSHED



Summary for Subcatchment E-1E: EX. WATERSHED

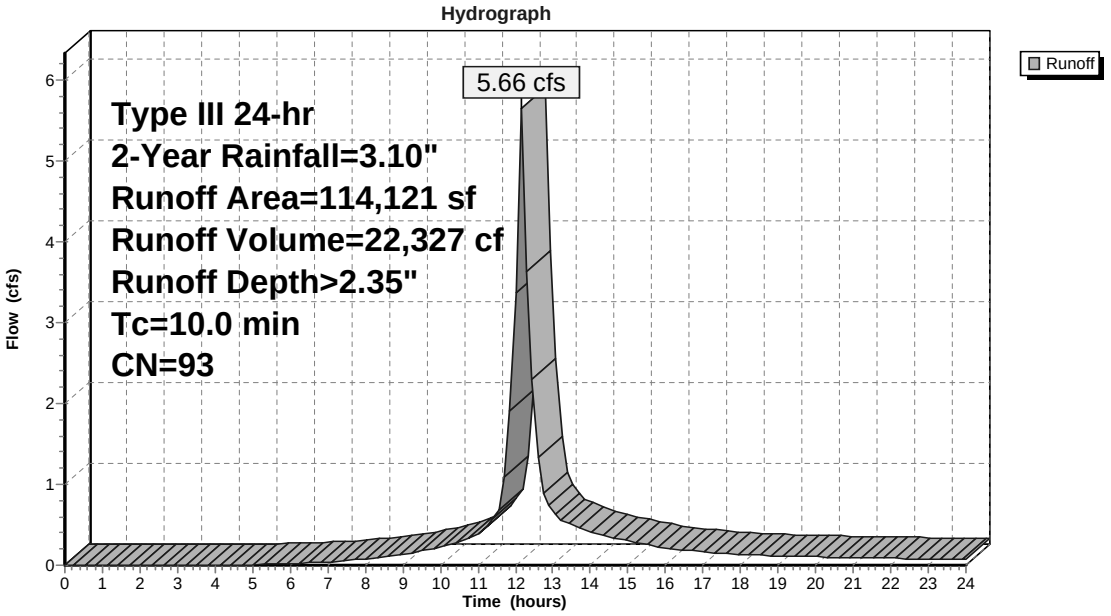
Runoff = 5.66 cfs @ 12.15 hrs, Volume= 22,327 cf, Depth> 2.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	6,384	86	<50% Grass cover, Poor, HSG C
	17,883	74	>75% Grass cover, Good, HSG C
	15,843	96	Gravel surface, HSG C
*	74,011	98	Impervious
	114,121	93	Weighted Average
	40,110		35.15% Pervious Area
	74,011		64.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1E: EX. WATERSHED



Summary for Subcatchment E-2A: EX. WATERSHED

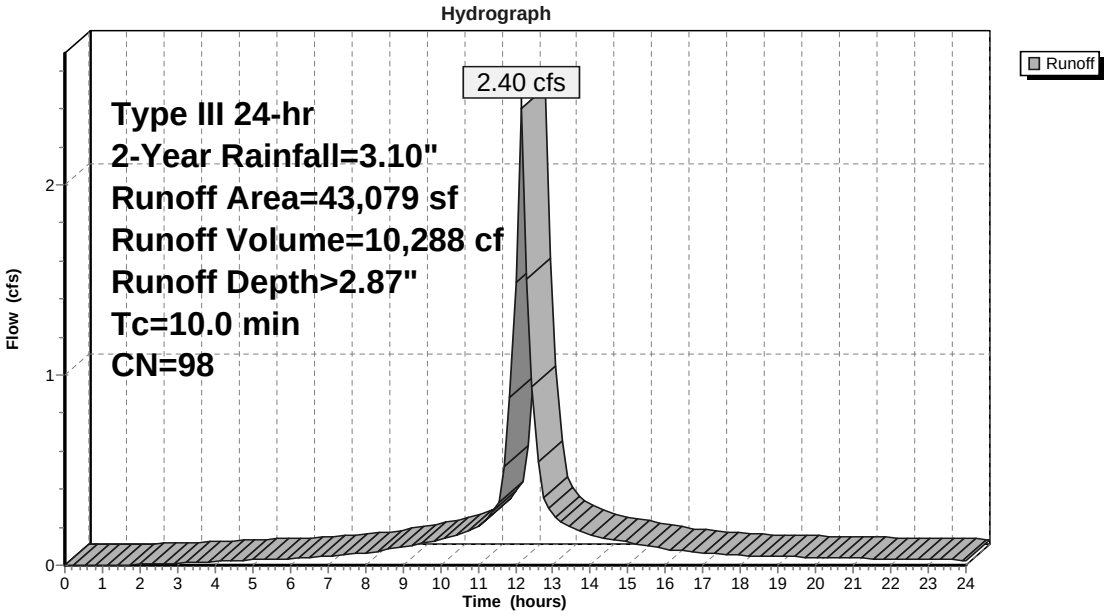
Runoff = 2.40 cfs @ 12.15 hrs, Volume= 10,288 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	1,483	86	<50% Grass cover, Poor, HSG C
*	41,596	98	Impervious
	43,079	98	Weighted Average
	1,483		3.44% Pervious Area
	41,596		96.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2A: EX. WATERSHED



Summary for Subcatchment E-2B: EX. WATERSHED

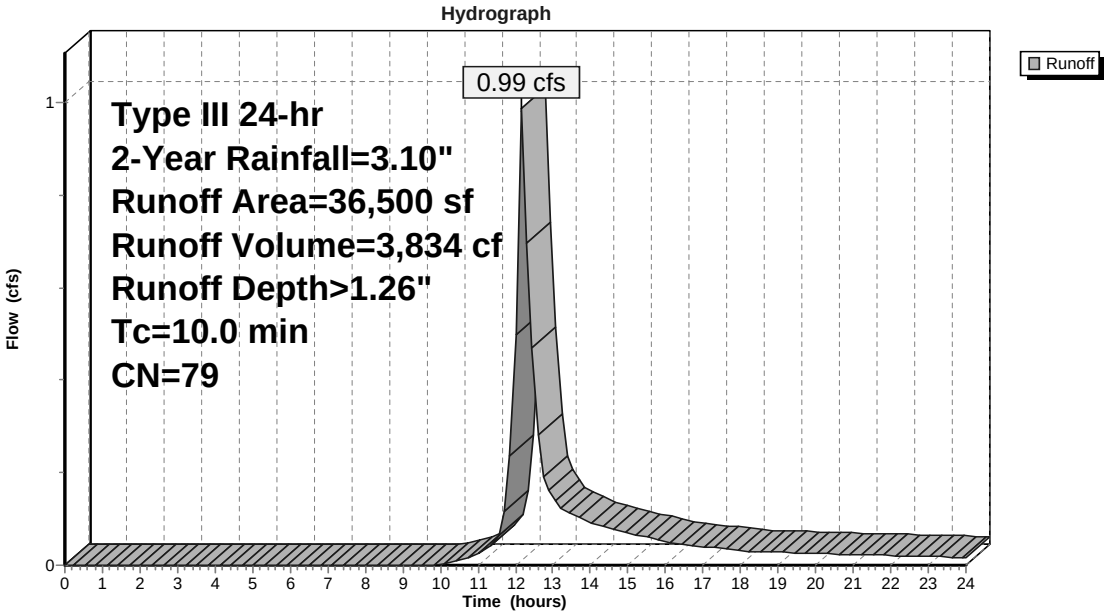
Runoff = 0.99 cfs @ 12.17 hrs, Volume= 3,834 cf, Depth> 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	1,043	86	<50% Grass cover, Poor, HSG C
	28,476	74	>75% Grass cover, Good, HSG C
*	6,981	98	Impervious
	36,500	79	Weighted Average
	29,519		80.87% Pervious Area
	6,981		19.13% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2B: EX. WATERSHED



Summary for Subcatchment E-2C: EX. WATERSHED

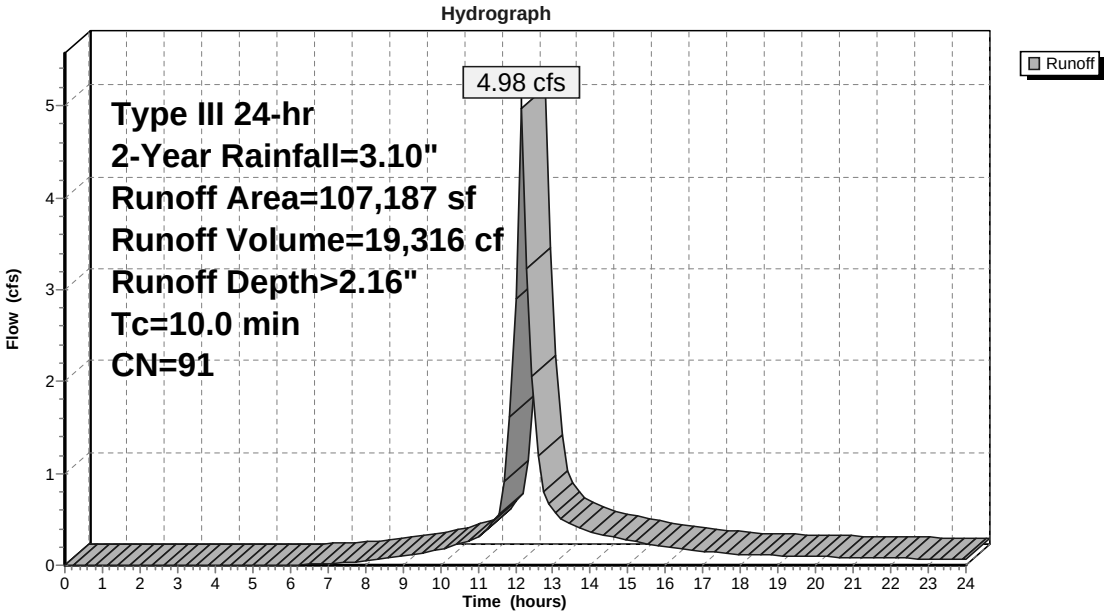
Runoff = 4.98 cfs @ 12.16 hrs, Volume= 19,316 cf, Depth> 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	2,816	86	<50% Grass cover, Poor, HSG C
	31,699	74	>75% Grass cover, Good, HSG C
*	72,672	98	Impervious
	107,187	91	Weighted Average
	34,515		32.20% Pervious Area
	72,672		67.80% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2C: EX. WATERSHED



Summary for Subcatchment E-3: EX. WATERSHED

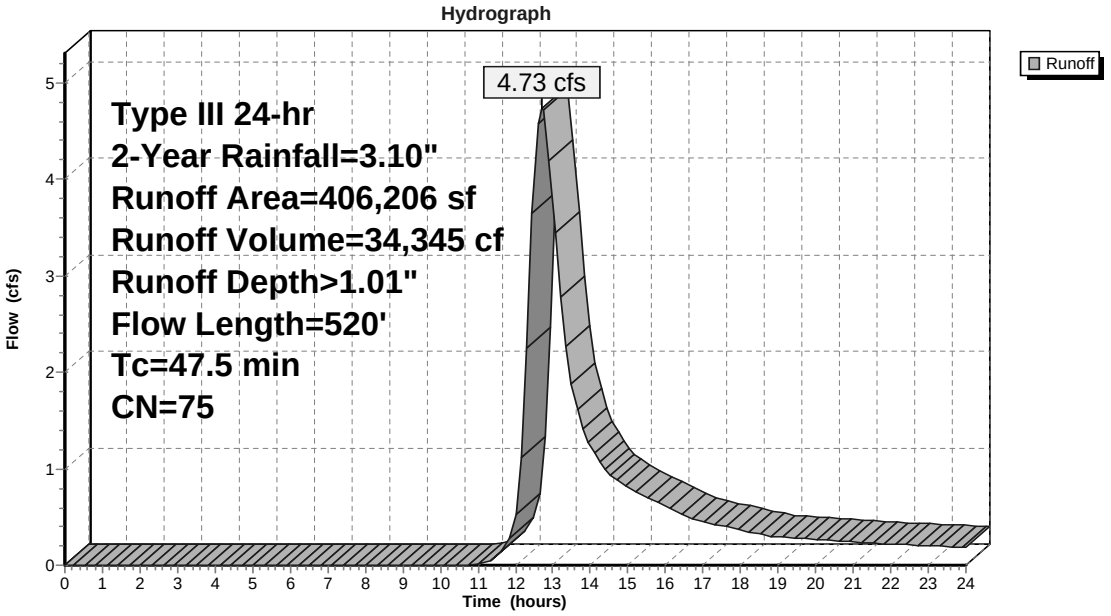
Runoff = 4.73 cfs @ 12.71 hrs, Volume= 34,345 cf, Depth> 1.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	237,023	77	Woods, Good, HSG D
*	9,386	98	Impervious
	159,797	70	Woods, Good, HSG C
	406,206	75	Weighted Average
	396,820		97.69% Pervious Area
	9,386		2.31% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	100	0.0100	0.06		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
1.3	40	0.0100	0.50		**Shallow Concentrated Flow, B-C** Woodland Kv= 5.0 fps
17.9	380	0.0050	0.35		**Shallow Concentrated Flow, C-D** Woodland Kv= 5.0 fps
47.5	520	Total			

Subcatchment E-3: EX. WATERSHED



Summary for Reach R-1A: DRAINAGE DITCH

Inflow Area = 451,423 sf, 65.30% Impervious, Inflow Depth > 2.19" for 2-Year event
Inflow = 12.53 cfs @ 12.39 hrs, Volume= 82,303 cf
Outflow = 12.45 cfs @ 12.38 hrs, Volume= 82,247 cf, Atten= 1%, Lag= 0.0 min

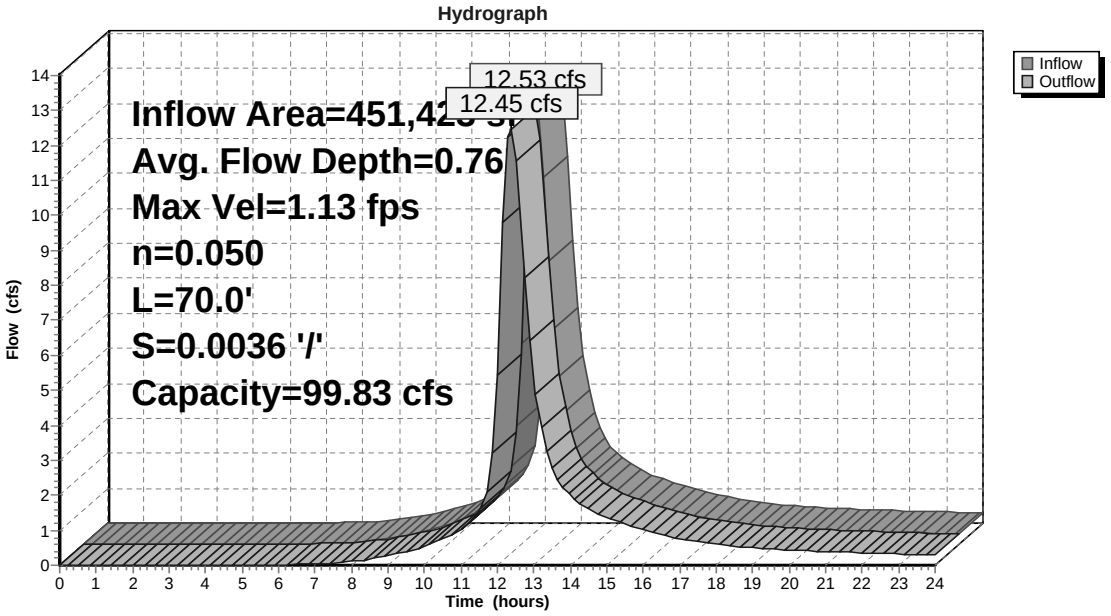
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 1.13 fps, Min. Travel Time= 1.0 min
Avg. Velocity = 0.44 fps, Avg. Travel Time= 2.7 min

Peak Storage= 771 cf @ 12.38 hrs
Average Depth at Peak Storage= 0.76' , Surface Width= 21.63'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 99.83 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 70.0' Slope= 0.0036 '/'
Inlet Invert= 78.75', Outlet Invert= 78.50'

‡

Reach R-1A: DRAINAGE DITCH



Summary for Reach R-1B: DRAINAGE DITCH

Inflow Area = 368,016 sf, 62.96% Impervious, Inflow Depth > 2.17" for 2-Year event
Inflow = 11.45 cfs @ 12.33 hrs, Volume= 66,402 cf
Outflow = 10.68 cfs @ 12.45 hrs, Volume= 65,985 cf, Atten= 7%, Lag= 7.4 min

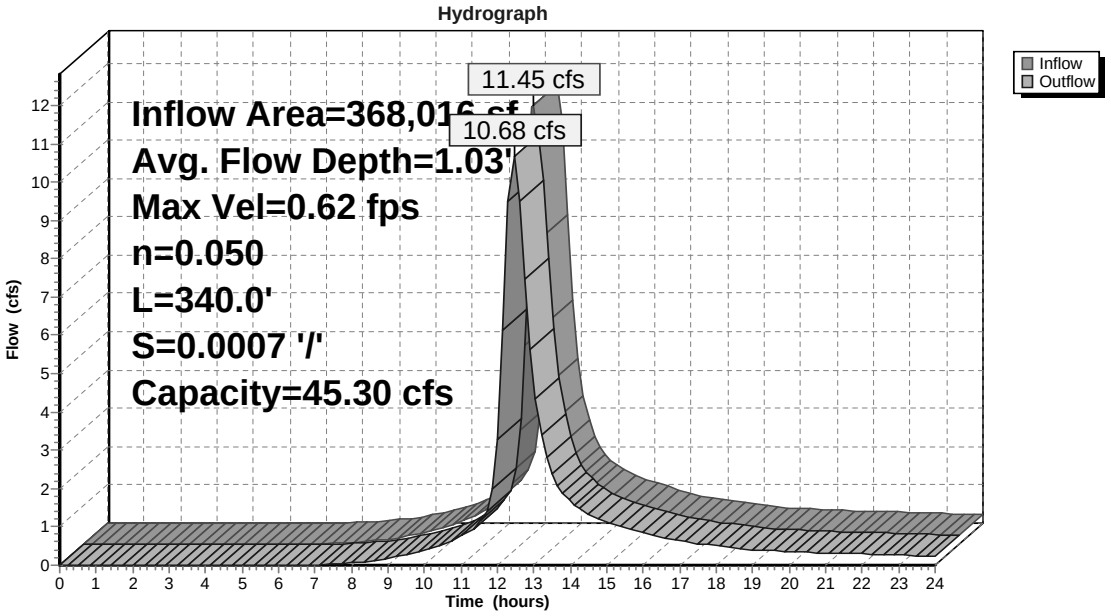
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 0.62 fps, Min. Travel Time= 9.1 min
Avg. Velocity = 0.24 fps, Avg. Travel Time= 23.6 min

Peak Storage= 5,824 cf @ 12.45 hrs
Average Depth at Peak Storage= 1.03' , Surface Width= 25.06'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 45.30 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 340.0' Slope= 0.0007 '/'
Inlet Invert= 79.00', Outlet Invert= 78.75'

‡

Reach R-1B: DRAINAGE DITCH



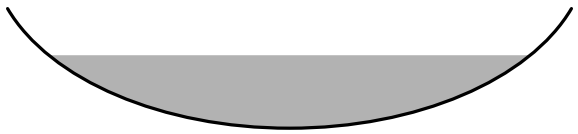
Summary for Reach R-1C: DRAINAGE DITCH

Inflow Area = 311,498 sf, 65.07% Impervious, Inflow Depth > 2.23" for 2-Year event
Inflow = 12.21 cfs @ 12.20 hrs, Volume= 57,910 cf
Outflow = 10.07 cfs @ 12.36 hrs, Volume= 57,428 cf, Atten= 17%, Lag= 9.6 min

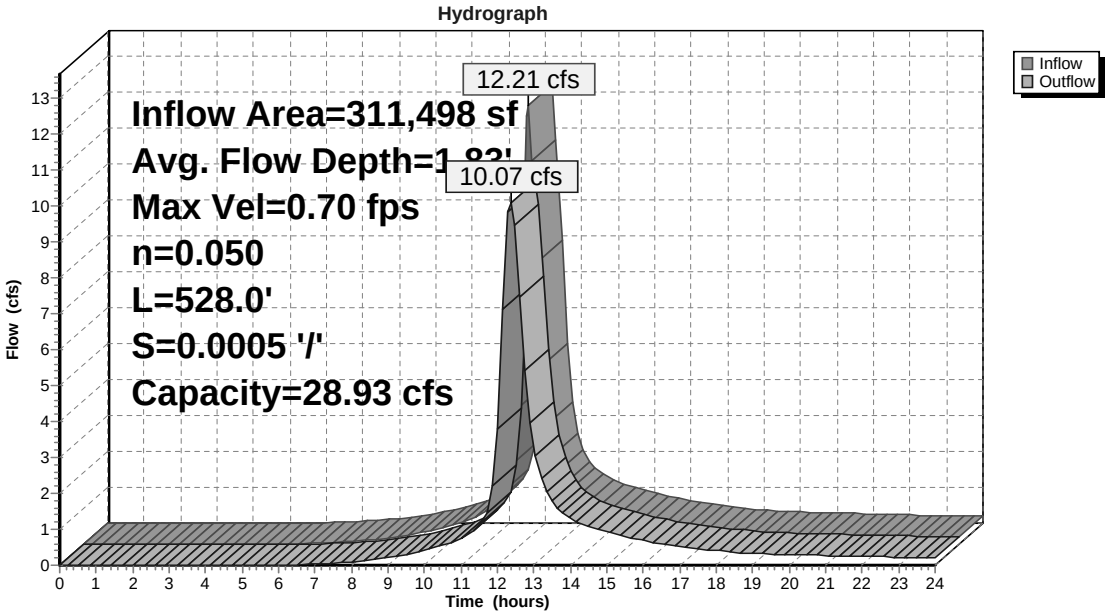
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 0.70 fps, Min. Travel Time= 12.5 min
Avg. Velocity = 0.27 fps, Avg. Travel Time= 32.3 min

Peak Storage= 7,518 cf @ 12.36 hrs
Average Depth at Peak Storage= 1.83' , Surface Width= 11.70'
Bank-Full Depth= 3.00' Flow Area= 30.0 sf, Capacity= 28.93 cfs

15.00' x 3.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0005 '/'
Inlet Invert= 79.25', Outlet Invert= 79.00'



Reach R-1C: DRAINAGE DITCH



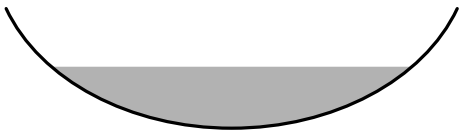
Summary for Reach R-1D: DRAINAGE DITCH

Inflow Area = 157,185 sf, 61.07% Impervious, Inflow Depth > 2.22" for 2-Year event
Inflow = 6.79 cfs @ 12.19 hrs, Volume= 29,112 cf
Outflow = 5.88 cfs @ 12.31 hrs, Volume= 28,929 cf, Atten= 13%, Lag= 7.3 min

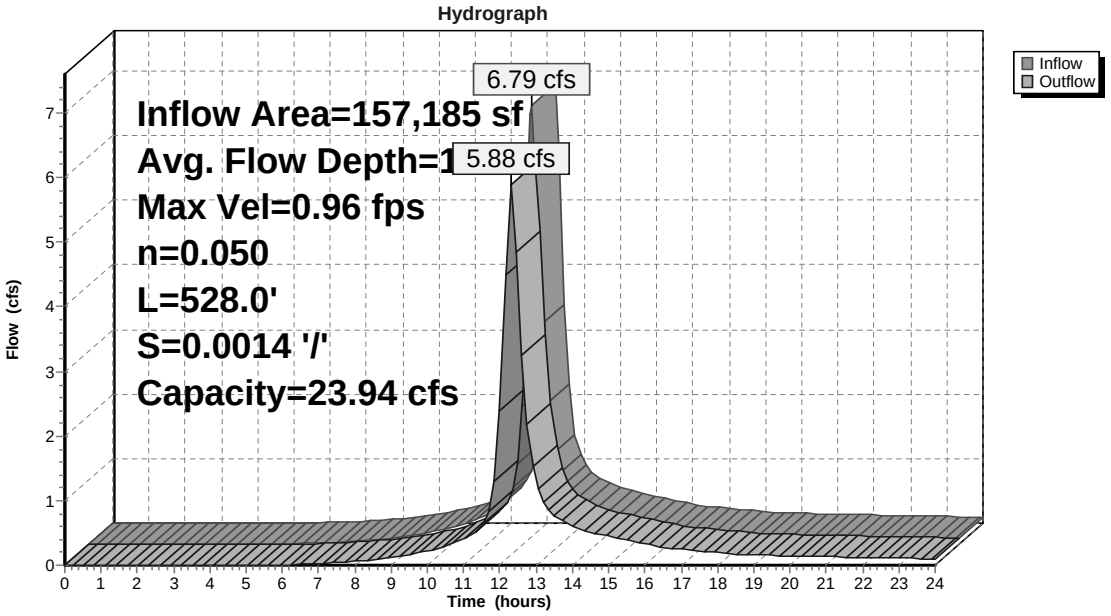
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 0.96 fps, Min. Travel Time= 9.2 min
Avg. Velocity = 0.35 fps, Avg. Travel Time= 24.9 min

Peak Storage= 3,238 cf @ 12.31 hrs
Average Depth at Peak Storage= 1.28' , Surface Width= 7.17'
Bank-Full Depth= 2.50' Flow Area= 16.7 sf, Capacity= 23.94 cfs

10.00' x 2.50' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0014 '/'
Inlet Invert= 80.00', Outlet Invert= 79.25'



Reach R-1D: DRAINAGE DITCH



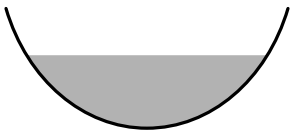
Summary for Reach R-1E: DRAINAGE DITCH

Inflow Area = 114,121 sf, 64.85% Impervious, Inflow Depth > 2.35" for 2-Year event
Inflow = 5.66 cfs @ 12.15 hrs, Volume= 22,327 cf
Outflow = 5.05 cfs @ 12.21 hrs, Volume= 22,274 cf, Atten= 11%, Lag= 3.1 min

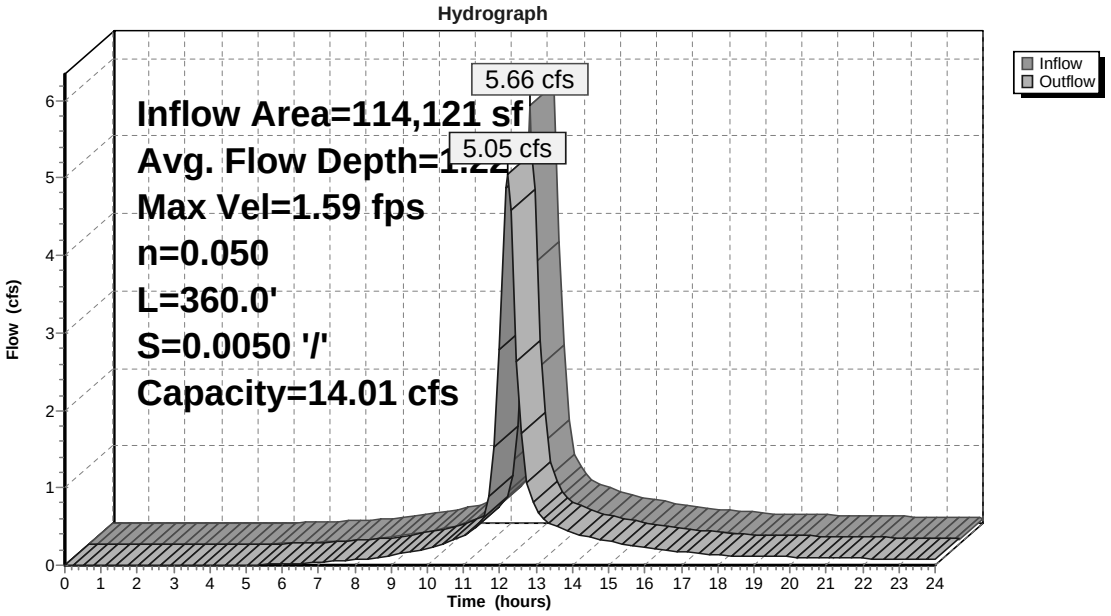
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 1.59 fps, Min. Travel Time= 3.8 min
Avg. Velocity = 0.60 fps, Avg. Travel Time= 10.0 min

Peak Storage= 1,139 cf @ 12.21 hrs
Average Depth at Peak Storage= 1.22' , Surface Width= 3.90'
Bank-Full Depth= 2.00' Flow Area= 6.7 sf, Capacity= 14.01 cfs

5.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 360.0' Slope= 0.0050 '/'
Inlet Invert= 81.80', Outlet Invert= 80.00'



Reach R-1E: DRAINAGE DITCH



Summary for Pond OFFSITE: OFFSITE PONDING AREA IN GRASS

Inflow Area = 107,187 sf, 67.80% Impervious, Inflow Depth > 1.92" for 2-Year event
Inflow = 4.61 cfs @ 12.19 hrs, Volume= 17,168 cf
Outflow = 4.50 cfs @ 12.21 hrs, Volume= 16,982 cf, Atten= 2%, Lag= 1.2 min
Primary = 4.50 cfs @ 12.21 hrs, Volume= 16,982 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 82.65' @ 12.21 hrs Surf.Area= 3,166 sf Storage= 941 cf

Plug-Flow detention time= 14.1 min calculated for 16,877 cf (98% of inflow)
Center-of-Mass det. time= 7.9 min (843.4 - 835.6)

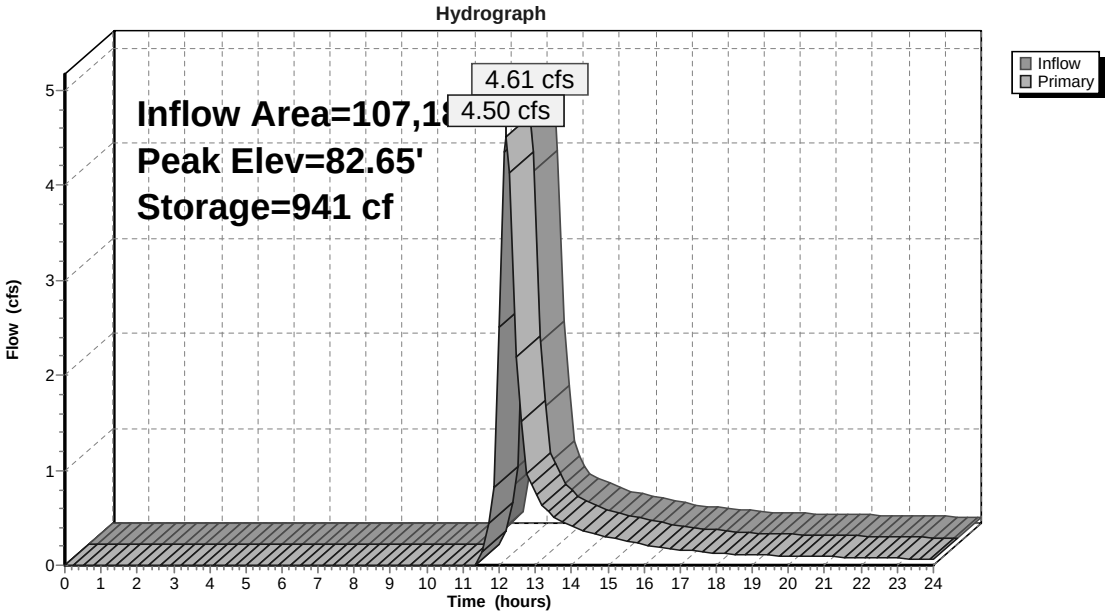
Volume	Invert	Avail.Storage	Storage Description
#1	82.10'	2,354 cf	**OFFSITE PONDING AREA (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.10	230	0	0
83.00	5,000	2,354	2,354

Device	Routing	Invert	Outlet Devices
#1	Primary	82.27'	**8.0" Round (3) 8" HDPE X 3.00 w/ 2.0" inside fill** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.10' / 81.40' S= 0.0333 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.28 sf
#2	Primary	82.60'	**100.0' long x 20.0' breadth WEIR FLOW OVER WALKING PATH** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=4.26 cfs @ 12.21 hrs HW=82.65' TW=0.00' (Dynamic Tailwater)
↑**1=(3) 8" HDPE** (Inlet Controls 1.15 cfs @ 1.59 fps)
└**2=WEIR FLOW OVER WALKING PATH** (Weir Controls 3.11 cfs @ 0.61 fps)

Pond OFFSITE: OFFSITE PONDING AREA IN GRASS



Summary for Pond P-2A: Parking Lot/Driveway

Inflow Area = 43,079 sf, 96.56% Impervious, Inflow Depth > 2.87" for 2-Year event
Inflow = 2.40 cfs @ 12.15 hrs, Volume= 10,288 cf
Outflow = 2.29 cfs @ 12.15 hrs, Volume= 10,286 cf, Atten= 5%, Lag= 0.1 min
Primary = 2.29 cfs @ 12.15 hrs, Volume= 10,286 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 84.03' @ 12.15 hrs Surf.Area= 491 sf Storage= 52 cf

Plug-Flow detention time= 0.6 min calculated for 10,222 cf (99% of inflow)
Center-of-Mass det. time= 0.5 min (760.7 - 760.3)

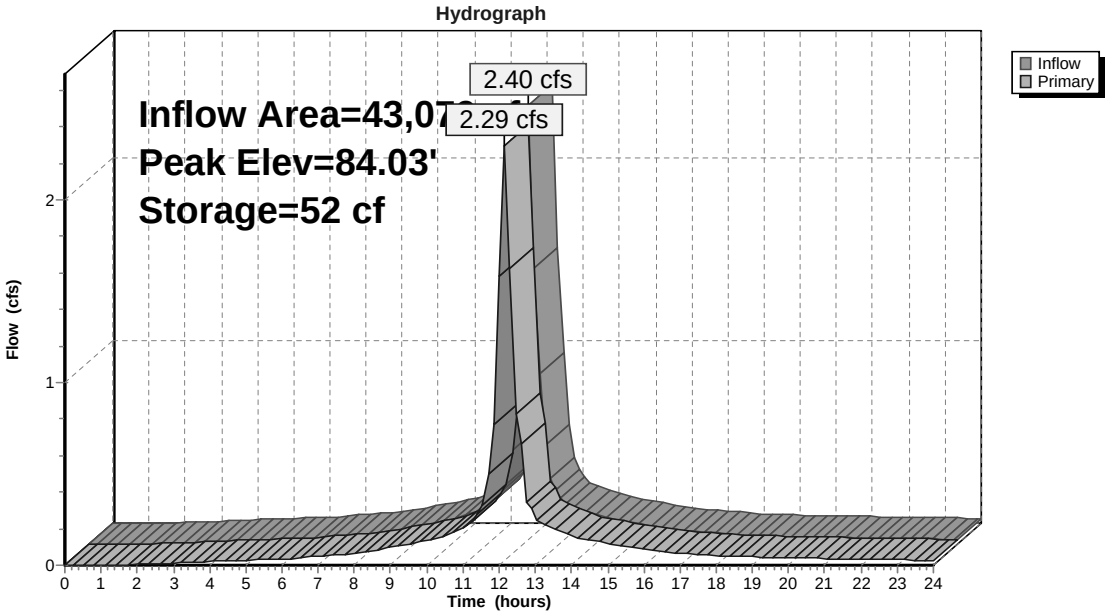
Volume	Invert	Avail.Storage	Storage Description
#1	82.38'	14 cf	**4.00'D x 1.10'H Ex.CB**
#2	83.45'	1,680 cf	**Parking/Driveway (Prismatic)**Listed below (Recalc)
		1,694 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.45	4	0	0
84.00	103	29	29
84.50	6,500	1,651	1,680

Device	Routing	Invert	Outlet Devices
#1	Primary	82.46'	**6.0" Round 6"PVC w/ 1.0" inside fill** L= 170.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.38' / 81.00' S= 0.0081 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.17 sf
#2	Primary	84.00'	**120.0' long x 50.0' breadth Weir Flow Over Curb Towards Lake** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=2.28 cfs @ 12.15 hrs HW=84.03' TW=0.00' (Dynamic Tailwater)
1=6"PVC (Barrel Controls 0.68 cfs @ 3.88 fps)
2=Weir Flow Over Curb Towards Lake (Weir Controls 1.60 cfs @ 0.46 fps)

Pond P-2A: Parking Lot/Driveway



Summary for Pond P-2C: EX. INFILTRTATION BASIN

Inflow Area = 107,187 sf, 67.80% Impervious, Inflow Depth > 2.16" for 2-Year event
Inflow = 4.98 cfs @ 12.16 hrs, Volume= 19,316 cf
Outflow = 4.61 cfs @ 12.19 hrs, Volume= 17,168 cf, Atten= 7%, Lag= 2.0 min
Primary = 4.61 cfs @ 12.19 hrs, Volume= 17,168 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 83.41' @ 12.19 hrs Surf.Area= 6,615 sf Storage= 2,764 cf

Plug-Flow detention time= 78.1 min calculated for 17,062 cf (88% of inflow)
Center-of-Mass det. time= 28.8 min (835.6 - 806.8)

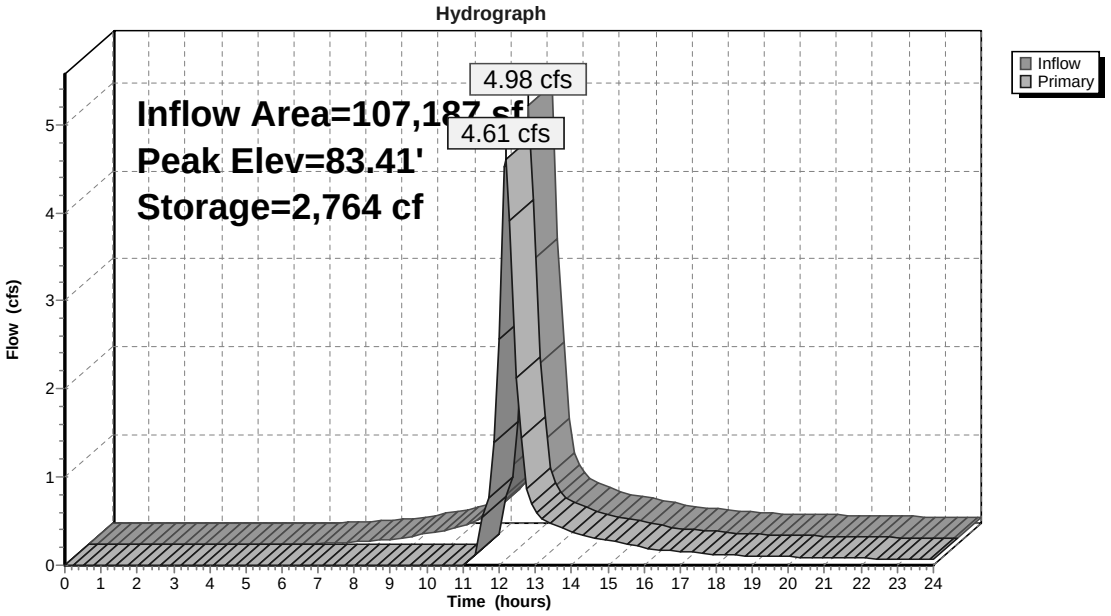
Volume	Invert	Avail.Storage	Storage Description
#1	82.50'	8,308 cf	**EX. INFILTRATION BASIN (Prismatic)**Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.50	430	0	0
83.00	2,900	833	833
84.00	12,050	7,475	8,308

Device	Routing	Invert	Outlet Devices
#1	Primary	83.30'	**50.0' long x 60.0' breadth GRASS/LAWN AREA** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=4.36 cfs @ 12.19 hrs HW=83.40' TW=82.65' (Dynamic Tailwater)
1=GRASS/LAWN AREA (Weir Controls 4.36 cfs @ 0.86 fps)

Pond P-2C: EX. INFILTRTATION BASIN

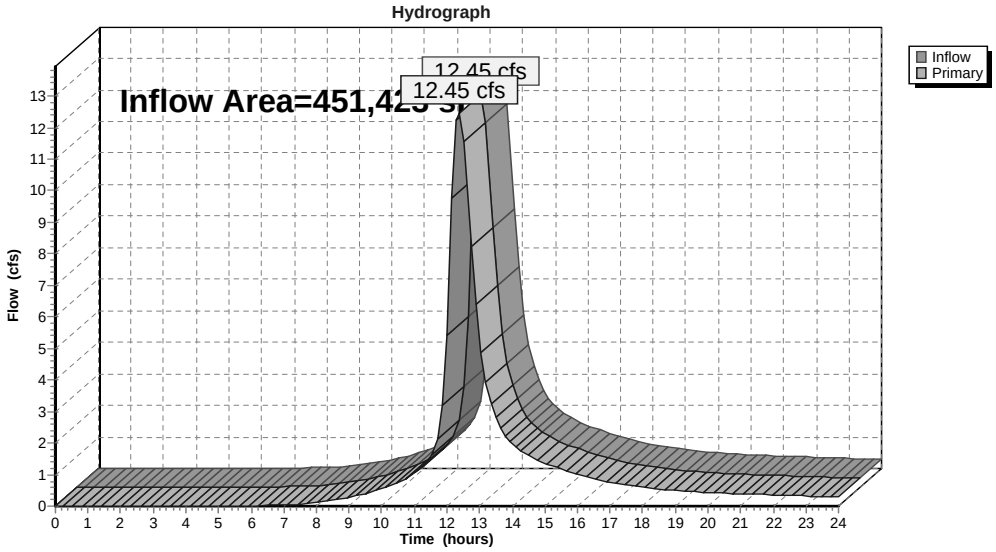


Summary for Link SP-1: STUDY POINT #1

Inflow Area = 451,423 sf, 65.30% Impervious, Inflow Depth > 2.19" for 2-Year event
Inflow = 12.45 cfs @ 12.38 hrs, Volume= 82,247 cf
Primary = 12.45 cfs @ 12.38 hrs, Volume= 82,247 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-1: STUDY POINT #1

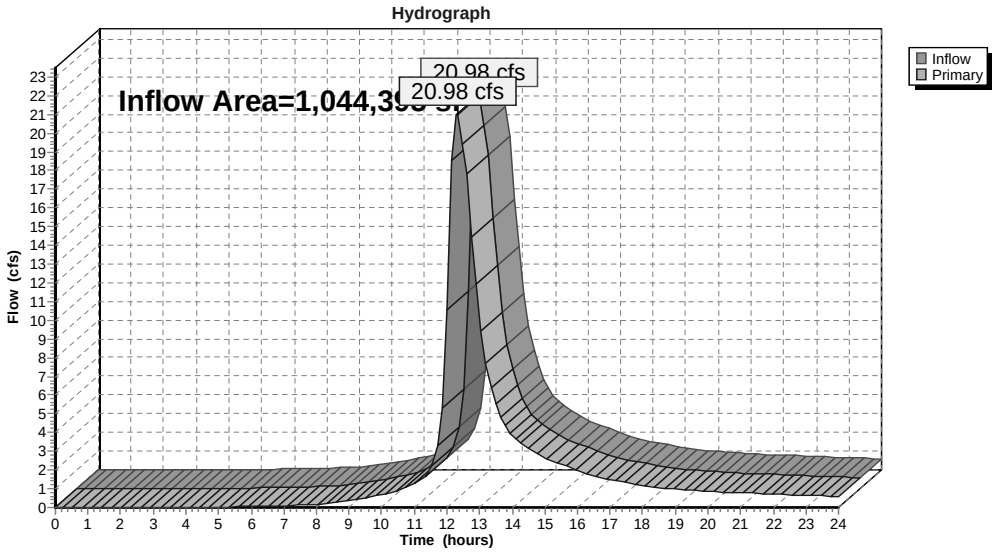


Summary for Link SP-2: STUDY POINT #2

Inflow Area = 1,044,395 sf, 40.73% Impervious, Inflow Depth > 1.70" for 2-Year event
Inflow = 20.98 cfs @ 12.32 hrs, Volume= 147,693 cf
Primary = 20.98 cfs @ 12.32 hrs, Volume= 147,693 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-2: STUDY POINT #2

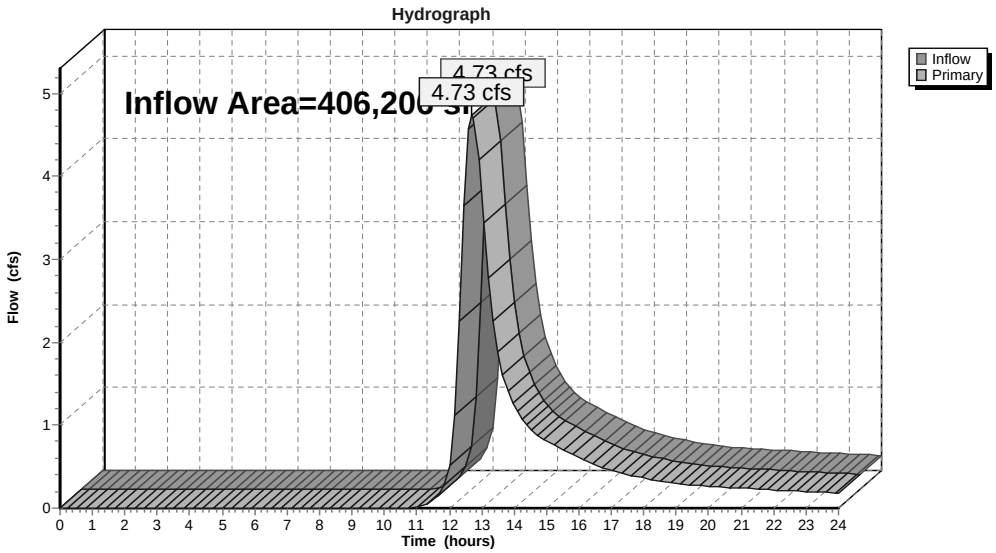


Summary for Link SP-3: STUDY POINT #3

Inflow Area = 406,206 sf, 2.31% Impervious, Inflow Depth > 1.01" for 2-Year event
Inflow = 4.73 cfs @ 12.71 hrs, Volume= 34,345 cf
Primary = 4.73 cfs @ 12.71 hrs, Volume= 34,345 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-3: STUDY POINT #3



Summary for Subcatchment E-1A: EX. WATERSHED

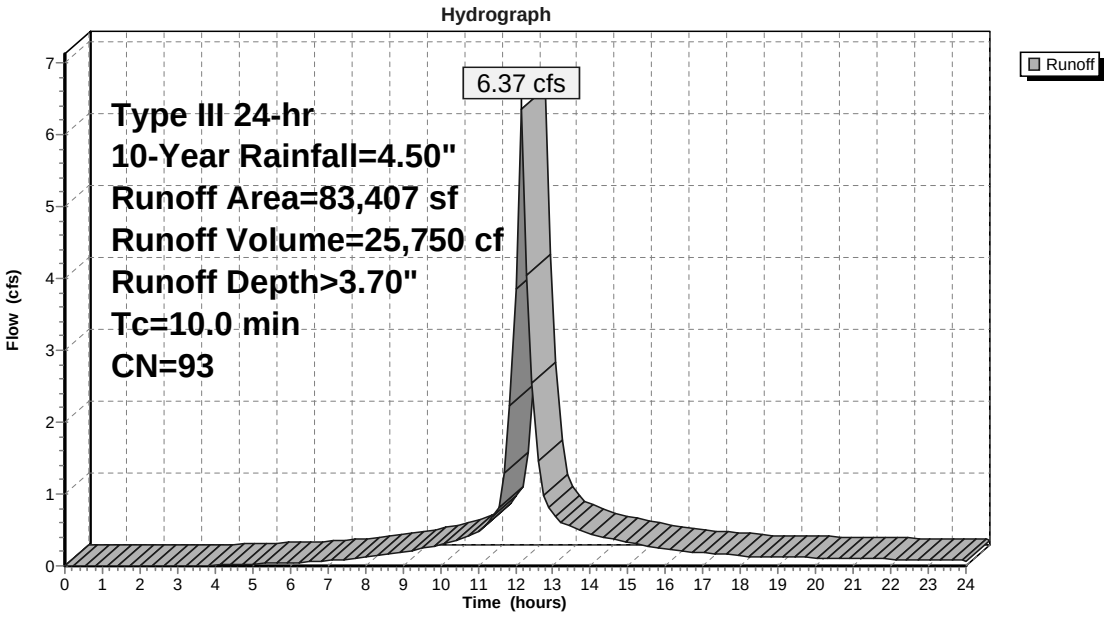
Runoff = 6.37 cfs @ 12.15 hrs, Volume= 25,750 cf, Depth> 3.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	6,343	86	<50% Grass cover, Poor, HSG C
	8,692	74	>75% Grass cover, Good, HSG C
	5,285	79	Woods/grass comb., Good, HSG D
*	63,087	98	Impervious
	83,407	93	Weighted Average
	20,320		24.36% Pervious Area
	63,087		75.64% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1A: EX. WATERSHED



Summary for Subcatchment E-1B: EX. WATERSHED

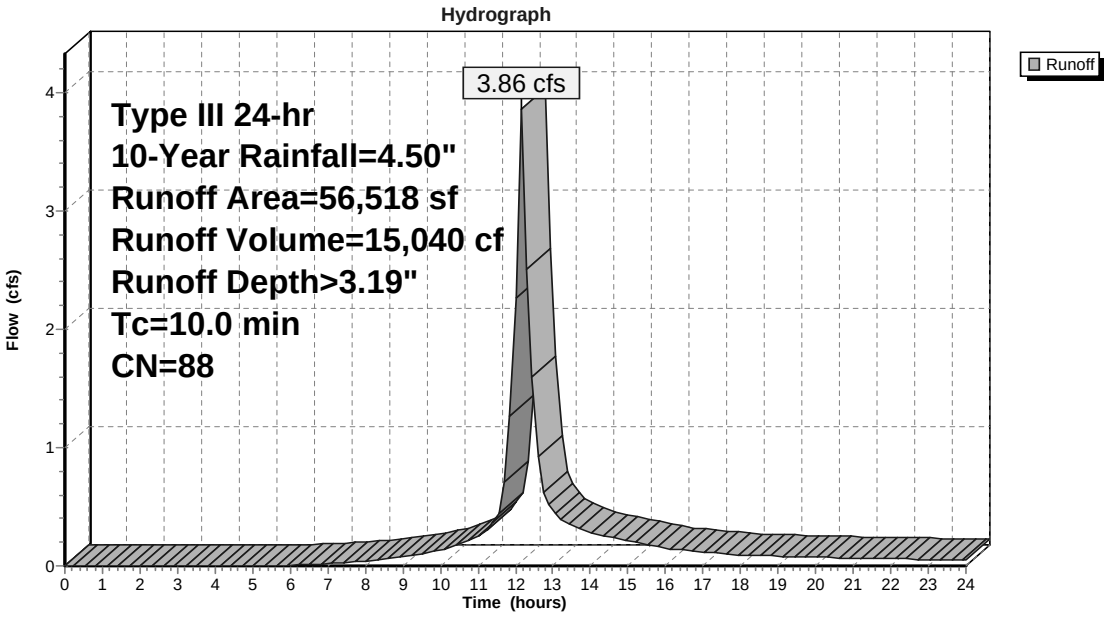
Runoff = 3.86 cfs @ 12.16 hrs, Volume= 15,040 cf, Depth> 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	3,722	86	<50% Grass cover, Poor, HSG C
	8,826	74	>75% Grass cover, Good, HSG C
	14,964	79	Woods/grass comb., Good, HSG D
*	29,006	98	Impervious
	56,518	88	Weighted Average
	27,512		48.68% Pervious Area
	29,006		51.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1B: EX. WATERSHED



Summary for Subcatchment E-1C: EX. WATERSHED

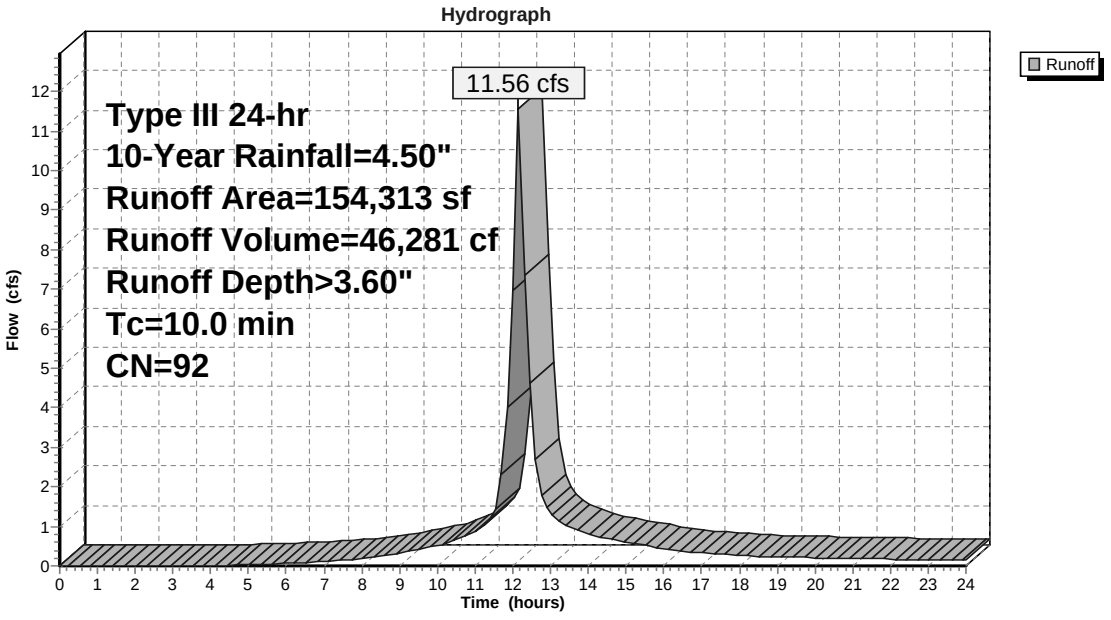
Runoff = 11.56 cfs @ 12.15 hrs, Volume= 46,281 cf, Depth> 3.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	3,840	86	<50% Grass cover, Poor, HSG C
	20,360	74	>75% Grass cover, Good, HSG C
	23,421	79	Woods/grass comb., Good, HSG D
*	106,692	98	Impervious
	154,313	92	Weighted Average
	47,621		30.86% Pervious Area
	106,692		69.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1C: EX. WATERSHED



Summary for Subcatchment E-1D: EX. WATERSHED

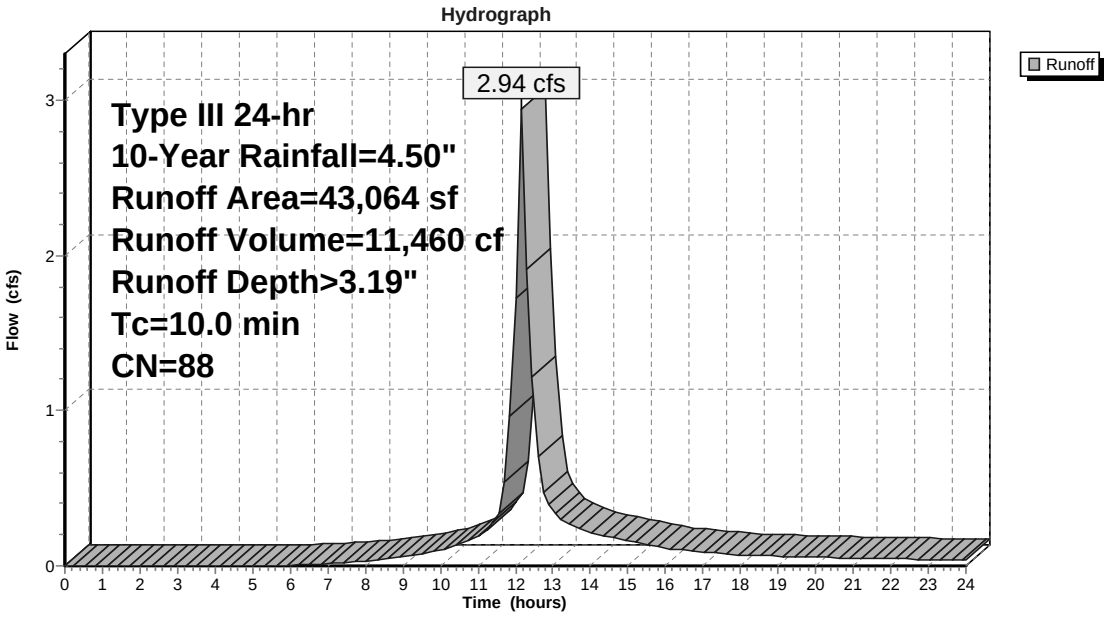
Runoff = 2.94 cfs @ 12.16 hrs, Volume= 11,460 cf, Depth> 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	5,705	86	<50% Grass cover, Poor, HSG C
	12,809	74	>75% Grass cover, Good, HSG C
	2,569	79	Woods/grass comb., Good, HSG D
*	21,981	98	Impervious
	43,064	88	Weighted Average
	21,083		48.96% Pervious Area
	21,981		51.04% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1D: EX. WATERSHED



Summary for Subcatchment E-1E: EX. WATERSHED

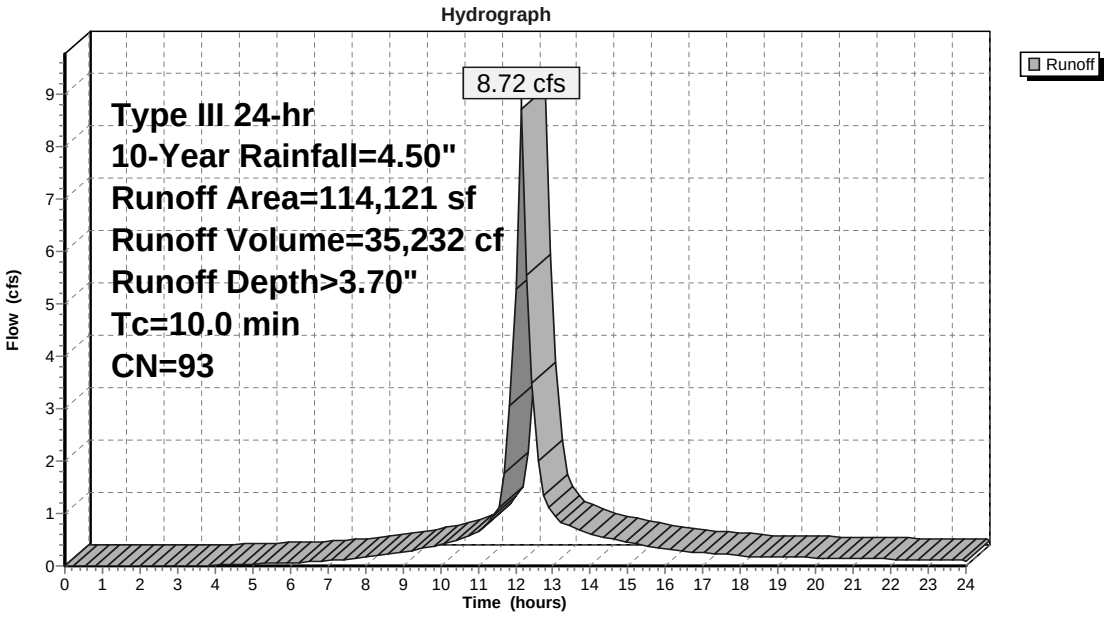
Runoff = 8.72 cfs @ 12.15 hrs, Volume= 35,232 cf, Depth> 3.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	6,384	86	<50% Grass cover, Poor, HSG C
	17,883	74	>75% Grass cover, Good, HSG C
	15,843	96	Gravel surface, HSG C
*	74,011	98	Impervious
	114,121	93	Weighted Average
	40,110		35.15% Pervious Area
	74,011		64.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1E: EX. WATERSHED



Summary for Subcatchment E-2A: EX. WATERSHED

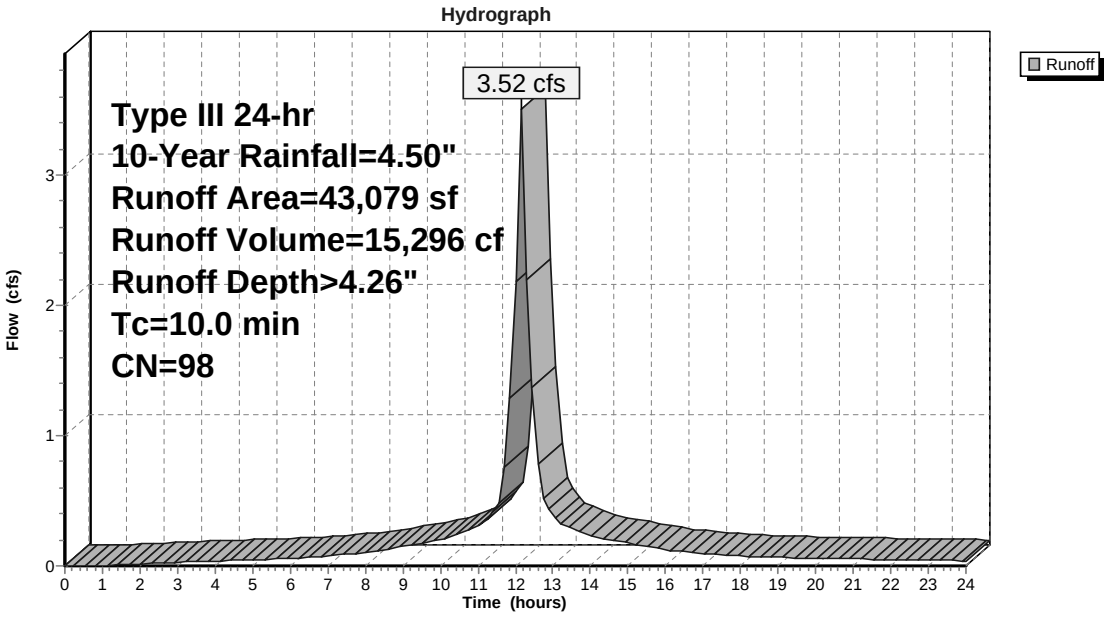
Runoff = 3.52 cfs @ 12.15 hrs, Volume= 15,296 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	1,483	86	<50% Grass cover, Poor, HSG C
*	41,596	98	Impervious
	43,079	98	Weighted Average
	1,483		3.44% Pervious Area
	41,596		96.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2A: EX. WATERSHED



Summary for Subcatchment E-2B: EX. WATERSHED

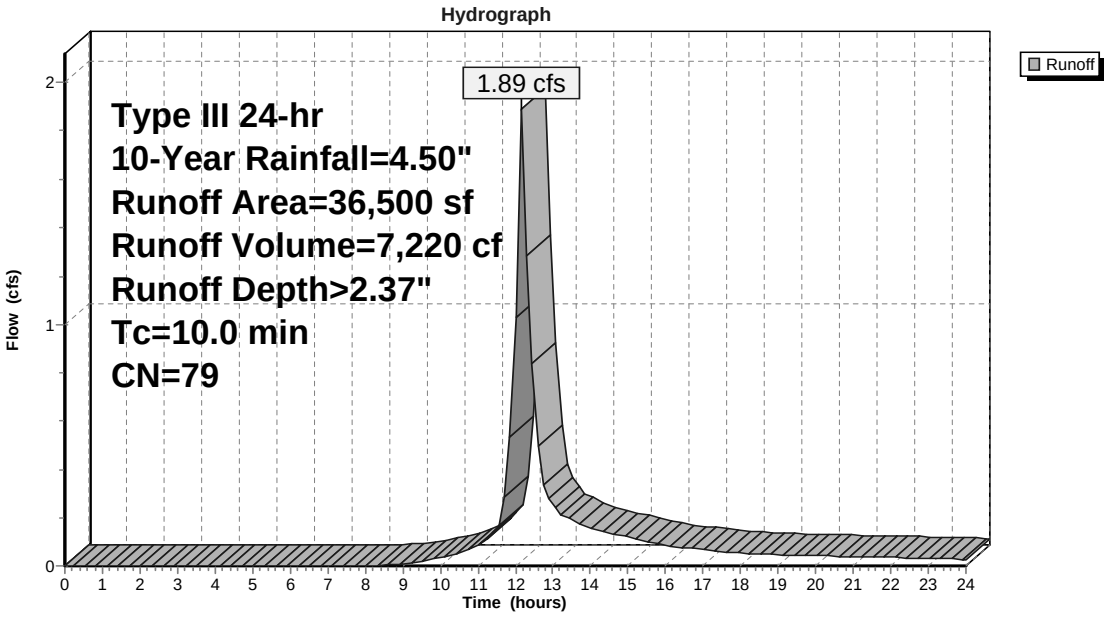
Runoff = 1.89 cfs @ 12.16 hrs, Volume= 7,220 cf, Depth> 2.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	1,043	86	<50% Grass cover, Poor, HSG C
	28,476	74	>75% Grass cover, Good, HSG C
*	6,981	98	Impervious
	36,500	79	Weighted Average
	29,519		80.87% Pervious Area
	6,981		19.13% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2B: EX. WATERSHED



Summary for Subcatchment E-2C: EX. WATERSHED

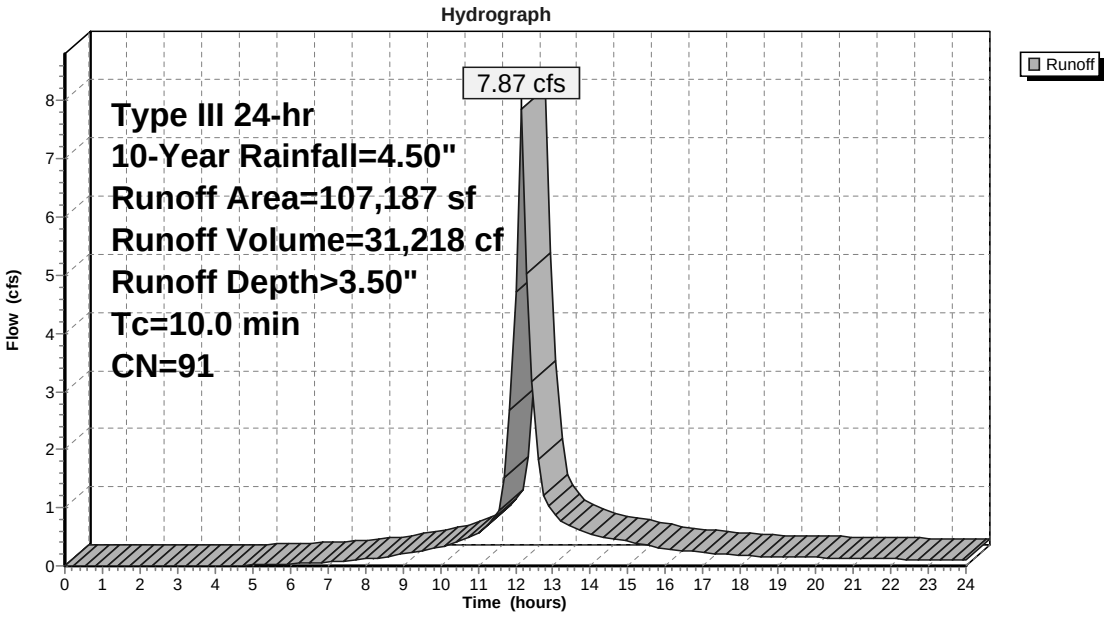
Runoff = 7.87 cfs @ 12.15 hrs, Volume= 31,218 cf, Depth> 3.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	2,816	86	<50% Grass cover, Poor, HSG C
	31,699	74	>75% Grass cover, Good, HSG C
*	72,672	98	Impervious
	107,187	91	Weighted Average
	34,515		32.20% Pervious Area
	72,672		67.80% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2C: EX. WATERSHED



Summary for Subcatchment E-3: EX. WATERSHED

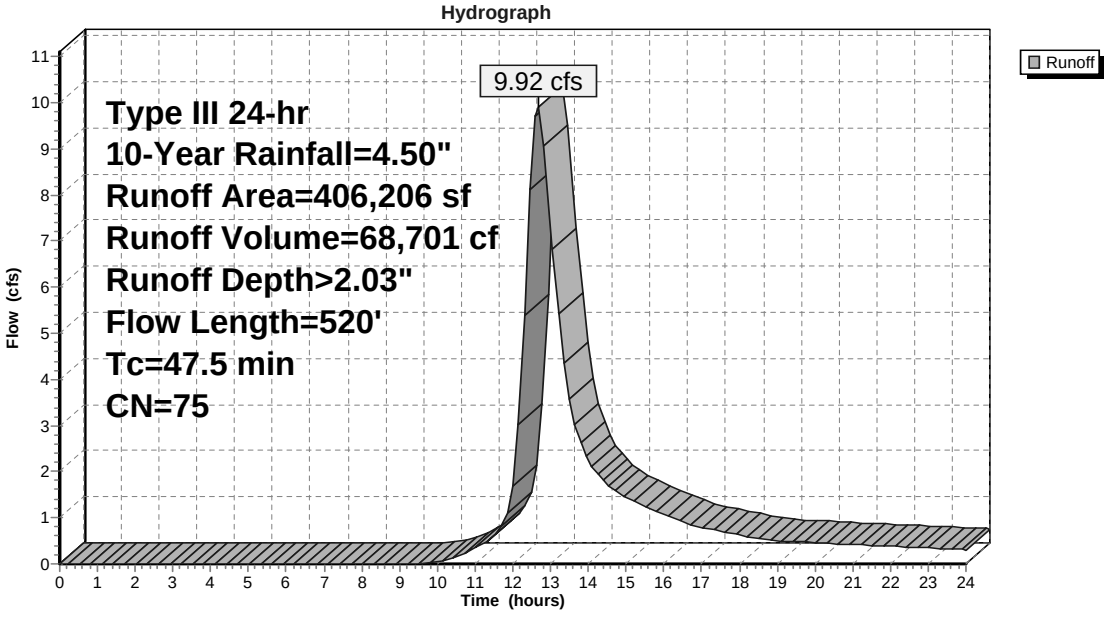
Runoff = 9.92 cfs @ 12.67 hrs, Volume= 68,701 cf, Depth> 2.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	237,023	77	Woods, Good, HSG D
*	9,386	98	Impervious
	159,797	70	Woods, Good, HSG C
	406,206	75	Weighted Average
	396,820		97.69% Pervious Area
	9,386		2.31% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	100	0.0100	0.06		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
1.3	40	0.0100	0.50		**Shallow Concentrated Flow, B-C** Woodland Kv= 5.0 fps
17.9	380	0.0050	0.35		**Shallow Concentrated Flow, C-D** Woodland Kv= 5.0 fps
47.5	520	Total			

Subcatchment E-3: EX. WATERSHED



Summary for Reach R-1A: DRAINAGE DITCH

Inflow Area = 451,423 sf, 65.30% Impervious, Inflow Depth > 3.52" for 10-Year event
Inflow = 21.09 cfs @ 12.35 hrs, Volume= 132,274 cf
Outflow = 21.29 cfs @ 12.35 hrs, Volume= 132,199 cf, Atten= 0%, Lag= 0.0 min

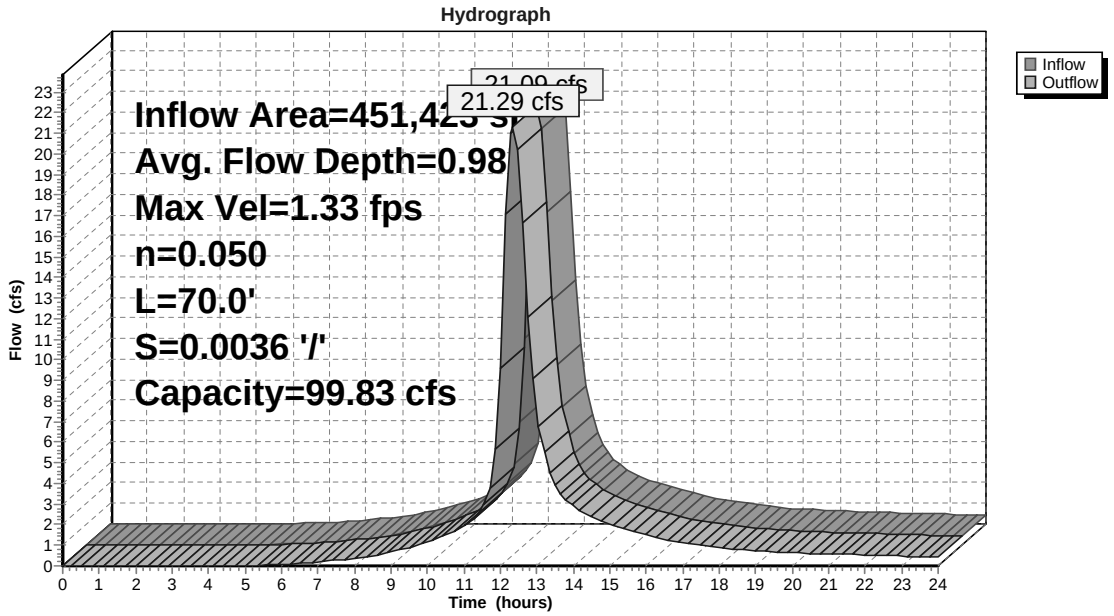
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 1.33 fps, Min. Travel Time= 0.9 min
Avg. Velocity = 0.50 fps, Avg. Travel Time= 2.3 min

Peak Storage= 1,119 cf @ 12.35 hrs
Average Depth at Peak Storage= 0.98' , Surface Width= 24.49'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 99.83 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 70.0' Slope= 0.0036 '/'
Inlet Invert= 78.75', Outlet Invert= 78.50'

‡

Reach R-1A: DRAINAGE DITCH



Summary for Reach R-1B: DRAINAGE DITCH

Inflow Area = 368,016 sf, 62.96% Impervious, Inflow Depth > 3.49" for 10-Year event
Inflow = 18.99 cfs @ 12.30 hrs, Volume= 107,070 cf
Outflow = 17.83 cfs @ 12.41 hrs, Volume= 106,524 cf, Atten= 6%, Lag= 6.2 min

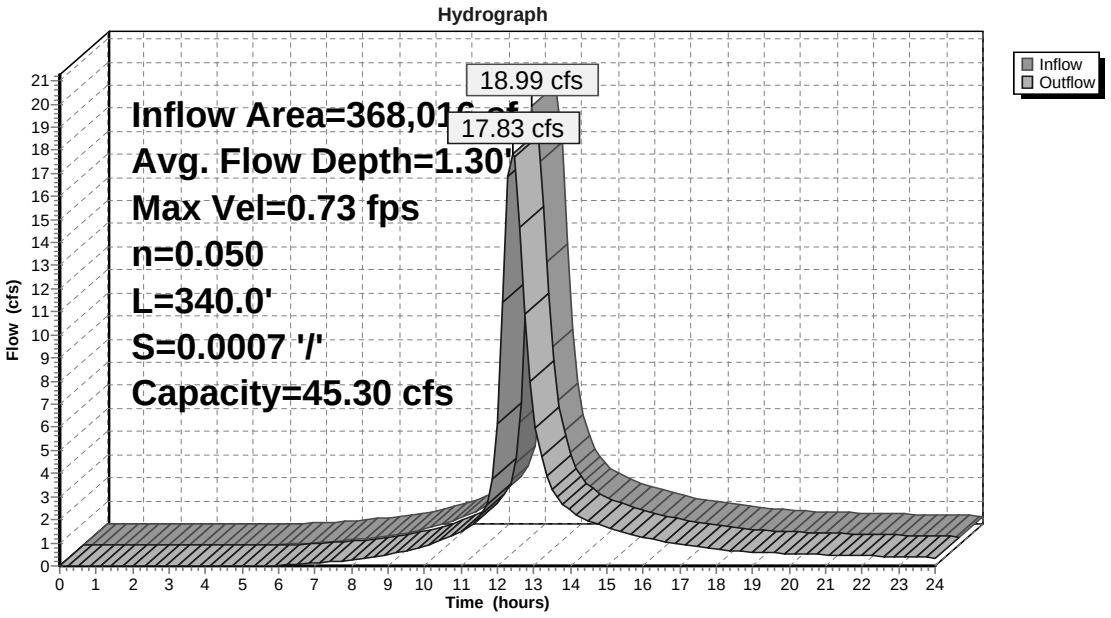
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 0.73 fps, Min. Travel Time= 7.8 min
Avg. Velocity = 0.27 fps, Avg. Travel Time= 20.7 min

Peak Storage= 8,310 cf @ 12.41 hrs
Average Depth at Peak Storage= 1.30' , Surface Width= 28.21'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 45.30 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 340.0' Slope= 0.0007 '/'
Inlet Invert= 79.00', Outlet Invert= 78.75'

‡

Reach R-1B: DRAINAGE DITCH



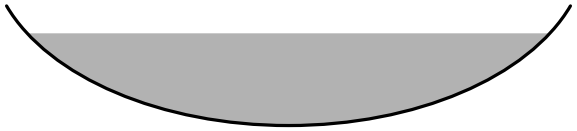
Summary for Reach R-1C: DRAINAGE DITCH

Inflow Area = 311,498 sf, 65.07% Impervious, Inflow Depth > 3.57" for 10-Year event
Inflow = 19.59 cfs @ 12.19 hrs, Volume= 92,663 cf
Outflow = 16.62 cfs @ 12.33 hrs, Volume= 92,030 cf, Atten= 15%, Lag= 8.6 min

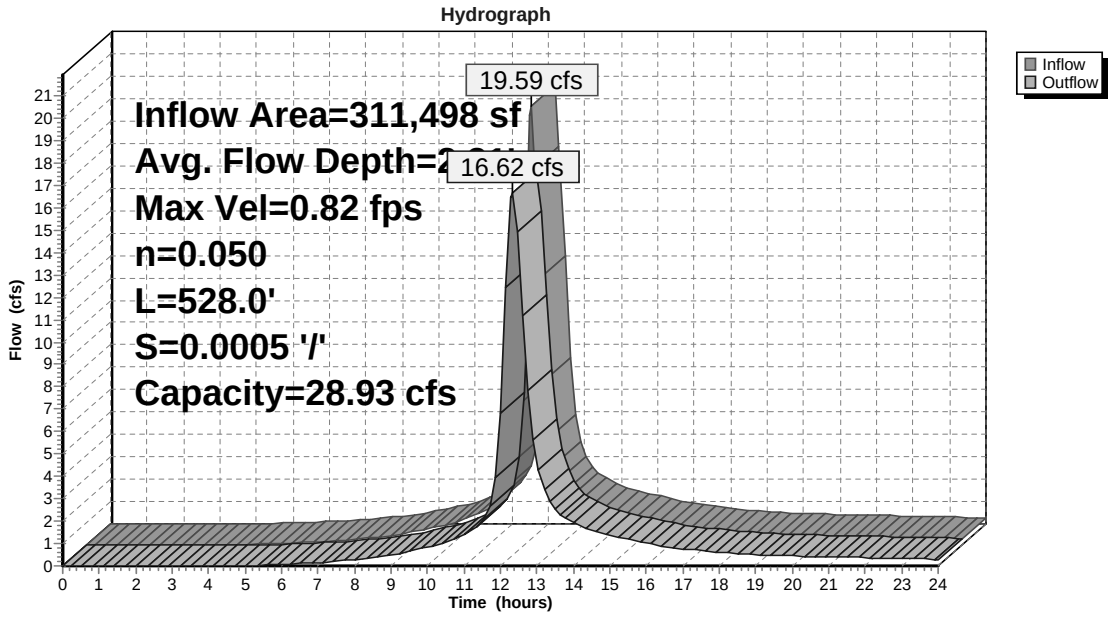
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 0.82 fps, Min. Travel Time= 10.8 min
Avg. Velocity = 0.31 fps, Avg. Travel Time= 28.7 min

Peak Storage= 10,701 cf @ 12.33 hrs
Average Depth at Peak Storage= 2.31' , Surface Width= 13.16'
Bank-Full Depth= 3.00' Flow Area= 30.0 sf, Capacity= 28.93 cfs

15.00' x 3.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0005 '/'
Inlet Invert= 79.25', Outlet Invert= 79.00'



Reach R-1C: DRAINAGE DITCH



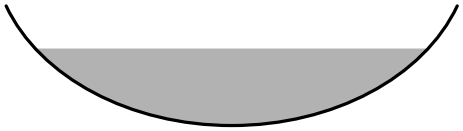
Summary for Reach R-1D: DRAINAGE DITCH

Inflow Area = 157,185 sf, 61.07% Impervious, Inflow Depth > 3.56" for 10-Year event
Inflow = 10.75 cfs @ 12.18 hrs, Volume= 46,622 cf
Outflow = 9.47 cfs @ 12.29 hrs, Volume= 46,382 cf, Atten= 12%, Lag= 6.7 min

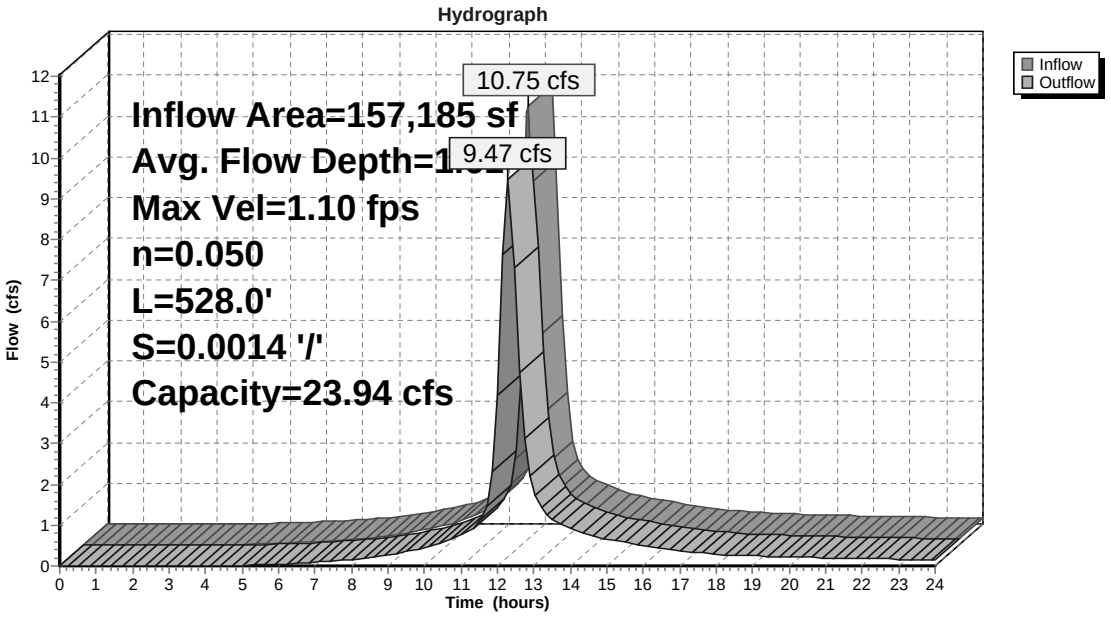
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 1.10 fps, Min. Travel Time= 8.0 min
Avg. Velocity = 0.40 fps, Avg. Travel Time= 22.0 min

Peak Storage= 4,539 cf @ 12.29 hrs
Average Depth at Peak Storage= 1.61' , Surface Width= 8.02'
Bank-Full Depth= 2.50' Flow Area= 16.7 sf, Capacity= 23.94 cfs

10.00' x 2.50' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0014 '/'
Inlet Invert= 80.00', Outlet Invert= 79.25'



Reach R-1D: DRAINAGE DITCH



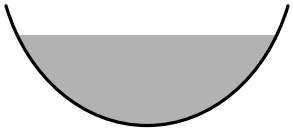
Summary for Reach R-1E: DRAINAGE DITCH

Inflow Area = 114,121 sf, 64.85% Impervious, Inflow Depth > 3.70" for 10-Year event
Inflow = 8.72 cfs @ 12.15 hrs, Volume= 35,232 cf
Outflow = 7.86 cfs @ 12.20 hrs, Volume= 35,162 cf, Atten= 10%, Lag= 2.7 min

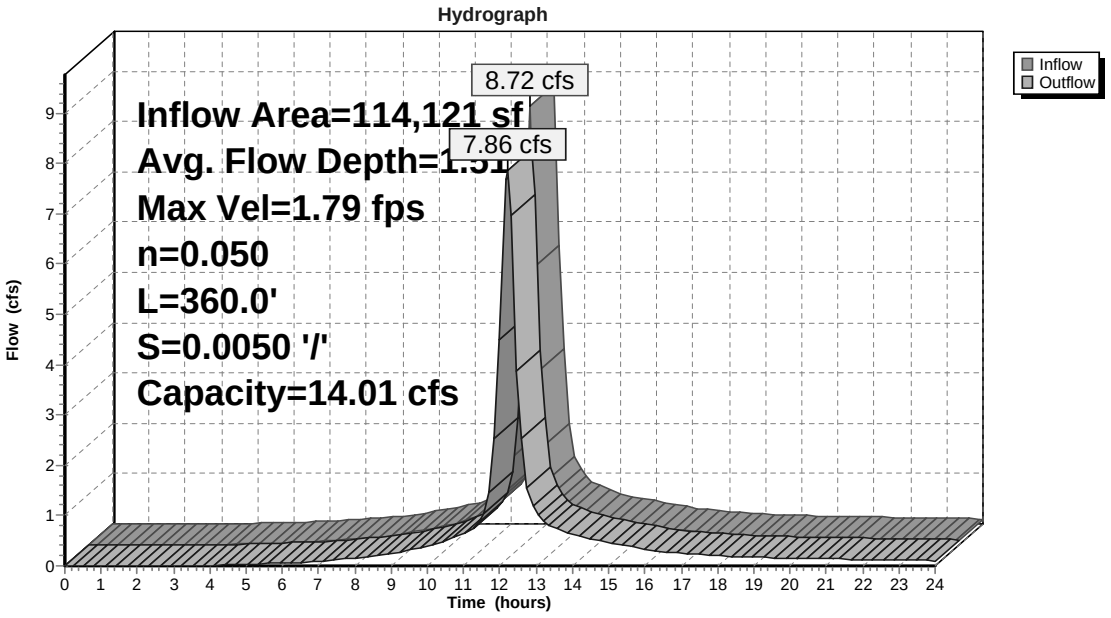
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 1.79 fps, Min. Travel Time= 3.3 min
Avg. Velocity = 0.67 fps, Avg. Travel Time= 8.9 min

Peak Storage= 1,573 cf @ 12.20 hrs
Average Depth at Peak Storage= 1.51' , Surface Width= 4.35'
Bank-Full Depth= 2.00' Flow Area= 6.7 sf, Capacity= 14.01 cfs

5.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 360.0' Slope= 0.0050 '/'
Inlet Invert= 81.80', Outlet Invert= 80.00'



Reach R-1E: DRAINAGE DITCH



Summary for Pond OFFSITE: OFFSITE PONDING AREA IN GRASS

Inflow Area = 107,187 sf, 67.80% Impervious, Inflow Depth > 3.25" for 10-Year event
Inflow = 7.35 cfs @ 12.18 hrs, Volume= 29,060 cf
Outflow = 7.31 cfs @ 12.19 hrs, Volume= 28,849 cf, Atten= 1%, Lag= 0.6 min
Primary = 7.31 cfs @ 12.19 hrs, Volume= 28,849 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 82.68' @ 12.19 hrs Surf.Area= 3,304 sf Storage= 1,025 cf

Plug-Flow detention time= 11.4 min calculated for 28,849 cf (99% of inflow)
Center-of-Mass det. time= 7.0 min (823.8 - 816.9)

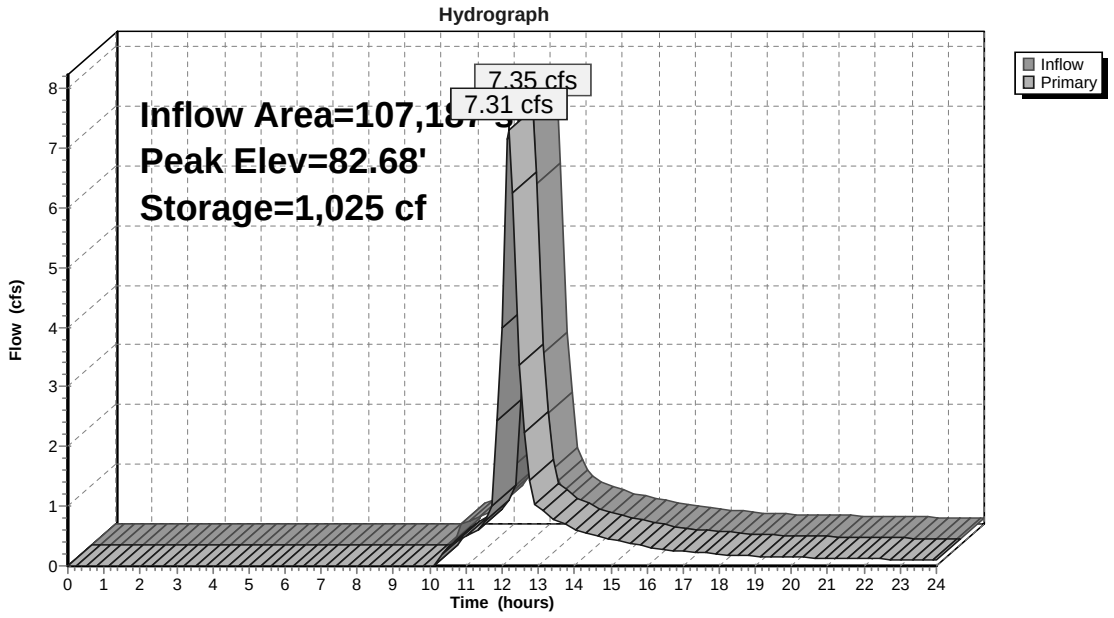
Volume	Invert	Avail.Storage	Storage Description
#1	82.10'	2,354 cf	**OFFSITE PONDING AREA (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.10	230	0	0
83.00	5,000	2,354	2,354

Device	Routing	Invert	Outlet Devices
#1	Primary	82.27'	**8.0" Round (3) 8" HDPE X 3.00 w/ 2.0" inside fill** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.10' / 81.40' S= 0.0333 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.28 sf
#2	Primary	82.60'	**100.0' long x 20.0' breadth WEIR FLOW OVER WALKING PATH** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=6.90 cfs @ 12.19 hrs HW=82.68' TW=0.00' (Dynamic Tailwater)
└─**1=(3) 8" HDPE** (Inlet Controls 1.26 cfs @ 1.66 fps)
└─**2=WEIR FLOW OVER WALKING PATH** (Weir Controls 5.64 cfs @ 0.74 fps)

Pond OFFSITE: OFFSITE PONDING AREA IN GRASS



Summary for Pond P-2A: Parking Lot/Driveway

Inflow Area = 43,079 sf, 96.56% Impervious, Inflow Depth > 4.26" for 10-Year event
Inflow = 3.52 cfs @ 12.15 hrs, Volume= 15,296 cf
Outflow = 3.56 cfs @ 12.15 hrs, Volume= 15,294 cf, Atten= 0%, Lag= 0.1 min
Primary = 3.56 cfs @ 12.15 hrs, Volume= 15,294 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 84.04' @ 12.15 hrs Surf.Area= 667 sf Storage= 60 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= 0.4 min (753.4 - 753.0)

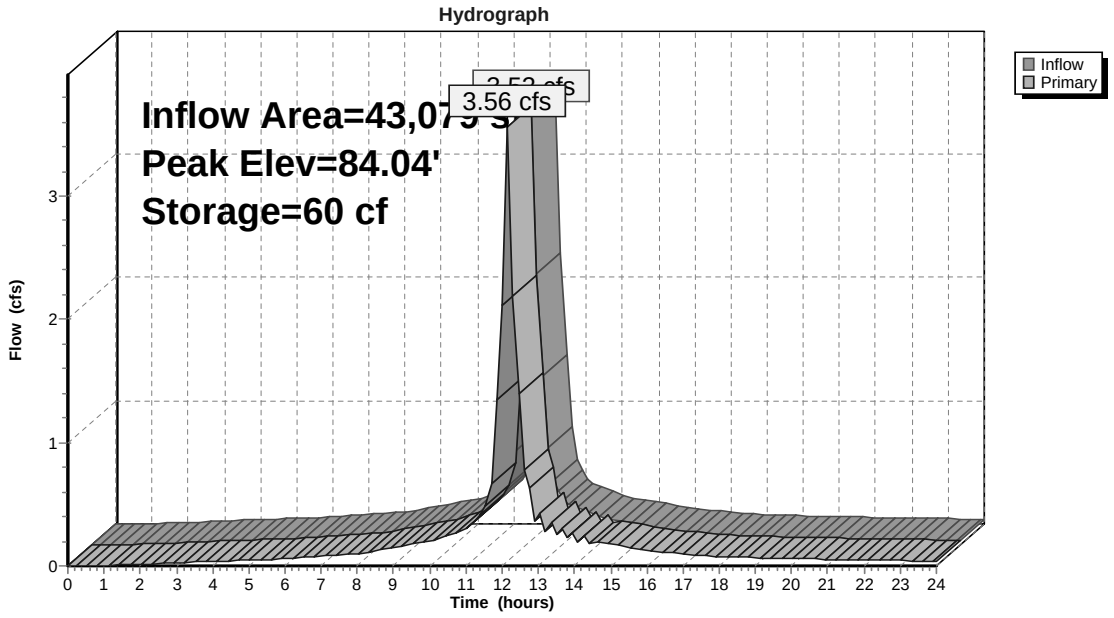
Volume	Invert	Avail.Storage	Storage Description
#1	82.38'	14 cf	**4.00'D x 1.10'H Ex.CB**
#2	83.45'	1,680 cf	**Parking/Driveway (Prismatic)**Listed below (Recalc)
		1,694 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.45	4	0	0
84.00	103	29	29
84.50	6,500	1,651	1,680

Device	Routing	Invert	Outlet Devices
#1	Primary	82.46'	**6.0" Round 6"PVC w/ 1.0" inside fill** L= 170.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.38' / 81.00' S= 0.0081 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.17 sf
#2	Primary	84.00'	**120.0' long x 50.0' breadth Weir Flow Over Curb Towards Lake** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=3.54 cfs @ 12.15 hrs HW=84.04' TW=0.00' (Dynamic Tailwater)
└**1=6"PVC** (Barrel Controls 0.68 cfs @ 3.89 fps)
└**2=Weir Flow Over Curb Towards Lake** (Weir Controls 2.85 cfs @ 0.55 fps)

Pond P-2A: Parking Lot/Driveway



Summary for Pond P-2C: EX. INFILTRTATION BASIN

Inflow Area = 107,187 sf, 67.80% Impervious, Inflow Depth > 3.50" for 10-Year event
Inflow = 7.87 cfs @ 12.15 hrs, Volume= 31,218 cf
Outflow = 7.35 cfs @ 12.18 hrs, Volume= 29,060 cf, Atten= 7%, Lag= 1.7 min
Primary = 7.35 cfs @ 12.18 hrs, Volume= 29,060 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 83.44' @ 12.18 hrs Surf.Area= 6,968 sf Storage= 3,026 cf

Plug-Flow detention time= 59.0 min calculated for 29,060 cf (93% of inflow)
Center-of-Mass det. time= 23.4 min (816.9 - 793.5)

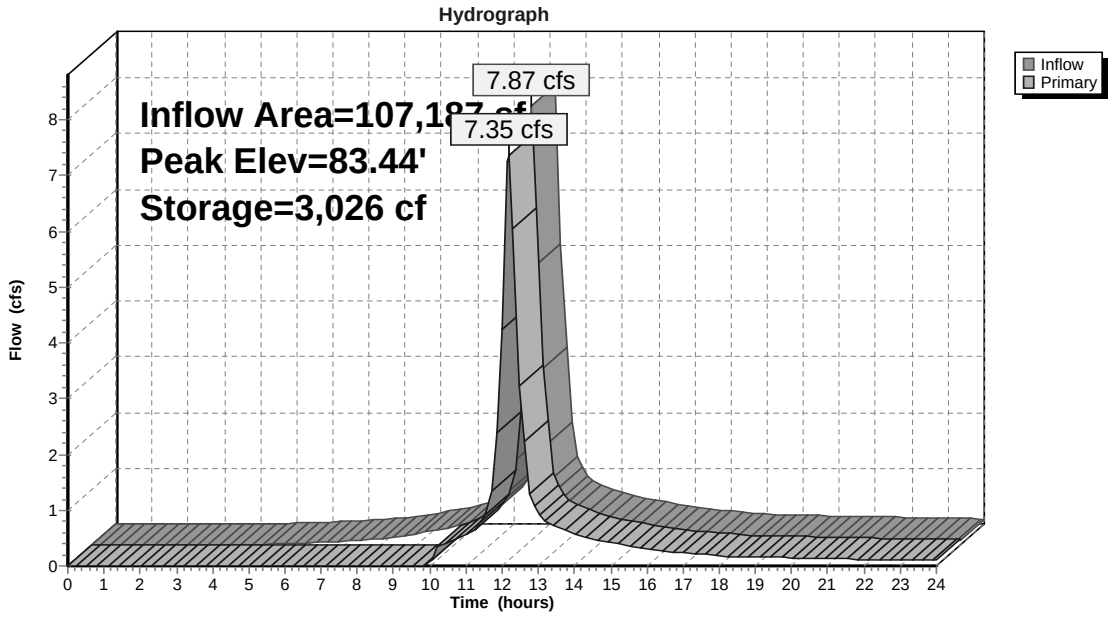
Volume	Invert	Avail.Storage	Storage Description
#1	82.50'	8,308 cf	**EX. INFILTRATION BASIN (Prismatic)**Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.50	430	0	0
83.00	2,900	833	833
84.00	12,050	7,475	8,308

Device	Routing	Invert	Outlet Devices
#1	Primary	83.30'	**50.0' long x 60.0' breadth GRASS/LAWN AREA** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=6.99 cfs @ 12.18 hrs HW=83.44' TW=82.68' (Dynamic Tailwater)
└**1=GRASS/LAWN AREA** (Weir Controls 6.99 cfs @ 1.00 fps)

Pond P-2C: EX. INFILTRTATION BASIN

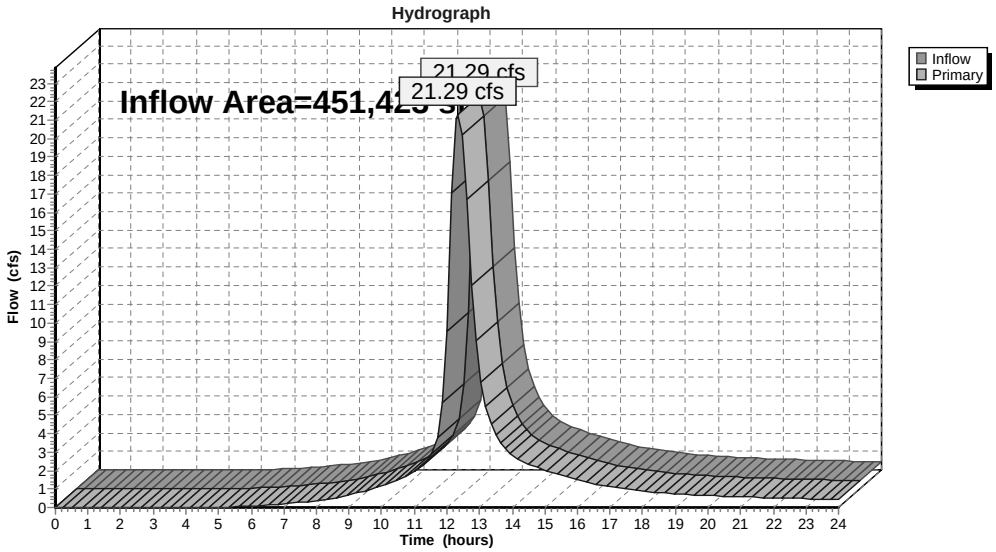


Summary for Link SP-1: STUDY POINT #1

Inflow Area = 451,423 sf, 65.30% Impervious, Inflow Depth > 3.51" for 10-Year event
Inflow = 21.29 cfs @ 12.35 hrs, Volume= 132,199 cf
Primary = 21.29 cfs @ 12.35 hrs, Volume= 132,199 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-1: STUDY POINT #1

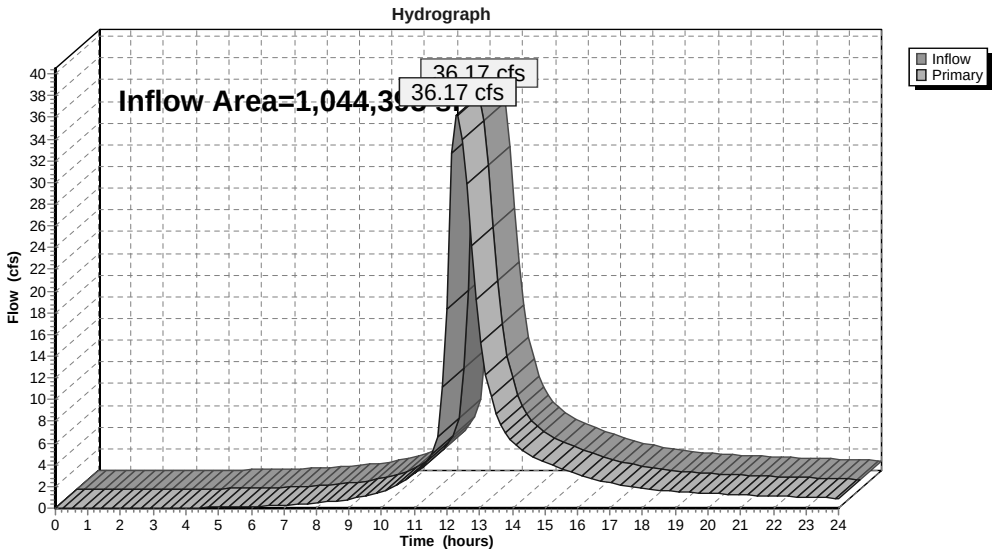


Summary for Link SP-2: STUDY POINT #2

Inflow Area = 1,044,395 sf, 40.73% Impervious, Inflow Depth > 2.90" for 10-Year event
Inflow = 36.17 cfs @ 12.32 hrs, Volume= 252,264 cf
Primary = 36.17 cfs @ 12.32 hrs, Volume= 252,264 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-2: STUDY POINT #2

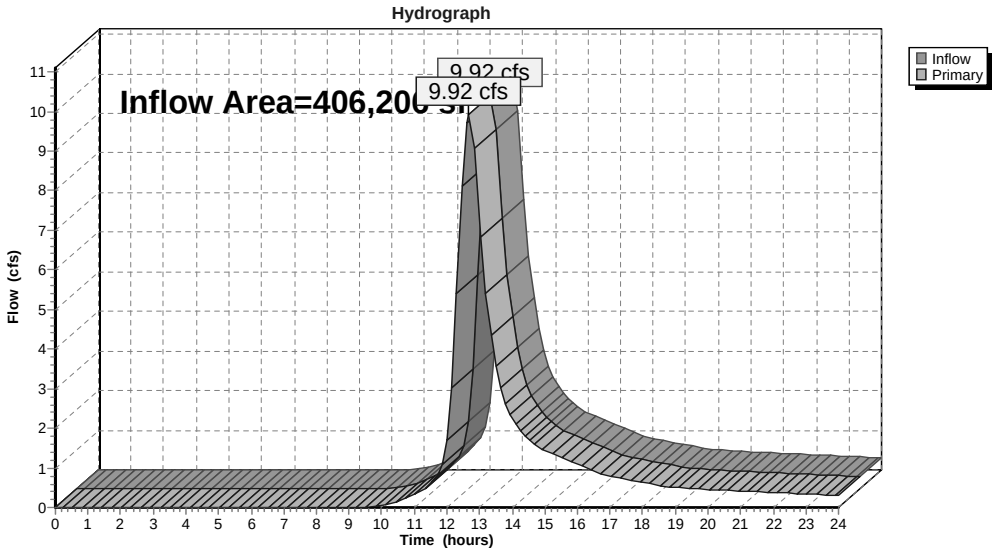


Summary for Link SP-3: STUDY POINT #3

Inflow Area = 406,206 sf, 2.31% Impervious, Inflow Depth > 2.03" for 10-Year event
Inflow = 9.92 cfs @ 12.67 hrs, Volume= 68,701 cf
Primary = 9.92 cfs @ 12.67 hrs, Volume= 68,701 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-3: STUDY POINT #3



Summary for Subcatchment E-1A: EX. WATERSHED

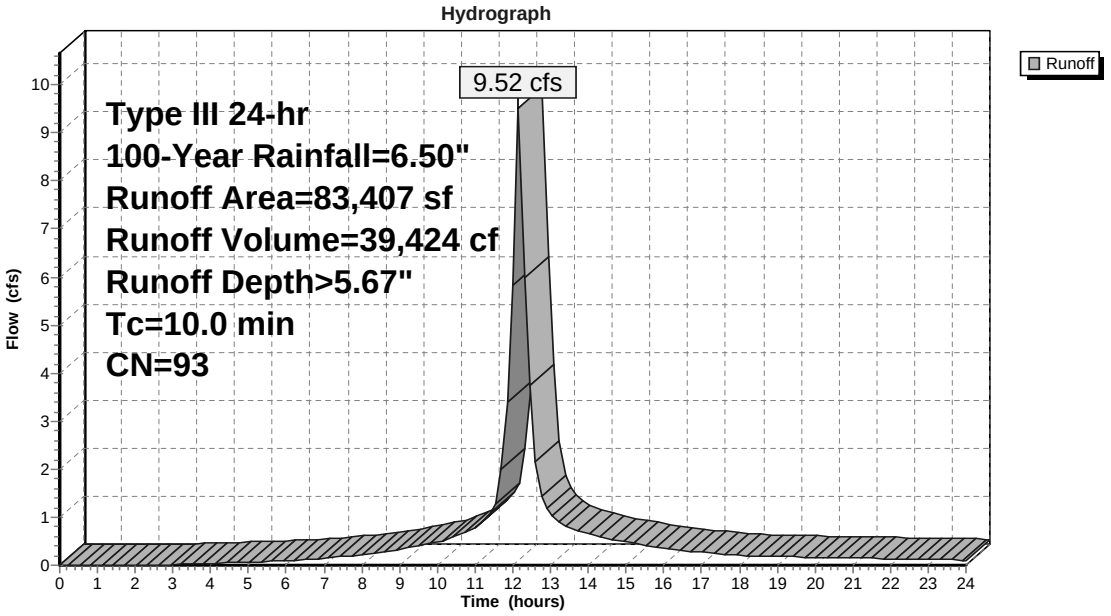
Runoff = 9.52 cfs @ 12.15 hrs, Volume= 39,424 cf, Depth> 5.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	6,343	86	<50% Grass cover, Poor, HSG C
	8,692	74	>75% Grass cover, Good, HSG C
	5,285	79	Woods/grass comb., Good, HSG D
*	63,087	98	Impervious
	83,407	93	Weighted Average
	20,320		24.36% Pervious Area
	63,087		75.64% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1A: EX. WATERSHED



Summary for Subcatchment E-1B: EX. WATERSHED

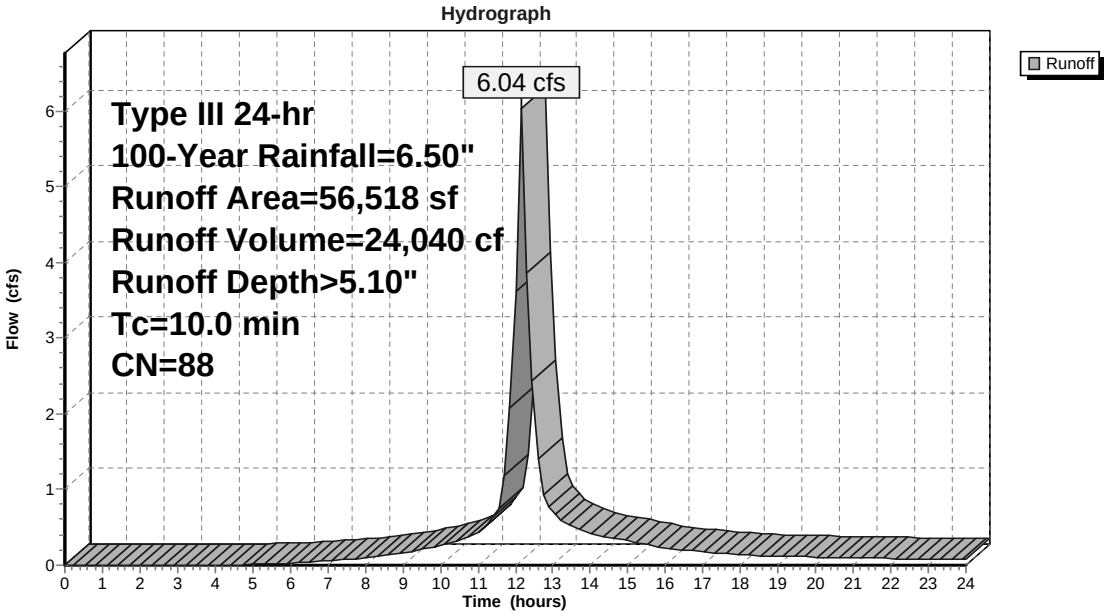
Runoff = 6.04 cfs @ 12.15 hrs, Volume= 24,040 cf, Depth> 5.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	3,722	86	<50% Grass cover, Poor, HSG C
	8,826	74	>75% Grass cover, Good, HSG C
	14,964	79	Woods/grass comb., Good, HSG D
*	29,006	98	Impervious
	56,518	88	Weighted Average
	27,512		48.68% Pervious Area
	29,006		51.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1B: EX. WATERSHED



Summary for Subcatchment E-1C: EX. WATERSHED

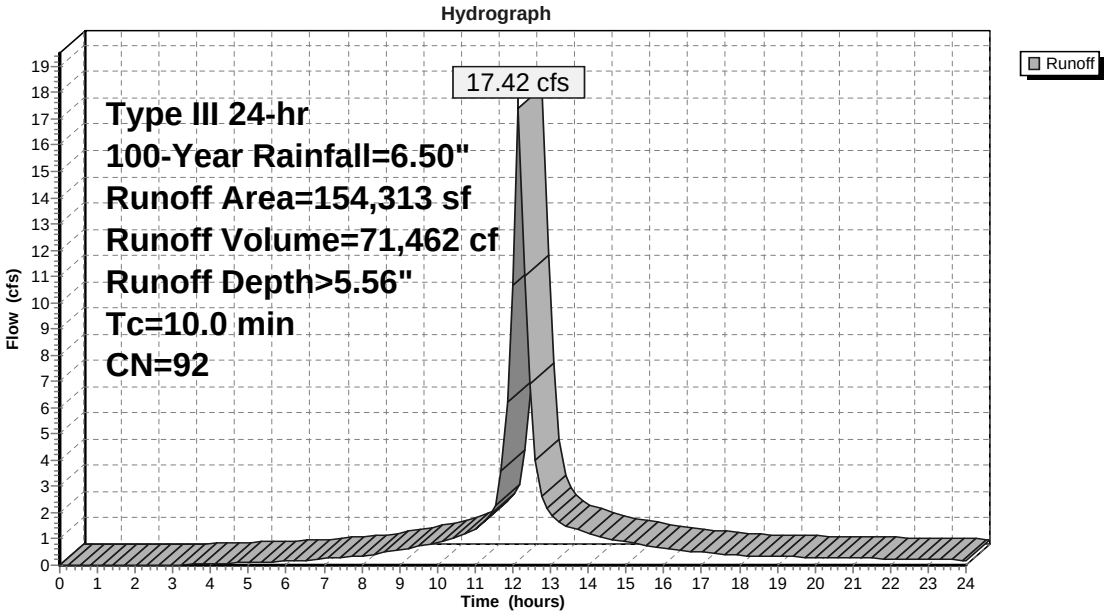
Runoff = 17.42 cfs @ 12.15 hrs, Volume= 71,462 cf, Depth> 5.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	3,840	86	<50% Grass cover, Poor, HSG C
	20,360	74	>75% Grass cover, Good, HSG C
	23,421	79	Woods/grass comb., Good, HSG D
*	106,692	98	Impervious
	154,313	92	Weighted Average
	47,621		30.86% Pervious Area
	106,692		69.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1C: EX. WATERSHED



Summary for Subcatchment E-1D: EX. WATERSHED

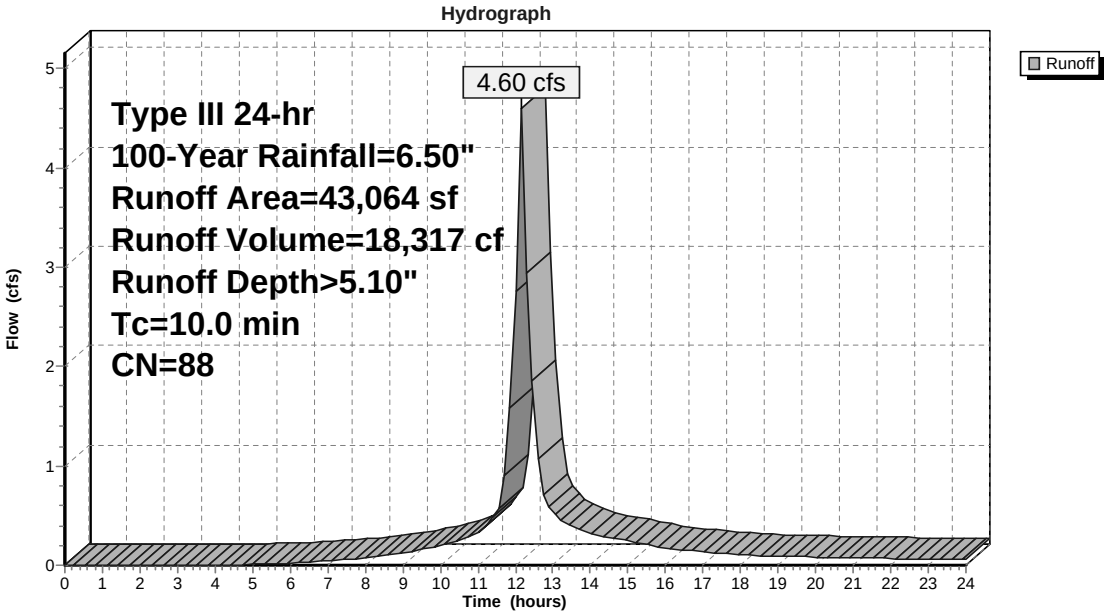
Runoff = 4.60 cfs @ 12.15 hrs, Volume= 18,317 cf, Depth> 5.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	5,705	86	<50% Grass cover, Poor, HSG C
	12,809	74	>75% Grass cover, Good, HSG C
	2,569	79	Woods/grass comb., Good, HSG D
*	21,981	98	Impervious
	43,064	88	Weighted Average
	21,083		48.96% Pervious Area
	21,981		51.04% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1D: EX. WATERSHED



Summary for Subcatchment E-1E: EX. WATERSHED

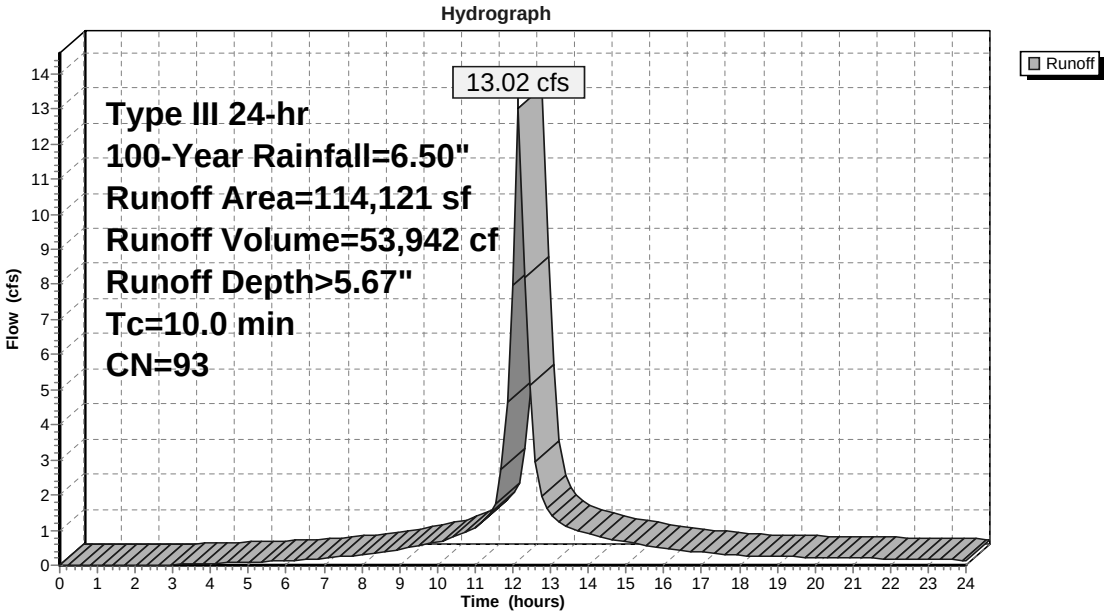
Runoff = 13.02 cfs @ 12.15 hrs, Volume= 53,942 cf, Depth> 5.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	6,384	86	<50% Grass cover, Poor, HSG C
	17,883	74	>75% Grass cover, Good, HSG C
	15,843	96	Gravel surface, HSG C
*	74,011	98	Impervious
	114,121	93	Weighted Average
	40,110		35.15% Pervious Area
	74,011		64.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-1E: EX. WATERSHED



Summary for Subcatchment E-2A: EX. WATERSHED

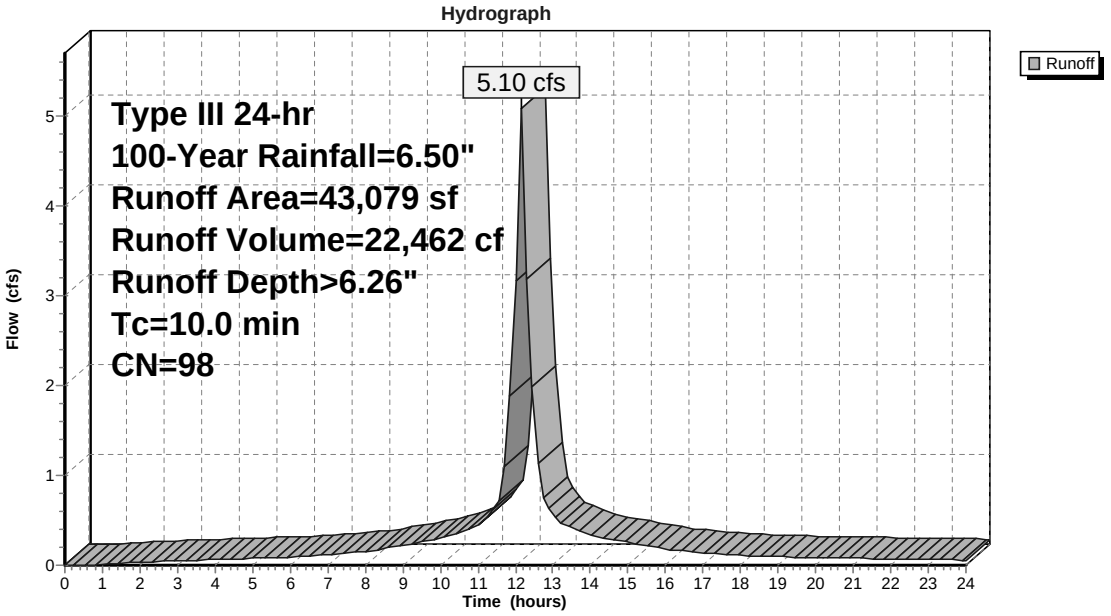
Runoff = 5.10 cfs @ 12.15 hrs, Volume= 22,462 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	1,483	86	<50% Grass cover, Poor, HSG C
*	41,596	98	Impervious
	43,079	98	Weighted Average
	1,483		3.44% Pervious Area
	41,596		96.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2A: EX. WATERSHED



Summary for Subcatchment E-2B: EX. WATERSHED

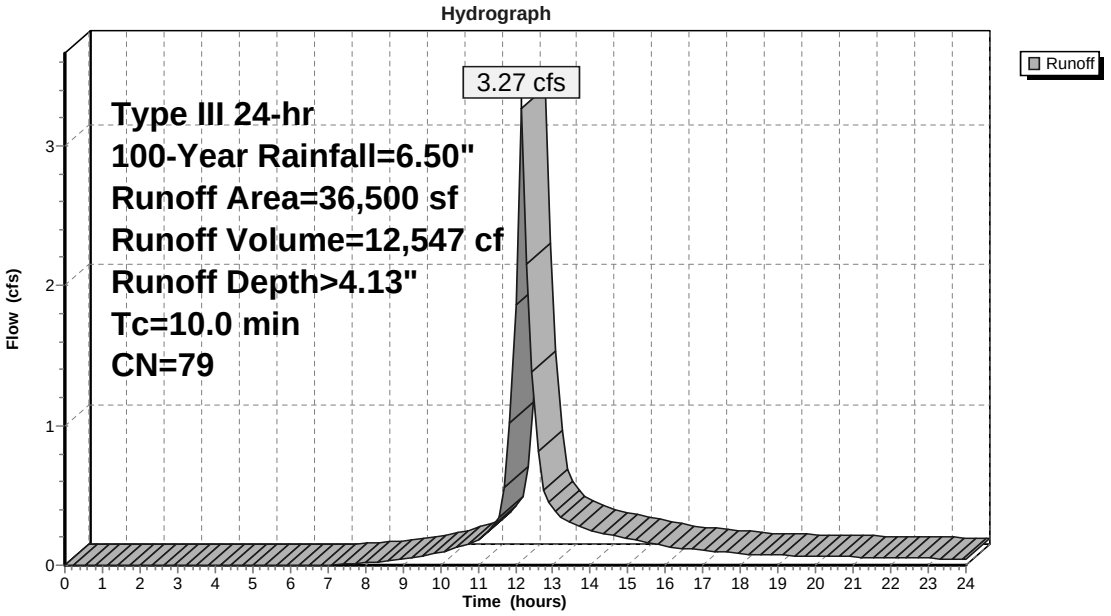
Runoff = 3.27 cfs @ 12.16 hrs, Volume= 12,547 cf, Depth> 4.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	1,043	86	<50% Grass cover, Poor, HSG C
	28,476	74	>75% Grass cover, Good, HSG C
*	6,981	98	Impervious
	36,500	79	Weighted Average
	29,519		80.87% Pervious Area
	6,981		19.13% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2B: EX. WATERSHED



Summary for Subcatchment E-2C: EX. WATERSHED

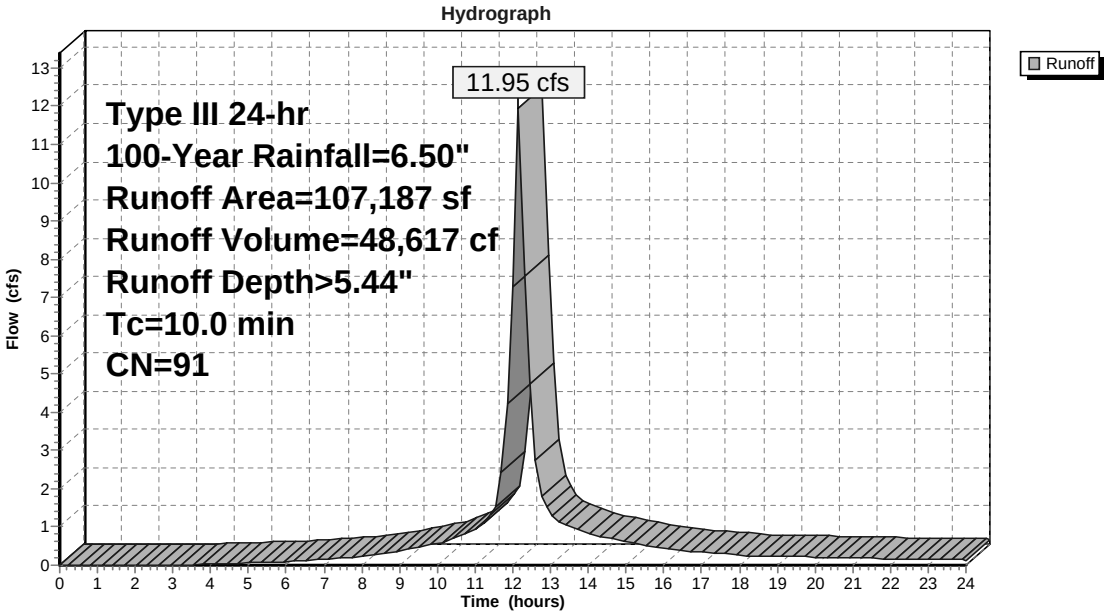
Runoff = 11.95 cfs @ 12.15 hrs, Volume= 48,617 cf, Depth> 5.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	2,816	86	<50% Grass cover, Poor, HSG C
	31,699	74	>75% Grass cover, Good, HSG C
*	72,672	98	Impervious
	107,187	91	Weighted Average
	34,515		32.20% Pervious Area
	72,672		67.80% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment E-2C: EX. WATERSHED



Summary for Subcatchment E-3: EX. WATERSHED

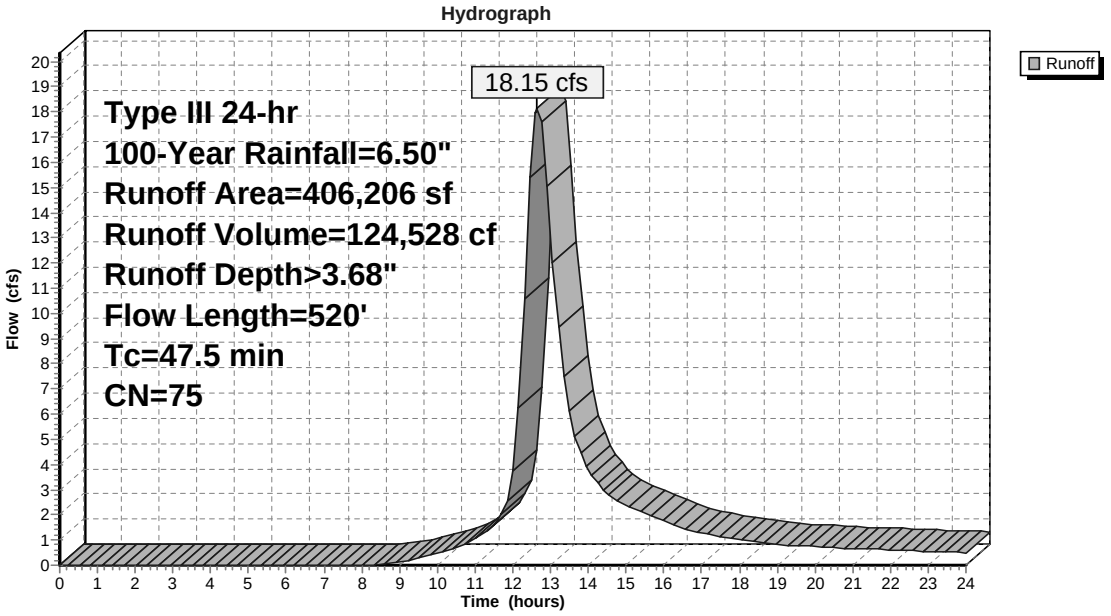
Runoff = 18.15 cfs @ 12.66 hrs, Volume= 124,528 cf, Depth> 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	237,023	77	Woods, Good, HSG D
*	9,386	98	Impervious
	159,797	70	Woods, Good, HSG C
	406,206	75	Weighted Average
	396,820		97.69% Pervious Area
	9,386		2.31% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	100	0.0100	0.06		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
1.3	40	0.0100	0.50		**Shallow Concentrated Flow, B-C** Woodland Kv= 5.0 fps
17.9	380	0.0050	0.35		**Shallow Concentrated Flow, C-D** Woodland Kv= 5.0 fps
47.5	520	Total			

Subcatchment E-3: EX. WATERSHED



Summary for Reach R-1A: DRAINAGE DITCH

Inflow Area = 451,423 sf, 65.30% Impervious, Inflow Depth > 5.46" for 100-Year event
Inflow = 33.83 cfs @ 12.32 hrs, Volume= 205,252 cf
Outflow = 34.16 cfs @ 12.33 hrs, Volume= 205,156 cf, Atten= 0%, Lag= 0.3 min

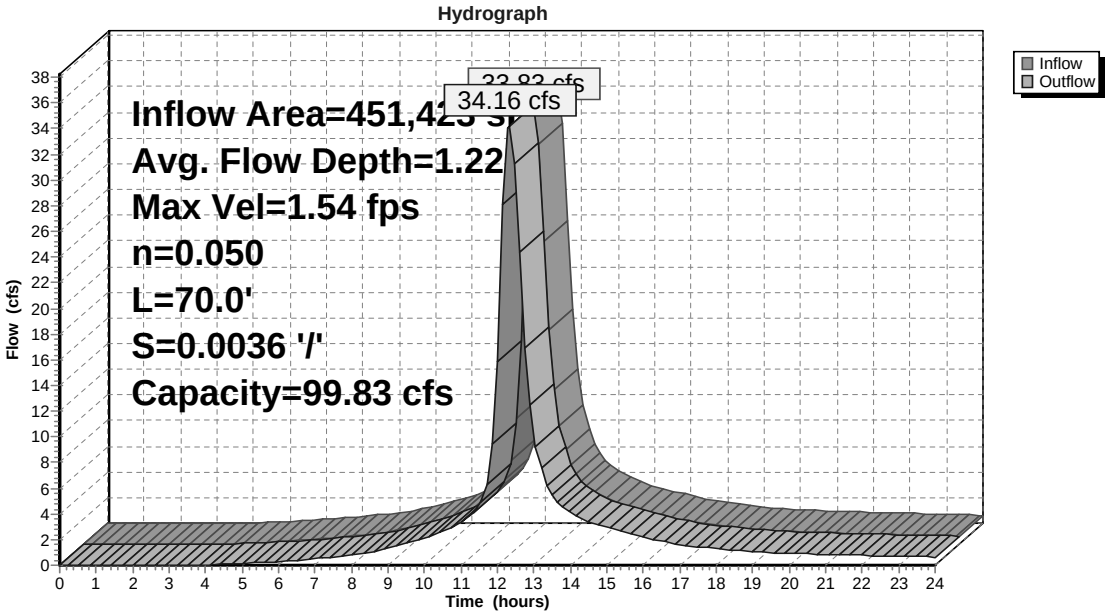
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 1.54 fps, Min. Travel Time= 0.8 min
Avg. Velocity = 0.57 fps, Avg. Travel Time= 2.1 min

Peak Storage= 1,552 cf @ 12.33 hrs
Average Depth at Peak Storage= 1.22' , Surface Width= 27.31'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 99.83 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 70.0' Slope= 0.0036 '/'
Inlet Invert= 78.75', Outlet Invert= 78.50'

‡

Reach R-1A: DRAINAGE DITCH



Summary for Reach R-1B: DRAINAGE DITCH

Inflow Area = 368,016 sf, 62.96% Impervious, Inflow Depth > 5.43" for 100-Year event
Inflow = 29.97 cfs @ 12.29 hrs, Volume= 166,536 cf
Outflow = 28.67 cfs @ 12.37 hrs, Volume= 165,828 cf, Atten= 4%, Lag= 4.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 0.84 fps, Min. Travel Time= 6.8 min
Avg. Velocity = 0.31 fps, Avg. Travel Time= 18.3 min

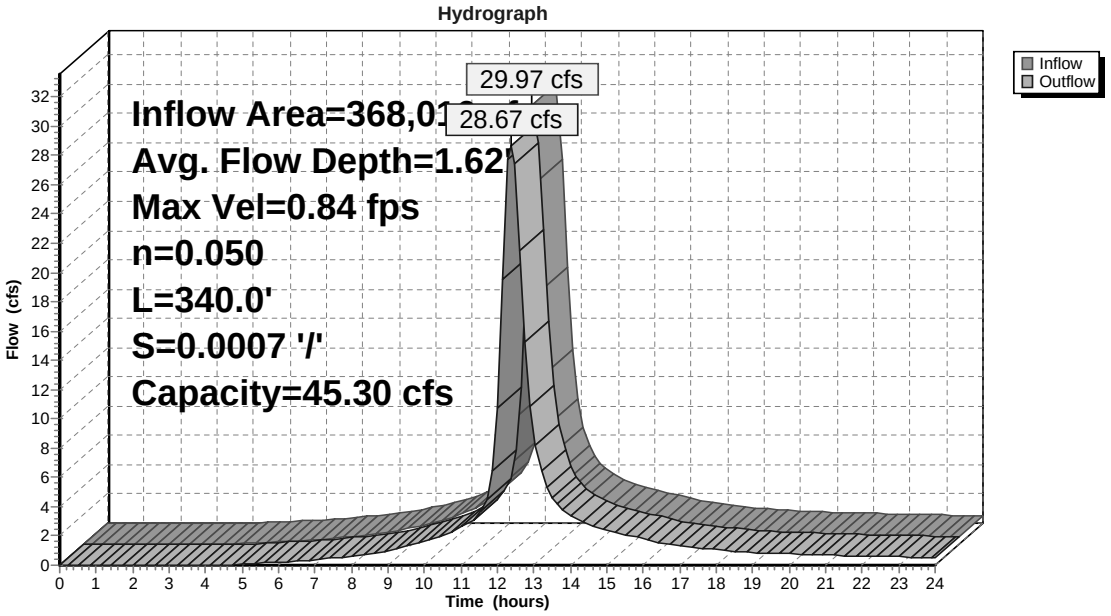
Peak Storage= 11,567 cf @ 12.37 hrs
Average Depth at Peak Storage= 1.62' , Surface Width= 31.51'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 45.30 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 340.0' Slope= 0.0007 '/'
Inlet Invert= 79.00', Outlet Invert= 78.75'

‡

Reach R-1B: DRAINAGE DITCH

Hydrograph



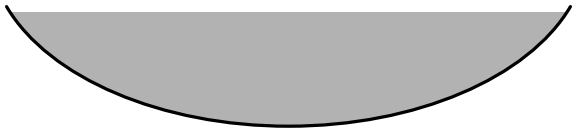
Summary for Reach R-1C: DRAINAGE DITCH

Inflow Area = 311,498 sf, 65.07% Impervious, Inflow Depth > 5.52" for 100-Year event
Inflow = 30.16 cfs @ 12.19 hrs, Volume= 143,317 cf
Outflow = 26.14 cfs @ 12.32 hrs, Volume= 142,496 cf, Atten= 13%, Lag= 7.8 min

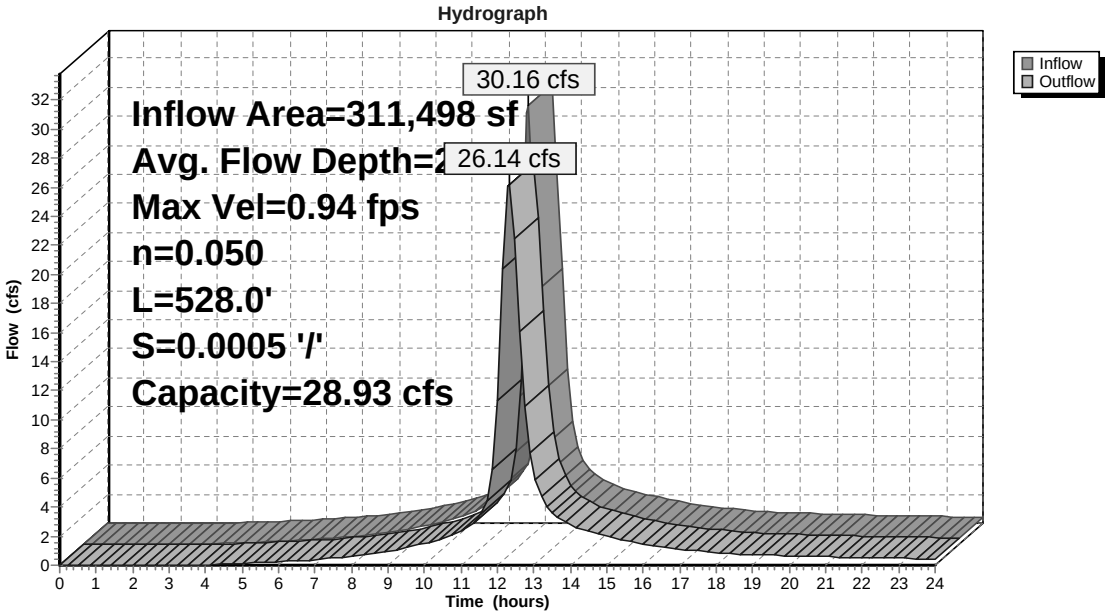
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 0.94 fps, Min. Travel Time= 9.4 min
Avg. Velocity = 0.35 fps, Avg. Travel Time= 25.4 min

Peak Storage= 14,741 cf @ 12.32 hrs
Average Depth at Peak Storage= 2.86' , Surface Width= 14.65'
Bank-Full Depth= 3.00' Flow Area= 30.0 sf, Capacity= 28.93 cfs

15.00' x 3.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0005 '/'
Inlet Invert= 79.25', Outlet Invert= 79.00'



Reach R-1C: DRAINAGE DITCH



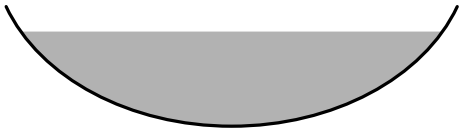
Summary for Reach R-1D: DRAINAGE DITCH

Inflow Area = 157,185 sf, 61.07% Impervious, Inflow Depth > 5.51" for 100-Year event
Inflow = 16.39 cfs @ 12.18 hrs, Volume= 72,168 cf
Outflow = 14.61 cfs @ 12.28 hrs, Volume= 71,855 cf, Atten= 11%, Lag= 6.2 min

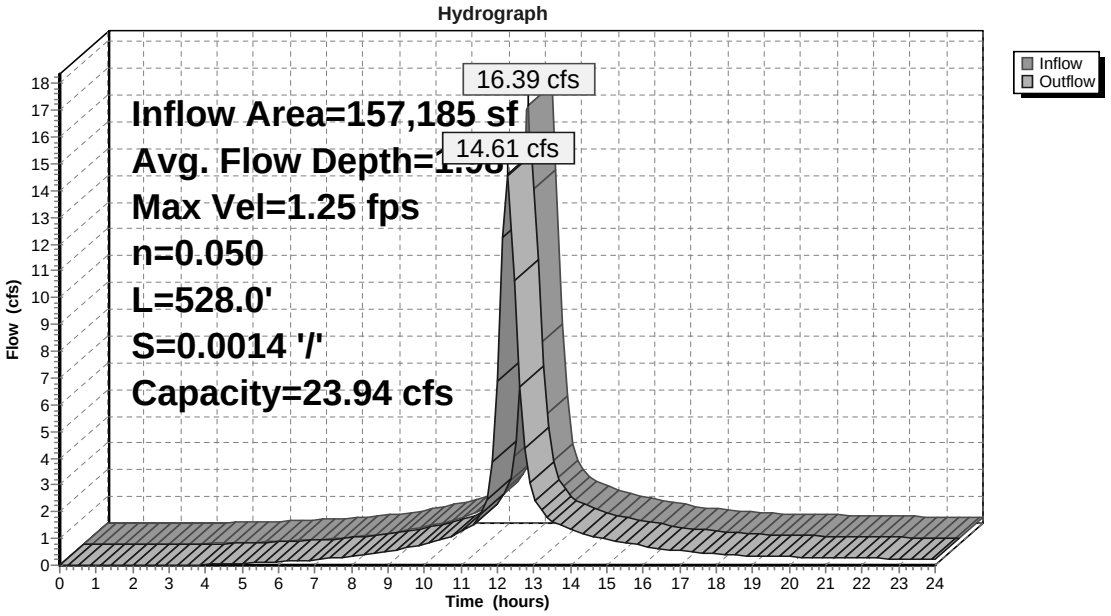
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 1.25 fps, Min. Travel Time= 7.1 min
Avg. Velocity = 0.45 fps, Avg. Travel Time= 19.5 min

Peak Storage= 6,180 cf @ 12.28 hrs
Average Depth at Peak Storage= 1.98' , Surface Width= 8.89'
Bank-Full Depth= 2.50' Flow Area= 16.7 sf, Capacity= 23.94 cfs

10.00' x 2.50' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0014 '/'
Inlet Invert= 80.00', Outlet Invert= 79.25'



Reach R-1D: DRAINAGE DITCH



Summary for Reach R-1E: DRAINAGE DITCH

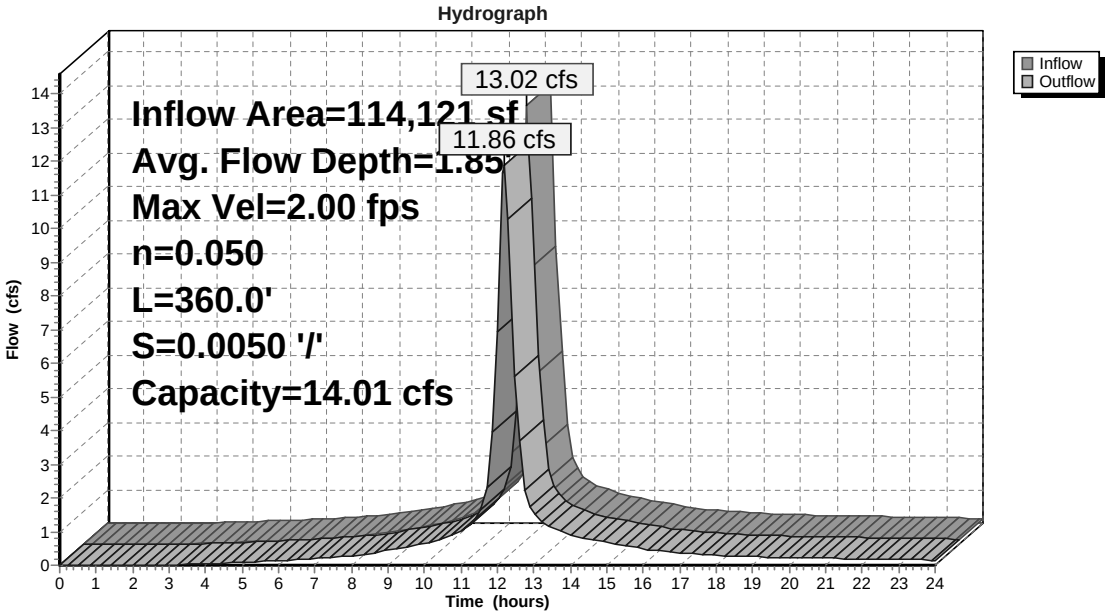
Inflow Area = 114,121 sf, 64.85% Impervious, Inflow Depth > 5.67" for 100-Year event
Inflow = 13.02 cfs @ 12.15 hrs, Volume= 53,942 cf
Outflow = 11.86 cfs @ 12.19 hrs, Volume= 53,850 cf, Atten= 9%, Lag= 2.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Max. Velocity= 2.00 fps, Min. Travel Time= 3.0 min
Avg. Velocity = 0.75 fps, Avg. Travel Time= 8.0 min

Peak Storage= 2,126 cf @ 12.19 hrs
Average Depth at Peak Storage= 1.85' , Surface Width= 4.80'
Bank-Full Depth= 2.00' Flow Area= 6.7 sf, Capacity= 14.01 cfs

5.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 360.0' Slope= 0.0050 '/'
Inlet Invert= 81.80', Outlet Invert= 80.00'

Reach R-1E: DRAINAGE DITCH



Summary for Pond OFFSITE: OFFSITE PONDING AREA IN GRASS

Inflow Area = 107,187 sf, 67.80% Impervious, Inflow Depth > 5.20" for 100-Year event
Inflow = 11.23 cfs @ 12.18 hrs, Volume= 46,446 cf
Outflow = 11.05 cfs @ 12.19 hrs, Volume= 46,201 cf, Atten= 2%, Lag= 0.7 min
Primary = 11.05 cfs @ 12.19 hrs, Volume= 46,201 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 82.71' @ 12.19 hrs Surf.Area= 3,460 sf Storage= 1,124 cf

Plug-Flow detention time= 9.5 min calculated for 46,201 cf (99% of inflow)
Center-of-Mass det. time= 6.2 min (807.0 - 800.9)

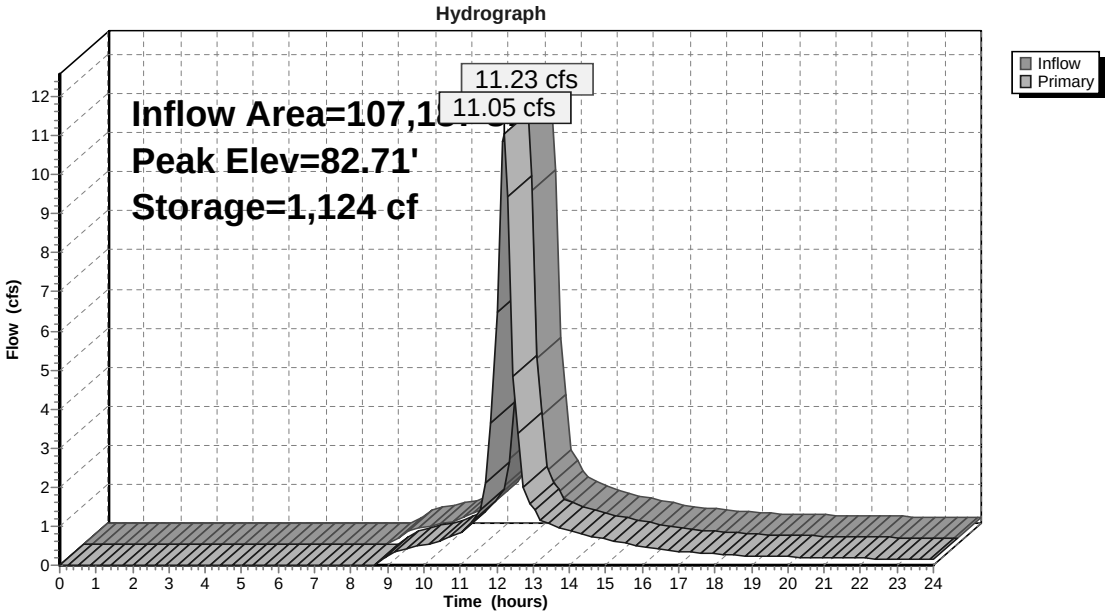
Volume	Invert	Avail.Storage	Storage Description
#1	82.10'	2,354 cf	**OFFSITE PONDING AREA (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.10	230	0	0
83.00	5,000	2,354	2,354

Device	Routing	Invert	Outlet Devices
#1	Primary	82.27'	**8.0" Round (3) 8" HDPE X 3.00 w/ 2.0" inside fill** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.10' / 81.40' S= 0.0333 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.28 sf
#2	Primary	82.60'	**100.0' long x 20.0' breadth WEIR FLOW OVER WALKING PATH** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=10.49 cfs @ 12.19 hrs HW=82.70' TW=0.00' (Dynamic Tailwater)
1=(3) 8" HDPE (Inlet Controls 1.38 cfs @ 1.73 fps)
2=WEIR FLOW OVER WALKING PATH (Weir Controls 9.11 cfs @ 0.87 fps)

Pond OFFSITE: OFFSITE PONDING AREA IN GRASS



Summary for Pond P-2A: Parking Lot/Driveway

Inflow Area = 43,079 sf, 96.56% Impervious, Inflow Depth > 6.26" for 100-Year event
Inflow = 5.10 cfs @ 12.15 hrs, Volume= 22,462 cf
Outflow = 5.06 cfs @ 12.15 hrs, Volume= 22,459 cf, Atten= 1%, Lag= 0.1 min
Primary = 5.06 cfs @ 12.15 hrs, Volume= 22,459 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 84.06' @ 12.15 hrs Surf.Area= 845 sf Storage= 70 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= 0.4 min (747.5 - 747.2)

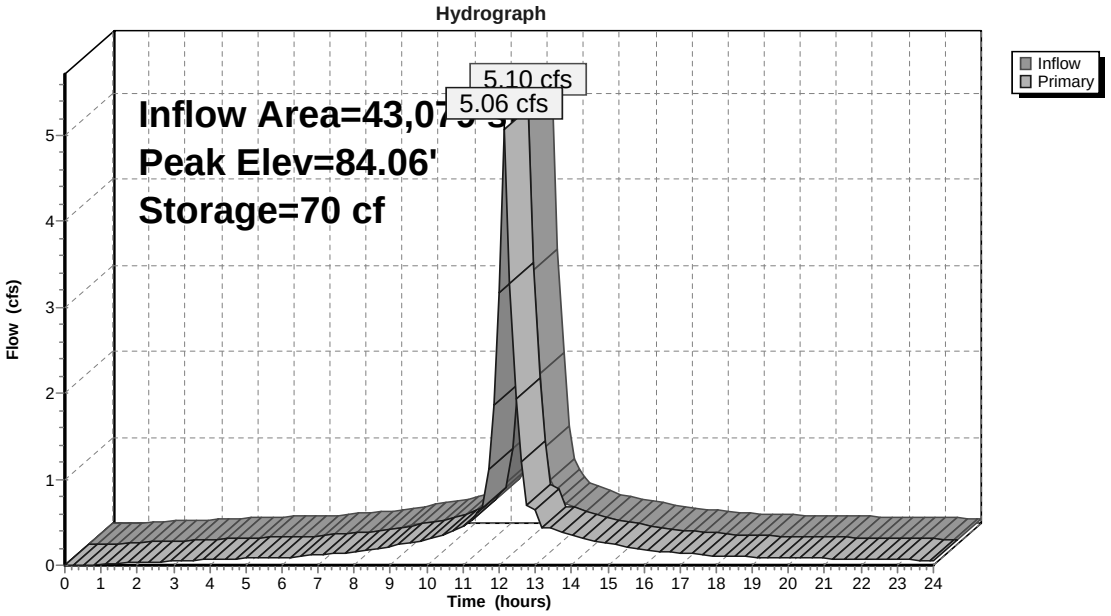
Volume	Invert	Avail.Storage	Storage Description
#1	82.38'	14 cf	**4.00'D x 1.10'H Ex.CB**
#2	83.45'	1,680 cf	**Parking/Driveway (Prismatic)**Listed below (Recalc)
		1,694 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.45	4	0	0
84.00	103	29	29
84.50	6,500	1,651	1,680

Device	Routing	Invert	Outlet Devices
#1	Primary	82.46'	**6.0" Round 6"PVC w/ 1.0" inside fill** L= 170.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.38' / 81.00' S= 0.0081 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.17 sf
#2	Primary	84.00'	**120.0' long x 50.0' breadth Weir Flow Over Curb Towards Lake** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=5.03 cfs @ 12.15 hrs HW=84.06' TW=0.00' (Dynamic Tailwater)
↑—**1=6"PVC** (Barrel Controls 0.68 cfs @ 3.90 fps)
└—**2=Weir Flow Over Curb Towards Lake** (Weir Controls 4.35 cfs @ 0.64 fps)

Pond P-2A: Parking Lot/Driveway



Summary for Pond P-2C: EX. INFILTRTATION BASIN

Inflow Area = 107,187 sf, 67.80% Impervious, Inflow Depth > 5.44" for 100-Year event
Inflow = 11.95 cfs @ 12.15 hrs, Volume= 48,617 cf
Outflow = 11.23 cfs @ 12.18 hrs, Volume= 46,446 cf, Atten= 6%, Lag= 1.5 min
Primary = 11.23 cfs @ 12.18 hrs, Volume= 46,446 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Peak Elev= 83.49' @ 12.18 hrs Surf.Area= 7,399 sf Storage= 3,365 cf

Plug-Flow detention time= 42.8 min calculated for 46,157 cf (95% of inflow)
Center-of-Mass det. time= 19.1 min (800.9 - 781.8)

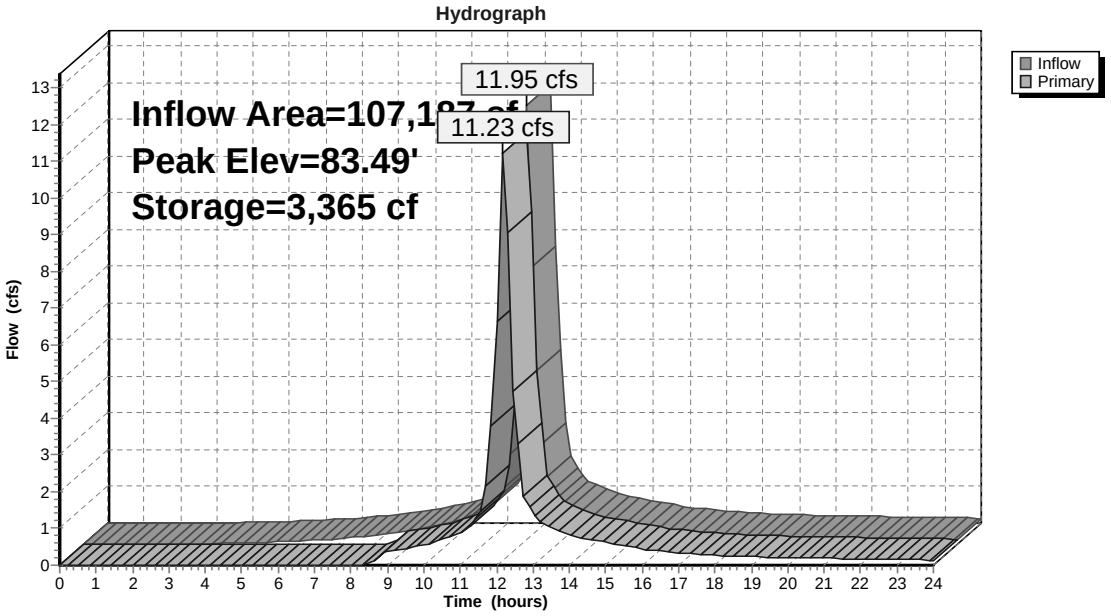
Volume	Invert	Avail.Storage	Storage Description
#1	82.50'	8,308 cf	**EX. INFILTRATION BASIN (Prismatic)**Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.50	430	0	0
83.00	2,900	833	833
84.00	12,050	7,475	8,308

Device	Routing	Invert	Outlet Devices
#1	Primary	83.30'	**50.0' long x 60.0' breadth GRASS/LAWN AREA** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=10.73 cfs @ 12.18 hrs HW=83.49' TW=82.71' (Dynamic Tailwater)
↑**1=GRASS/LAWN AREA** (Weir Controls 10.73 cfs @ 1.16 fps)

Pond P-2C: EX. INFILTRTATION BASIN

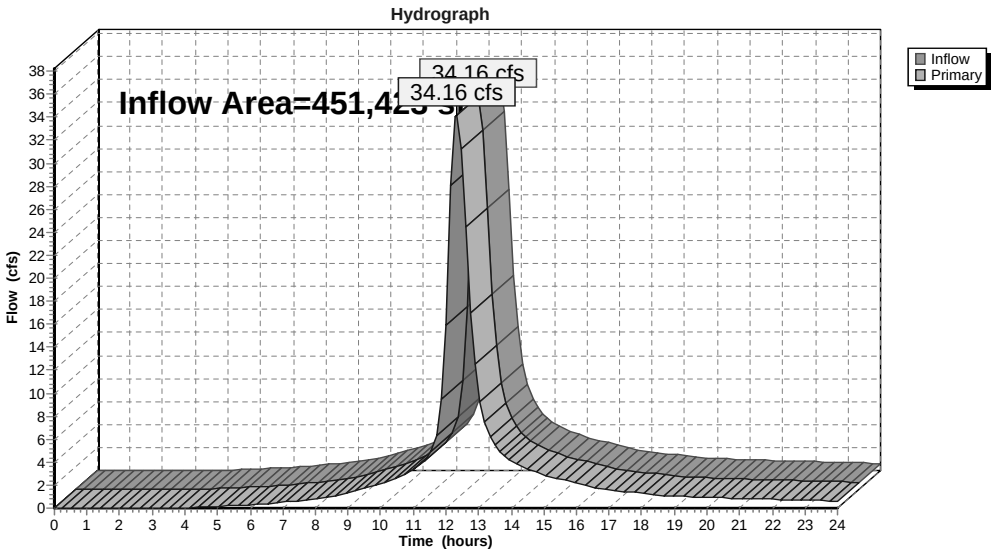


Summary for Link SP-1: STUDY POINT #1

Inflow Area = 451,423 sf, 65.30% Impervious, Inflow Depth > 5.45" for 100-Year event
Inflow = 34.16 cfs @ 12.33 hrs, Volume= 205,156 cf
Primary = 34.16 cfs @ 12.33 hrs, Volume= 205,156 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-1: STUDY POINT #1

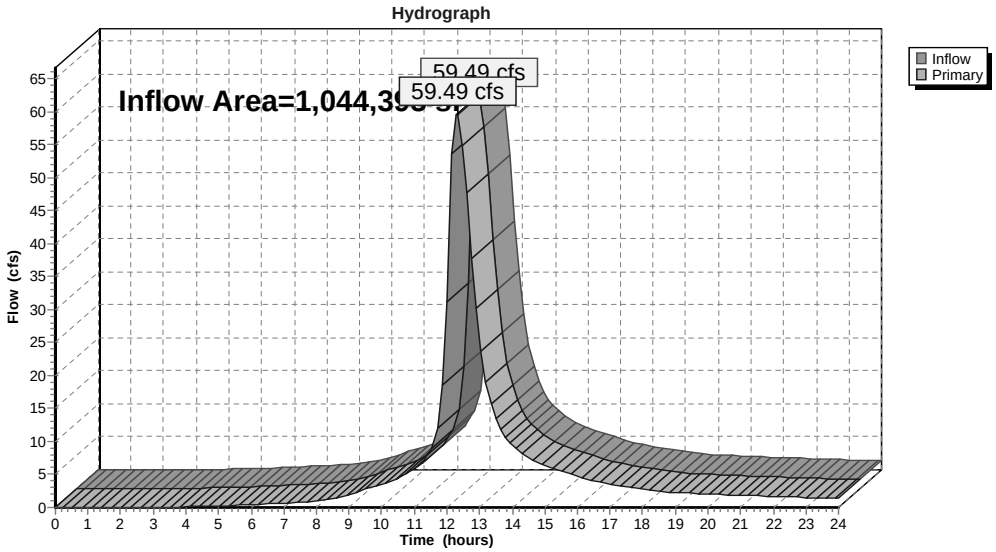


Summary for Link SP-2: STUDY POINT #2

Inflow Area = 1,044,395 sf, 40.73% Impervious, Inflow Depth > 4.72" for 100-Year event
Inflow = 59.49 cfs @ 12.31 hrs, Volume= 410,892 cf
Primary = 59.49 cfs @ 12.31 hrs, Volume= 410,892 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-2: STUDY POINT #2

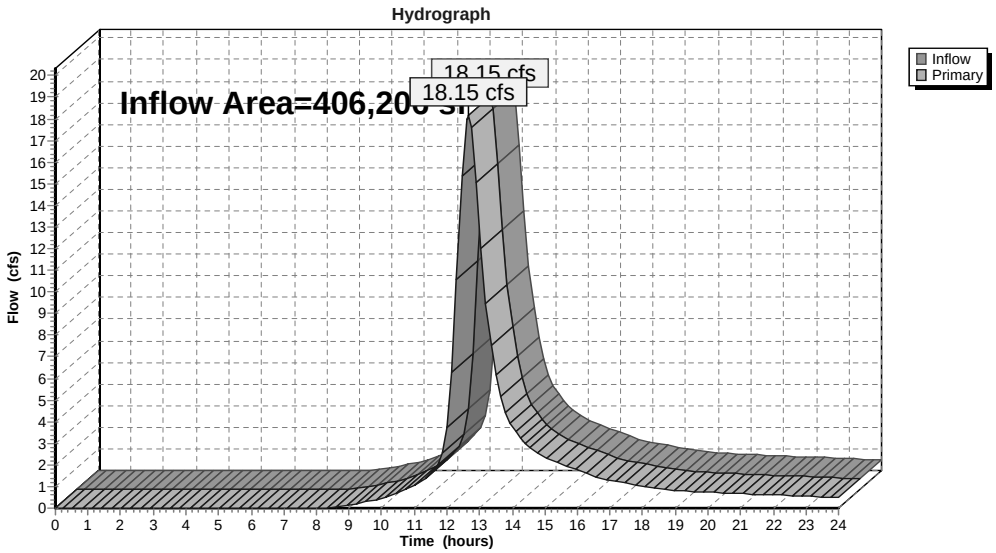


Summary for Link SP-3: STUDY POINT #3

Inflow Area = 406,206 sf, 2.31% Impervious, Inflow Depth > 3.68" for 100-Year event
Inflow = 18.15 cfs @ 12.66 hrs, Volume= 124,528 cf
Primary = 18.15 cfs @ 12.66 hrs, Volume= 124,528 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-3: STUDY POINT #3



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1611-08-Existing Condtions
Prepared by Microsoft
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Printed 3/8/2021

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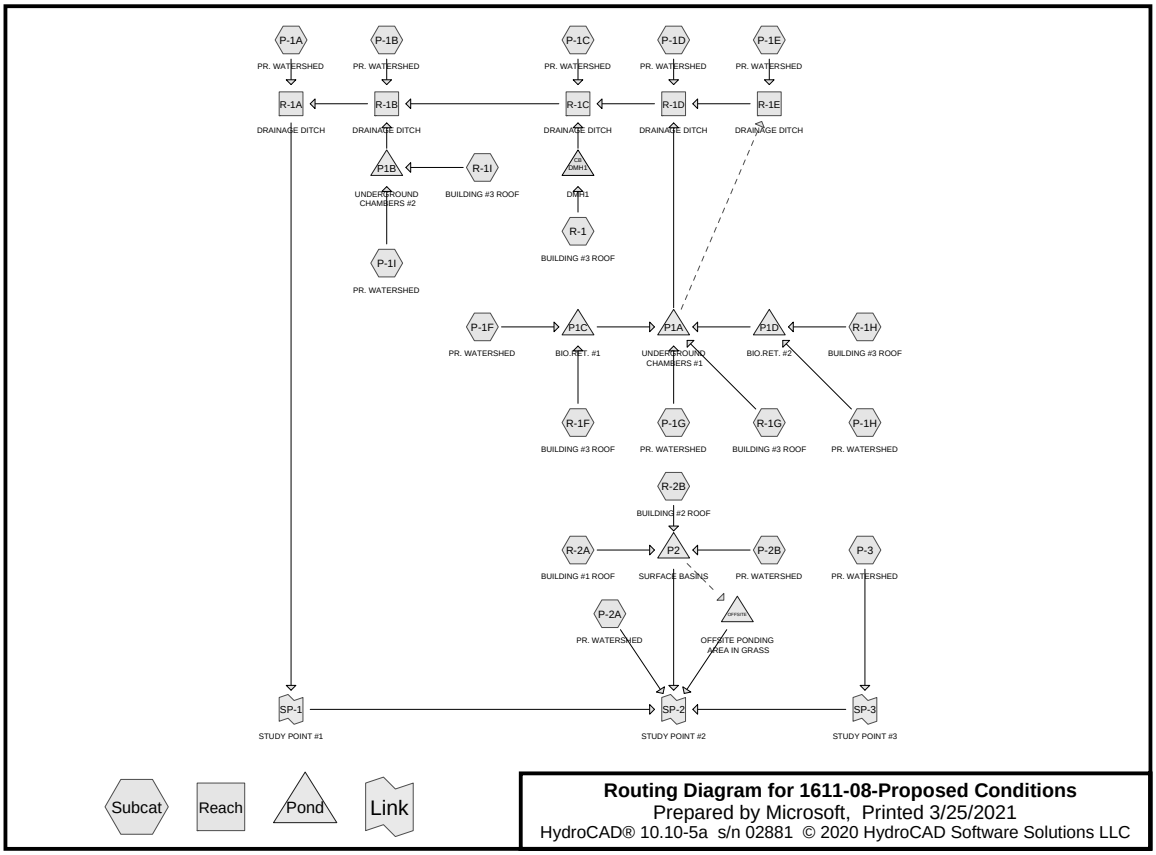
PROPOSED DRAINAGE ANALYSIS



Routing Diagram for 1611-08-Proposed Conditions
Prepared by Microsoft, Printed 3/25/2021
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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Year	Type III 24-hr		Default	24.00	1	3.10	2
2	10-Year	Type III 24-hr		Default	24.00	1	4.50	2
3	100-Year	Type III 24-hr		Default	24.00	1	6.50	2

Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers
0	0	222,347	0	0	222,347	>75% Grass cover, Good	P-1A, P-1B, P-1C, P-1D, P-1E, P-1F, P-1G, P-1H, P-1I, P-2A, P-2B, P-3
0	0	0	0	202,361	202,361	Impervious	P-1A, P-1B, P-1C, P-1D, P-1E, P-1F, P-1G, P-1H, P-1I, P-2A, P-2B, P-3
0	0	0	0	182,015	182,015	Roof	R-1, R-1F, R-1G, R-1H, R-1I, R-2A, R-2B
0	0	155,718	235,711	0	391,429	Woods, Good	P-3
0	0	0	46,247	0	46,247	Woods/grass comb., Good	P-1A, P-1B, P-1C, P-1D
0	**0**	**378,065**	**281,958**	**384,376**	**1,044,399**	**TOTAL AREA**	

Summary for Subcatchment P-1A: PR. WATERSHED

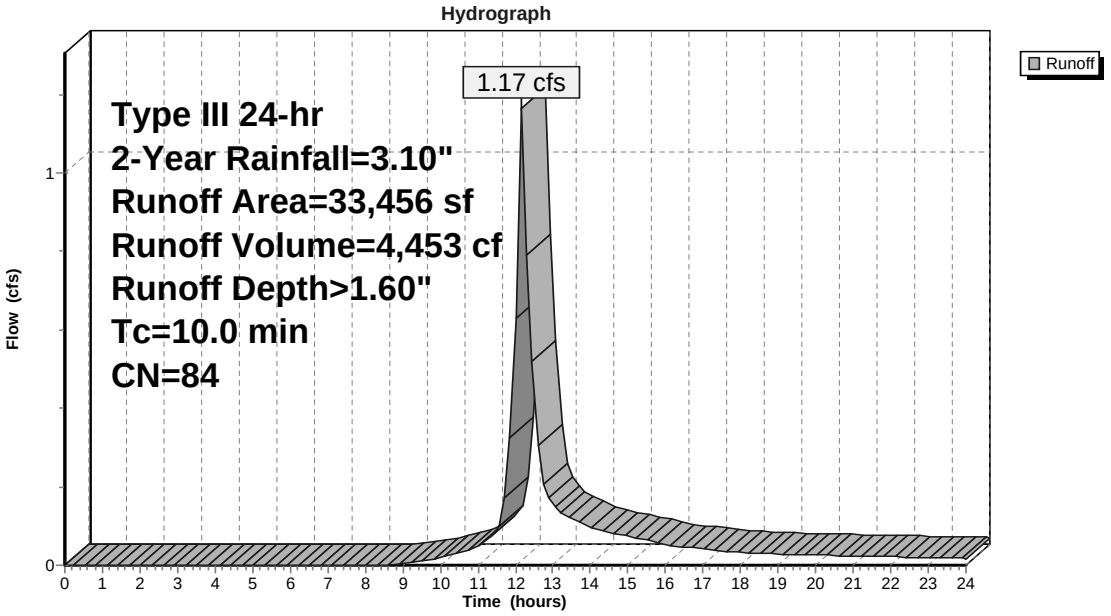
Runoff = 1.17 cfs @ 12.16 hrs, Volume= 4,453 cf, Depth> 1.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	14,724	74	>75% Grass cover, Good, HSG C
	5,389	79	Woods/grass comb., Good, HSG D
*	13,343	98	Impervious
	33,456	84	Weighted Average
	20,113		60.12% Pervious Area
	13,343		39.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1A: PR. WATERSHED



Summary for Subcatchment P-1B: PR. WATERSHED

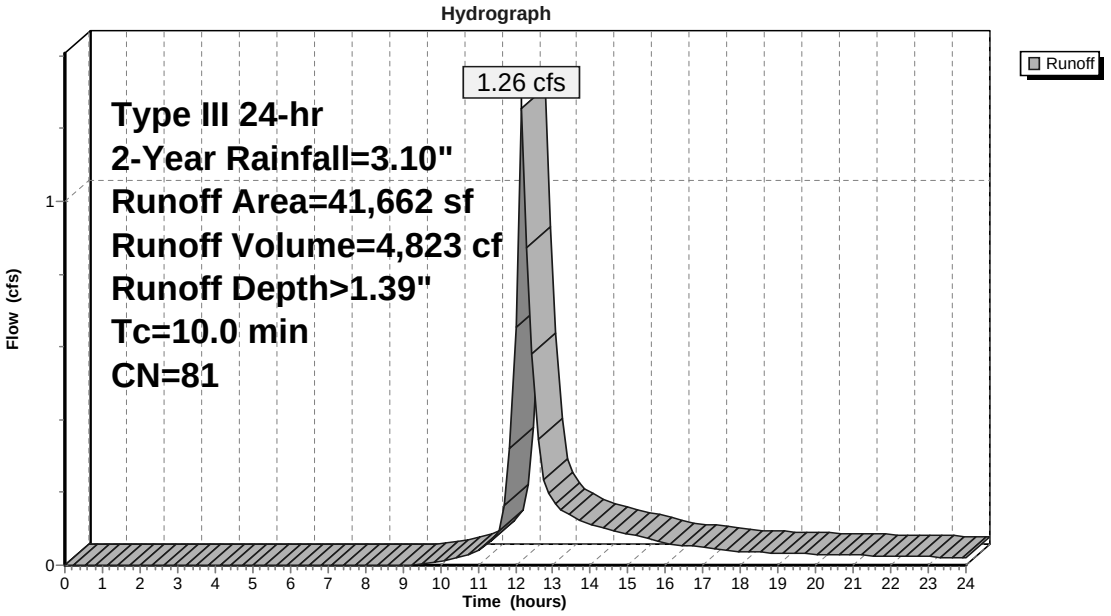
Runoff = 1.26 cfs @ 12.17 hrs, Volume= 4,823 cf, Depth> 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	17,448	74	>75% Grass cover, Good, HSG C
	15,311	79	Woods/grass comb., Good, HSG D
*	8,903	98	Impervious
	41,662	81	Weighted Average
	32,759		78.63% Pervious Area
	8,903		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1B: PR. WATERSHED



Summary for Subcatchment P-1C: PR. WATERSHED

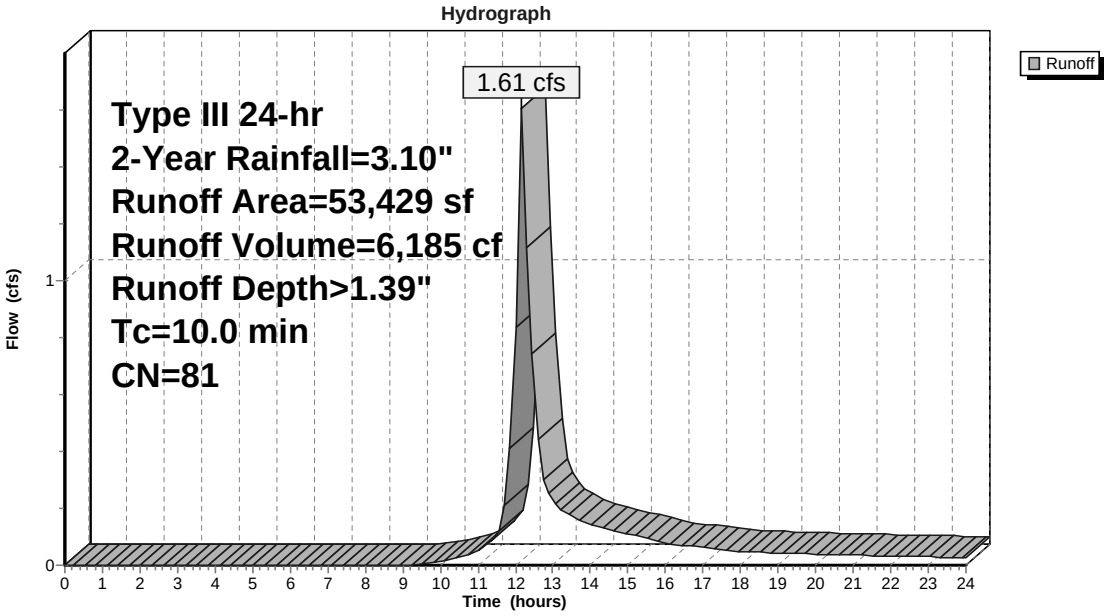
Runoff = 1.61 cfs @ 12.17 hrs, Volume= 6,185 cf, Depth> 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	20,039	74	>75% Grass cover, Good, HSG C
	22,761	79	Woods/grass comb., Good, HSG D
*	10,629	98	Impervious
	53,429	81	Weighted Average
	42,800		80.11% Pervious Area
	10,629		19.89% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1C: PR. WATERSHED



Summary for Subcatchment P-1D: PR. WATERSHED

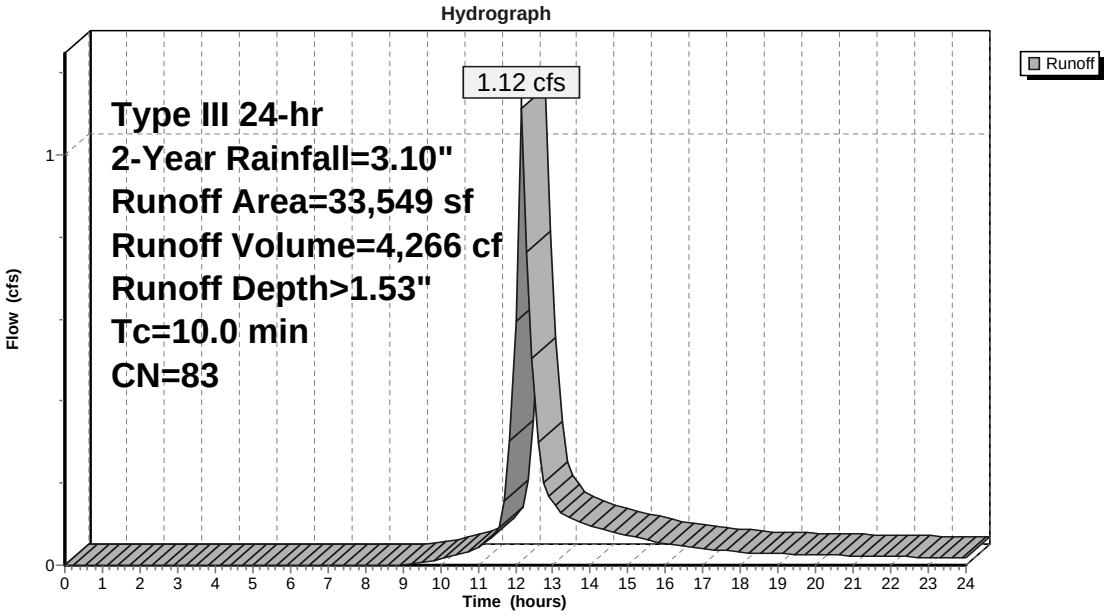
Runoff = 1.12 cfs @ 12.16 hrs, Volume= 4,266 cf, Depth> 1.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	19,032	74	>75% Grass cover, Good, HSG C
	2,786	79	Woods/grass comb., Good, HSG D
*	11,731	98	Impervious
	33,549	83	Weighted Average
	21,818		65.03% Pervious Area
	11,731		34.97% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1D: PR. WATERSHED



Summary for Subcatchment P-1E: PR. WATERSHED

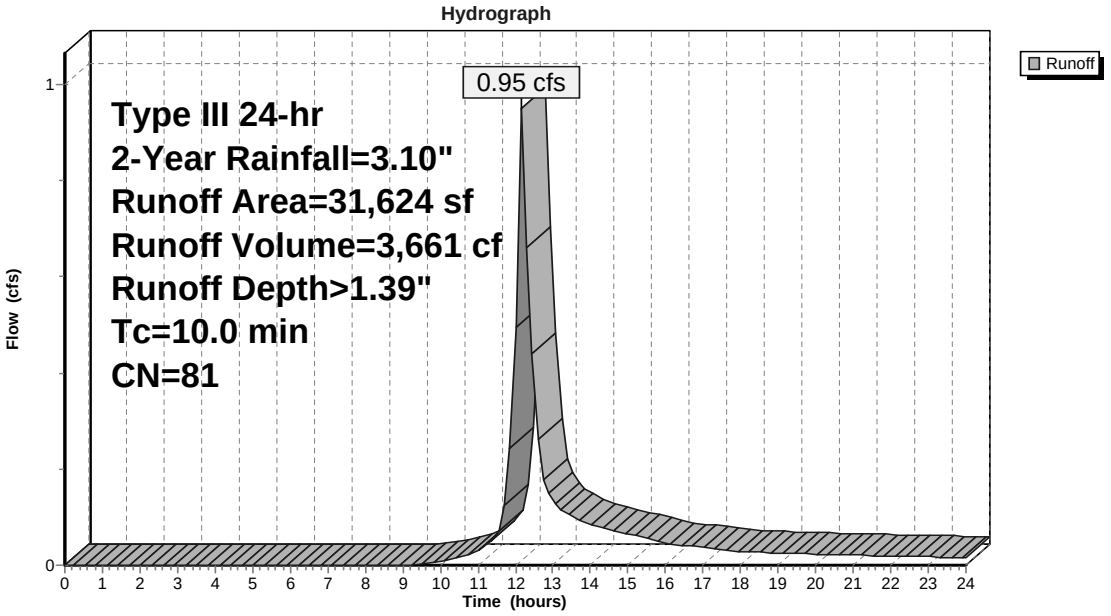
Runoff = 0.95 cfs @ 12.17 hrs, Volume= 3,661 cf, Depth> 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	22,158	74	>75% Grass cover, Good, HSG C
*	9,466	98	Impervious
	31,624	81	Weighted Average
	22,158		70.07% Pervious Area
	9,466		29.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1E: PR. WATERSHED



Summary for Subcatchment P-1F: PR. WATERSHED

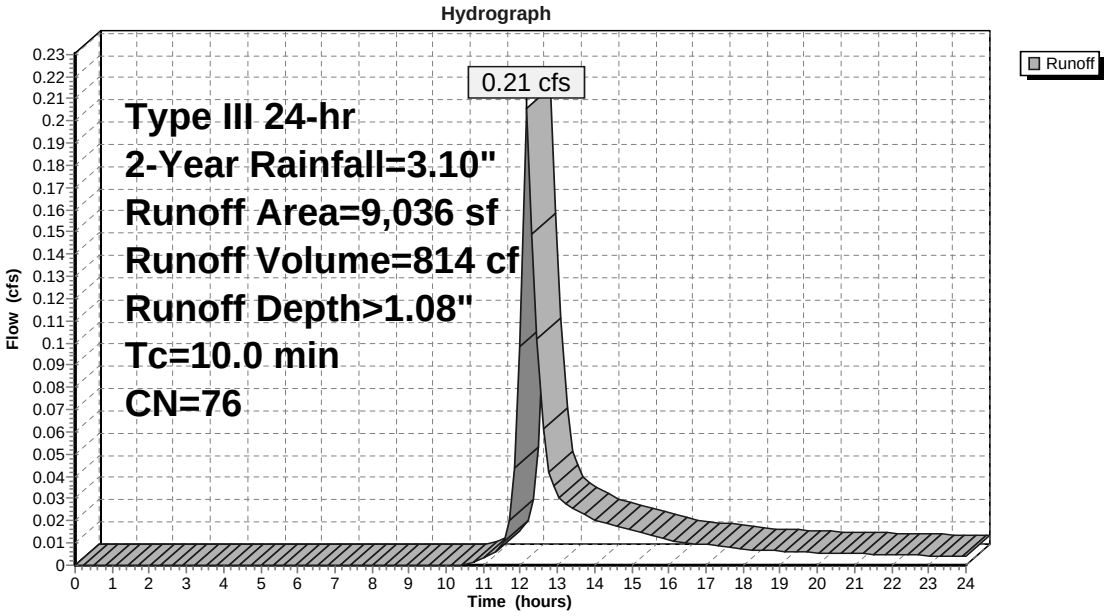
Runoff = 0.21 cfs @ 12.17 hrs, Volume= 814 cf, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	8,156	74	>75% Grass cover, Good, HSG C
*	880	98	Impervious
	9,036	76	Weighted Average
	8,156		90.26% Pervious Area
	880		9.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1F: PR. WATERSHED



Summary for Subcatchment P-1G: PR. WATERSHED

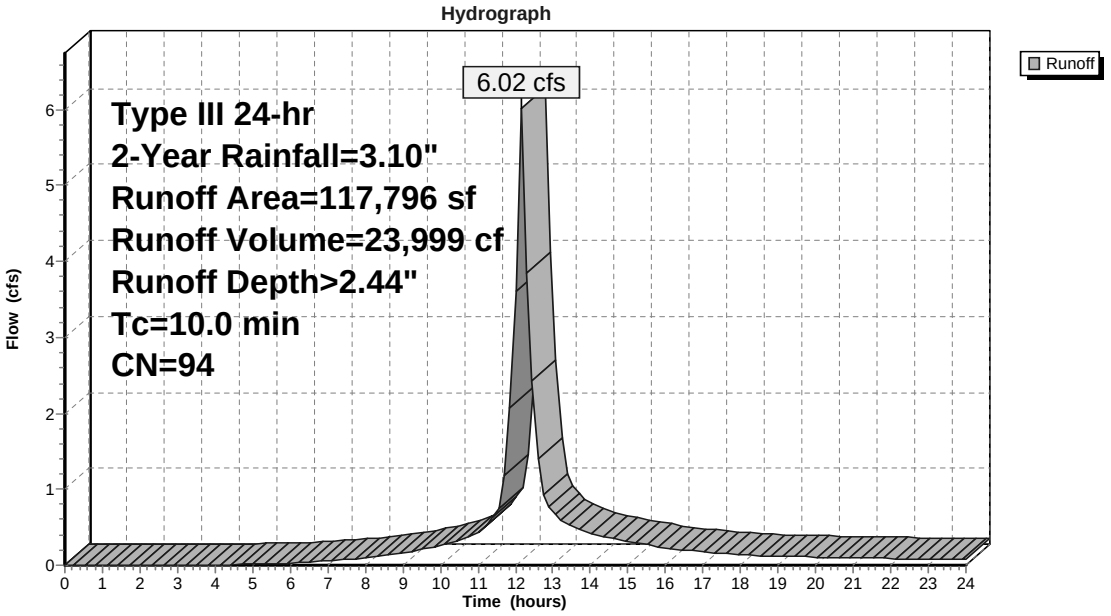
Runoff = 6.02 cfs @ 12.15 hrs, Volume= 23,999 cf, Depth> 2.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	21,862	74	>75% Grass cover, Good, HSG C
*	95,934	98	Impervious
	117,796	94	Weighted Average
	21,862		18.56% Pervious Area
	95,934		81.44% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1G: PR. WATERSHED



Summary for Subcatchment P-1H: PR. WATERSHED

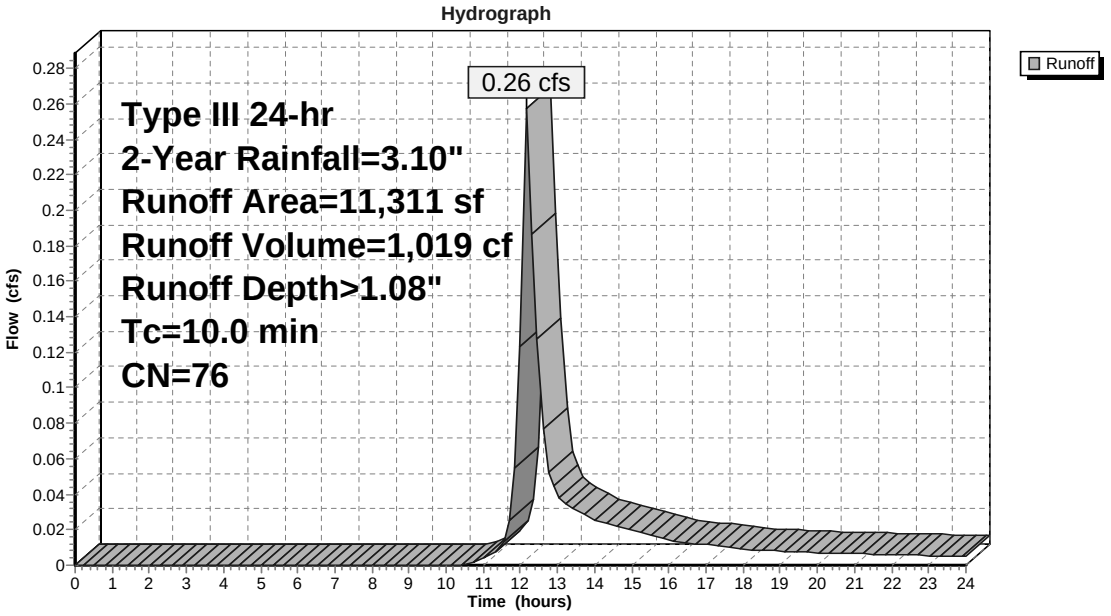
Runoff = 0.26 cfs @ 12.17 hrs, Volume= 1,019 cf, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	10,351	74	>75% Grass cover, Good, HSG C
*	960	98	Impervious
	11,311	76	Weighted Average
	10,351		91.51% Pervious Area
	960		8.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1H: PR. WATERSHED



Summary for Subcatchment P-1I: PR. WATERSHED

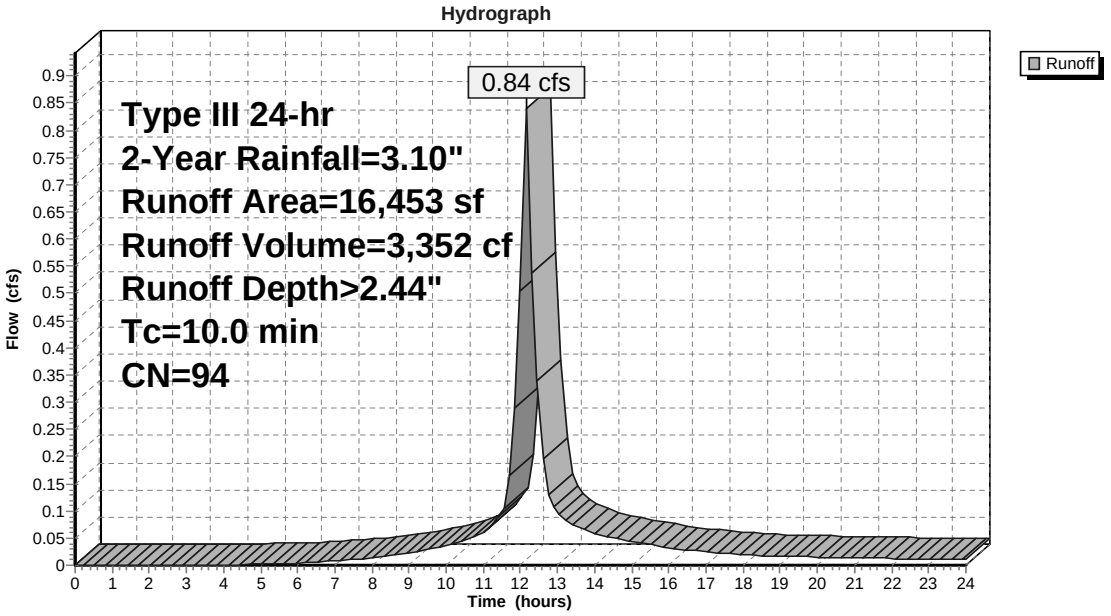
Runoff = 0.84 cfs @ 12.15 hrs, Volume= 3,352 cf, Depth> 2.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	2,946	74	>75% Grass cover, Good, HSG C
*	13,507	98	Impervious
	16,453	94	Weighted Average
	2,946		17.91% Pervious Area
	13,507		82.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1I: PR. WATERSHED



Summary for Subcatchment P-2A: PR. WATERSHED

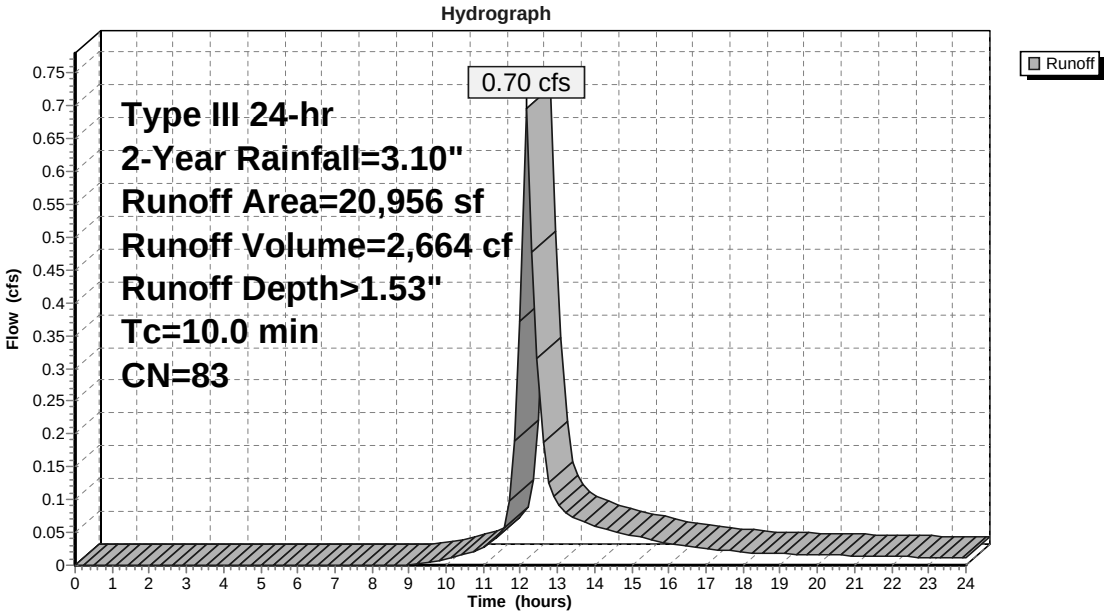
Runoff = 0.70 cfs @ 12.16 hrs, Volume= 2,664 cf, Depth> 1.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	12,753	74	>75% Grass cover, Good, HSG C
*	8,203	98	Impervious
	20,956	83	Weighted Average
	12,753		60.86% Pervious Area
	8,203		39.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-2A: PR. WATERSHED



Summary for Subcatchment P-2B: PR. WATERSHED

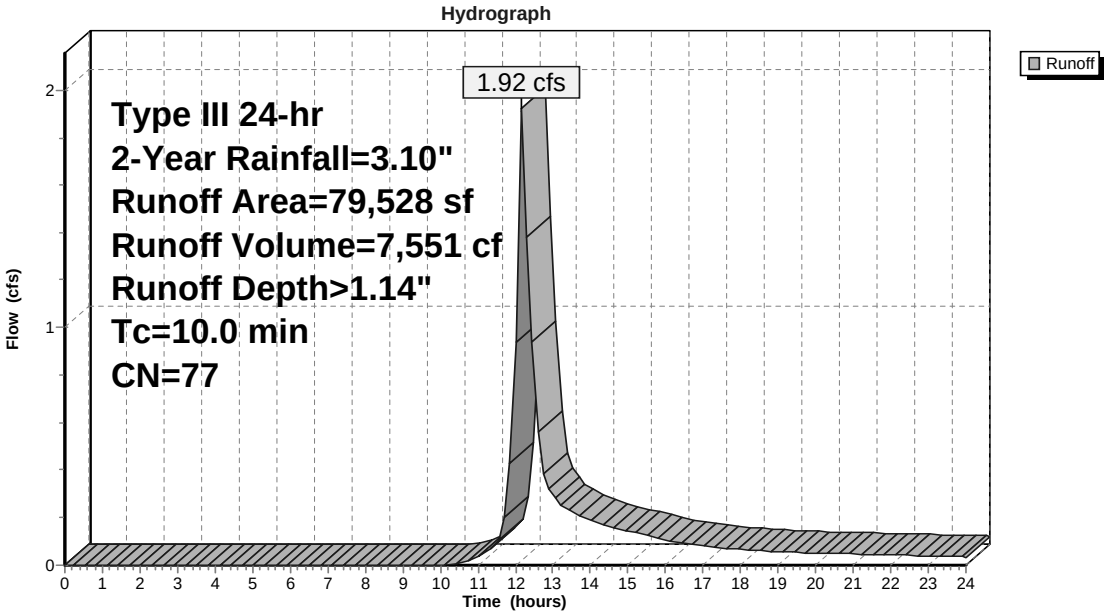
Runoff = 1.92 cfs @ 12.17 hrs, Volume= 7,551 cf, Depth> 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	69,830	74	>75% Grass cover, Good, HSG C
*	9,698	98	Impervious
	79,528	77	Weighted Average
	69,830		87.81% Pervious Area
	9,698		12.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-2B: PR. WATERSHED



Summary for Subcatchment P-3: PR. WATERSHED

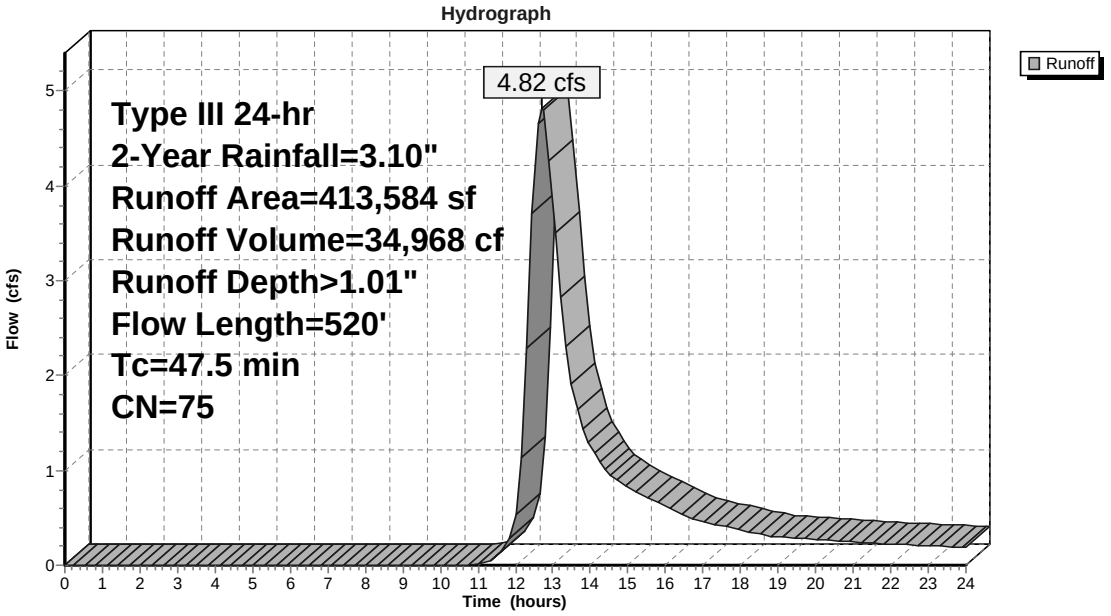
Runoff = 4.82 cfs @ 12.71 hrs, Volume= 34,968 cf, Depth> 1.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
	3,048	74	>75% Grass cover, Good, HSG C
	235,711	77	Woods, Good, HSG D
*	19,107	98	Impervious
	155,718	70	Woods, Good, HSG C
	413,584	75	Weighted Average
	394,477		95.38% Pervious Area
	19,107		4.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	100	0.0100	0.06		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
1.3	40	0.0100	0.50		**Shallow Concentrated Flow, B-C** Woodland Kv= 5.0 fps
17.9	380	0.0050	0.35		**Shallow Concentrated Flow, C-D** Woodland Kv= 5.0 fps
47.5	520	Total			

Subcatchment P-3: PR. WATERSHED



Summary for Subcatchment R-1: BUILDING #3 ROOF

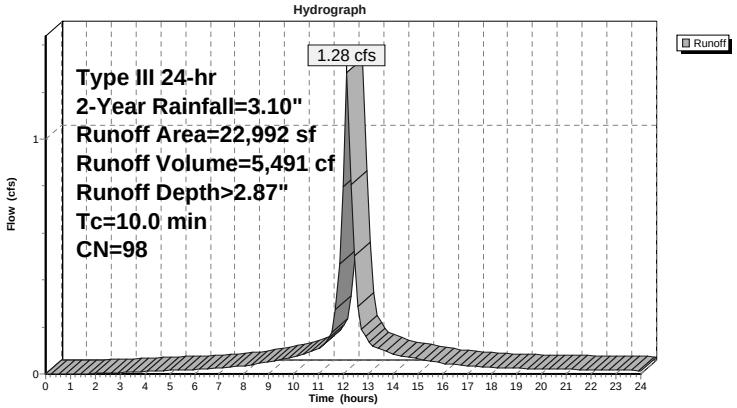
Runoff = 1.28 cfs @ 12.15 hrs, Volume= 5,491 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
*	22,992	98	Roof
	22,992		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1: BUILDING #3 ROOF



Summary for Subcatchment R-1F: BUILDING #3 ROOF

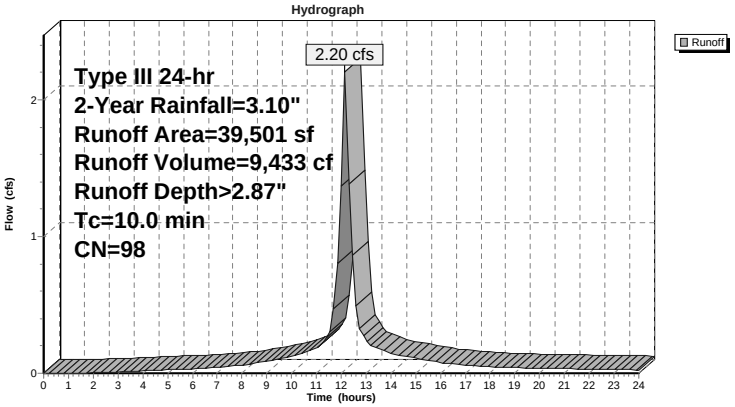
Runoff = 2.20 cfs @ 12.15 hrs, Volume= 9,433 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
*	39,501	98	Roof
	39,501		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1F: BUILDING #3 ROOF



Summary for Subcatchment R-1G: BUILDING #3 ROOF

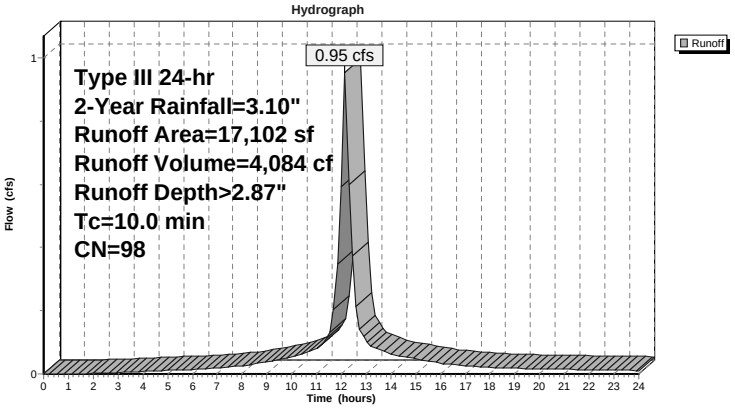
Runoff = 0.95 cfs @ 12.15 hrs, Volume= 4,084 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
*	17,102	98	Roof
	17,102		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1G: BUILDING #3 ROOF



Summary for Subcatchment R-1H: BUILDING #3 ROOF

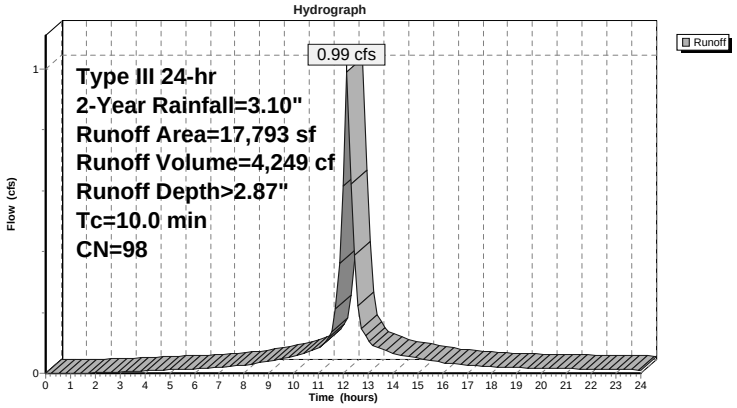
Runoff = 0.99 cfs @ 12.15 hrs, Volume= 4,249 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
*	17,793	98	Roof
	17,793		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1H: BUILDING #3 ROOF



Summary for Subcatchment R-1I: BUILDING #3 ROOF

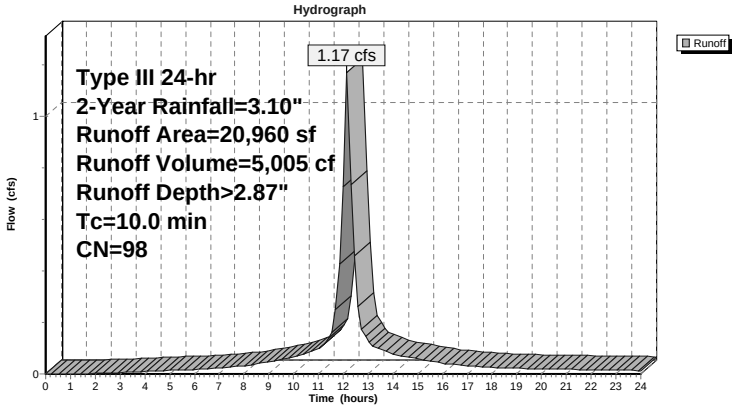
Runoff = 1.17 cfs @ 12.15 hrs, Volume= 5,005 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
*	20,960	98	Roof
	20,960		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1I: BUILDING #3 ROOF



Summary for Subcatchment R-2A: BUILDING #1 ROOF

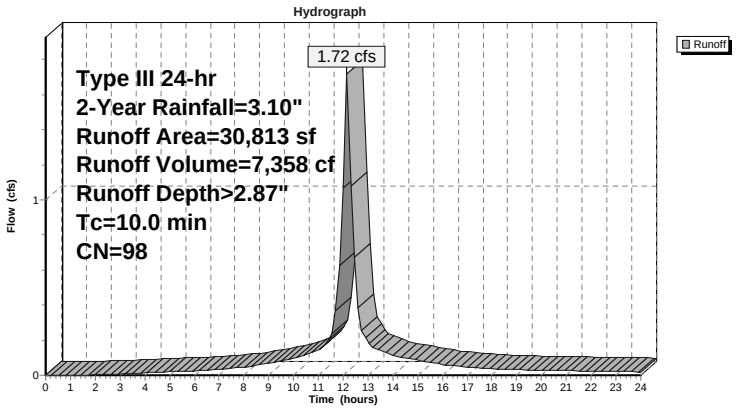
Runoff = 1.72 cfs @ 12.15 hrs, Volume= 7,358 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
*	30,813	98	Roof
	30,813		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-2A: BUILDING #1 ROOF



Summary for Subcatchment R-2B: BUILDING #2 ROOF

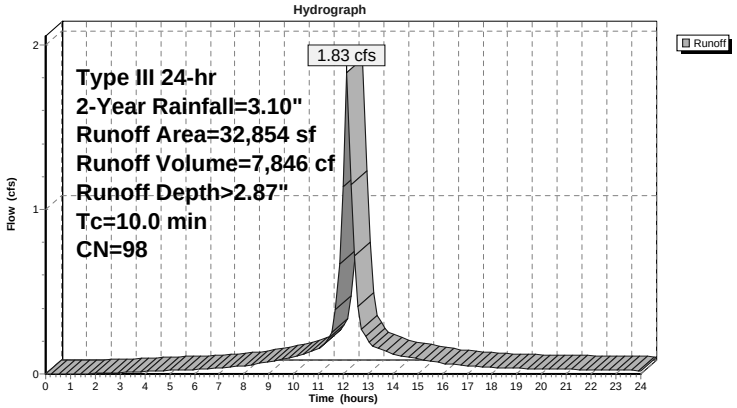
Runoff = 1.83 cfs @ 12.15 hrs, Volume= 7,846 cf, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description
*	32,854	98	Roof
	32,854		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-2B: BUILDING #2 ROOF



Summary for Reach R-1A: DRAINAGE DITCH

Inflow Area = 466,664 sf, 60.79% Impervious, Inflow Depth > 1.51" for 2-Year event
Inflow = 6.12 cfs @ 12.79 hrs, Volume= 58,638 cf
Outflow = 6.10 cfs @ 12.81 hrs, Volume= 58,579 cf, Atten= 0%, Lag= 1.4 min

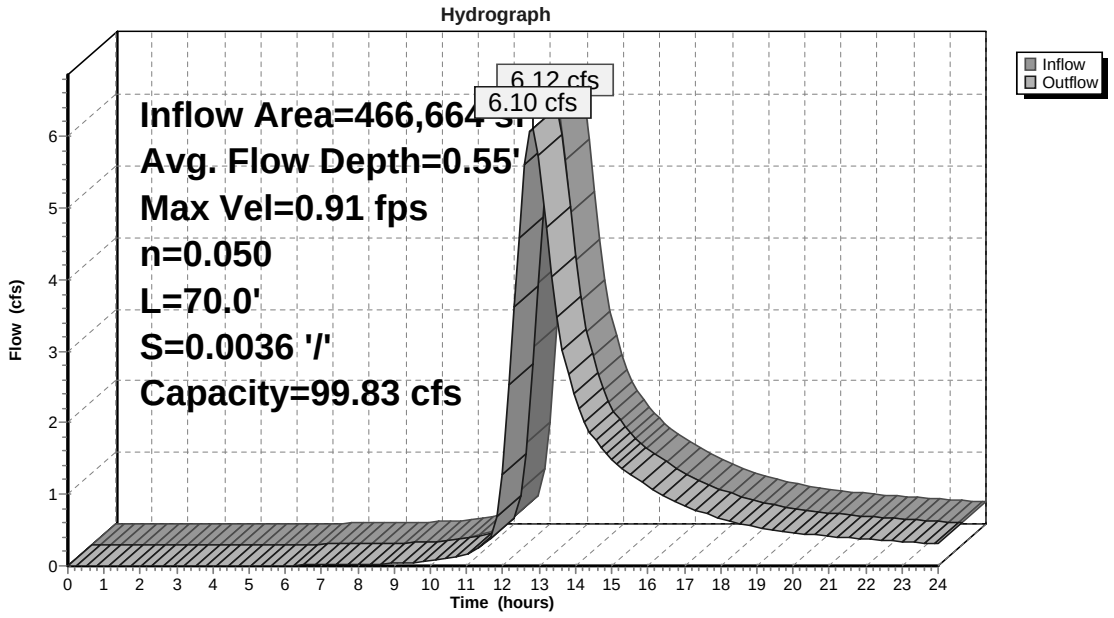
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.91 fps, Min. Travel Time= 1.3 min
Avg. Velocity = 0.36 fps, Avg. Travel Time= 3.2 min

Peak Storage= 470 cf @ 12.81 hrs
Average Depth at Peak Storage= 0.55' , Surface Width= 18.34'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 99.83 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 70.0' Slope= 0.0036 '/'
Inlet Invert= 78.75', Outlet Invert= 78.50'

‡

Reach R-1A: DRAINAGE DITCH



Summary for Reach R-1B: DRAINAGE DITCH

Inflow Area = 433,208 sf, 62.41% Impervious, Inflow Depth > 1.51" for 2-Year event
Inflow = 6.18 cfs @ 12.66 hrs, Volume= 54,661 cf
Outflow = 5.93 cfs @ 12.80 hrs, Volume= 54,186 cf, Atten= 4%, Lag= 8.1 min

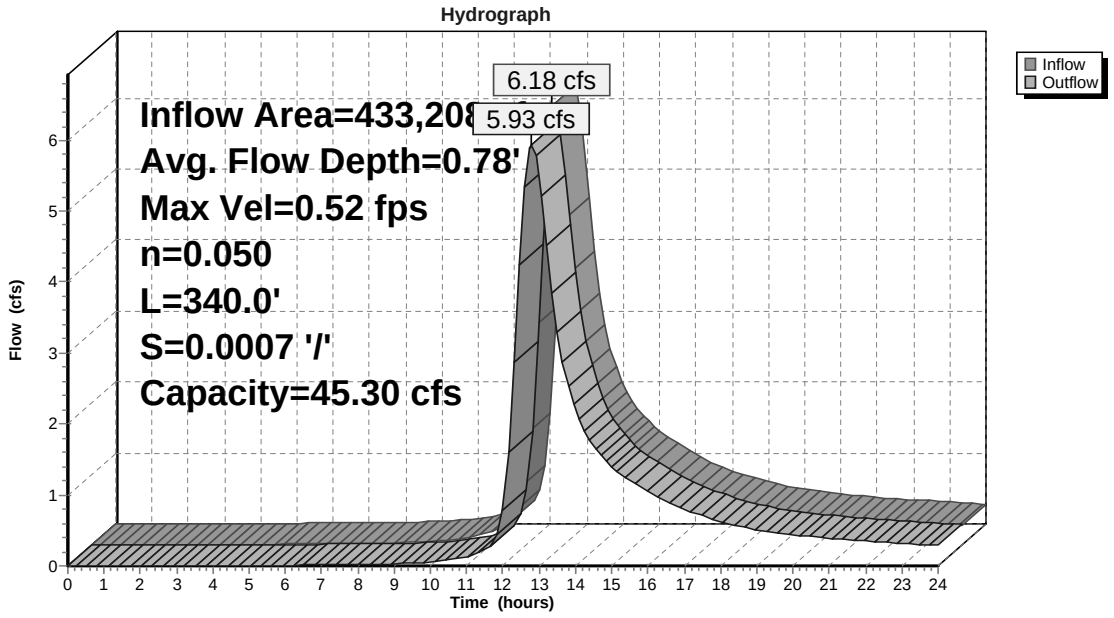
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.52 fps, Min. Travel Time= 10.9 min
Avg. Velocity = 0.20 fps, Avg. Travel Time= 28.5 min

Peak Storage= 3,875 cf @ 12.80 hrs
Average Depth at Peak Storage= 0.78' , Surface Width= 21.88'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 45.30 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 340.0' Slope= 0.0007 '/'
Inlet Invert= 79.00', Outlet Invert= 78.75'

‡

Reach R-1B: DRAINAGE DITCH



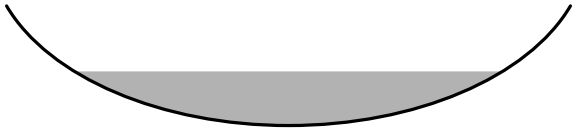
Summary for Reach R-1C: DRAINAGE DITCH

Inflow Area = 354,133 sf, 64.10% Impervious, Inflow Depth > 1.55" for 2-Year event
Inflow = 5.97 cfs @ 12.56 hrs, Volume= 45,817 cf
Outflow = 5.33 cfs @ 12.73 hrs, Volume= 45,277 cf, Atten= 11%, Lag= 10.0 min

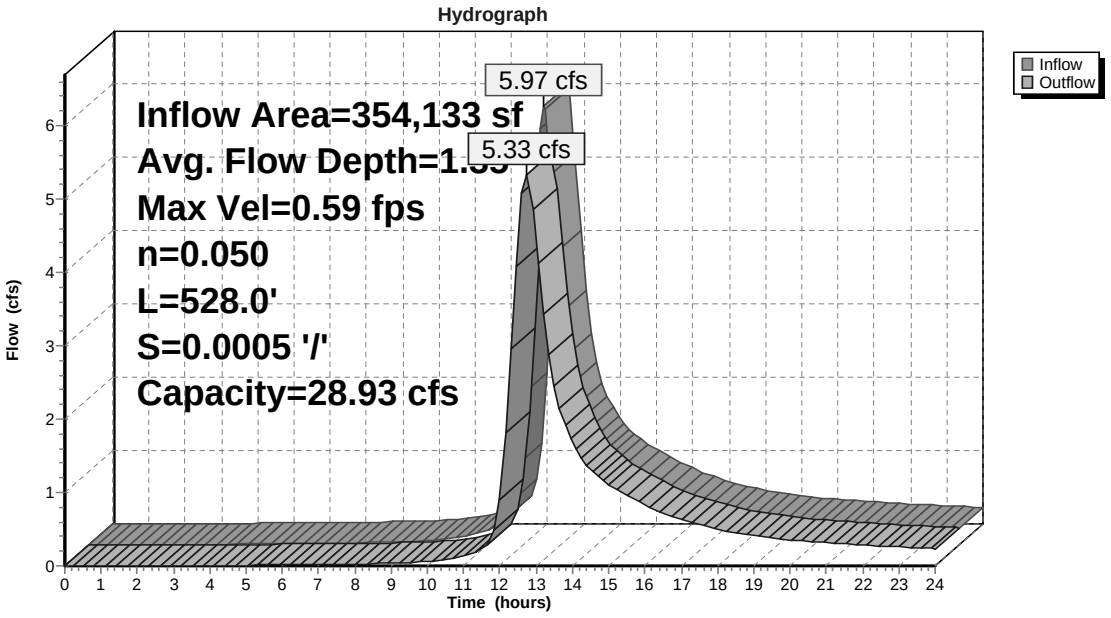
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.59 fps, Min. Travel Time= 15.0 min
Avg. Velocity = 0.22 fps, Avg. Travel Time= 39.3 min

Peak Storage= 4,801 cf @ 12.73 hrs
Average Depth at Peak Storage= 1.35' , Surface Width= 10.08'
Bank-Full Depth= 3.00' Flow Area= 30.0 sf, Capacity= 28.93 cfs

15.00' x 3.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0005 '/'
Inlet Invert= 79.25', Outlet Invert= 79.00'



Reach R-1C: DRAINAGE DITCH



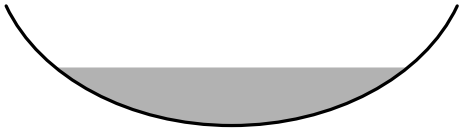
Summary for Reach R-1D: DRAINAGE DITCH

Inflow Area = 277,712 sf, 69.63% Impervious, Inflow Depth > 1.49" for 2-Year event
Inflow = 5.82 cfs @ 12.47 hrs, Volume= 34,425 cf
Outflow = 5.20 cfs @ 12.61 hrs, Volume= 34,141 cf, Atten= 11%, Lag= 8.2 min

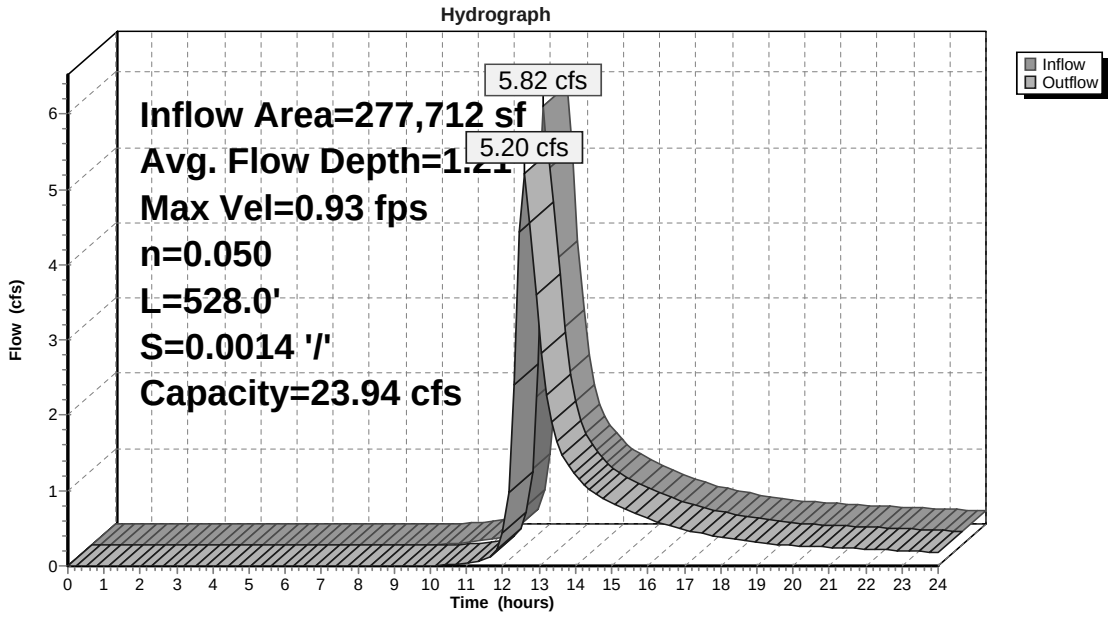
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.93 fps, Min. Travel Time= 9.5 min
Avg. Velocity = 0.41 fps, Avg. Travel Time= 21.5 min

Peak Storage= 2,968 cf @ 12.61 hrs
Average Depth at Peak Storage= 1.21' , Surface Width= 6.96'
Bank-Full Depth= 2.50' Flow Area= 16.7 sf, Capacity= 23.94 cfs

10.00' x 2.50' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0014 '/'
Inlet Invert= 80.00', Outlet Invert= 79.25'



Reach R-1D: DRAINAGE DITCH



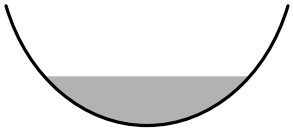
Summary for Reach R-1E: DRAINAGE DITCH

Inflow Area = 31,624 sf, 29.93% Impervious, Inflow Depth > 5.35" for 2-Year event
Inflow = 2.26 cfs @ 12.44 hrs, Volume= 14,094 cf
Outflow = 2.23 cfs @ 12.50 hrs, Volume= 14,037 cf, Atten= 1%, Lag= 3.7 min

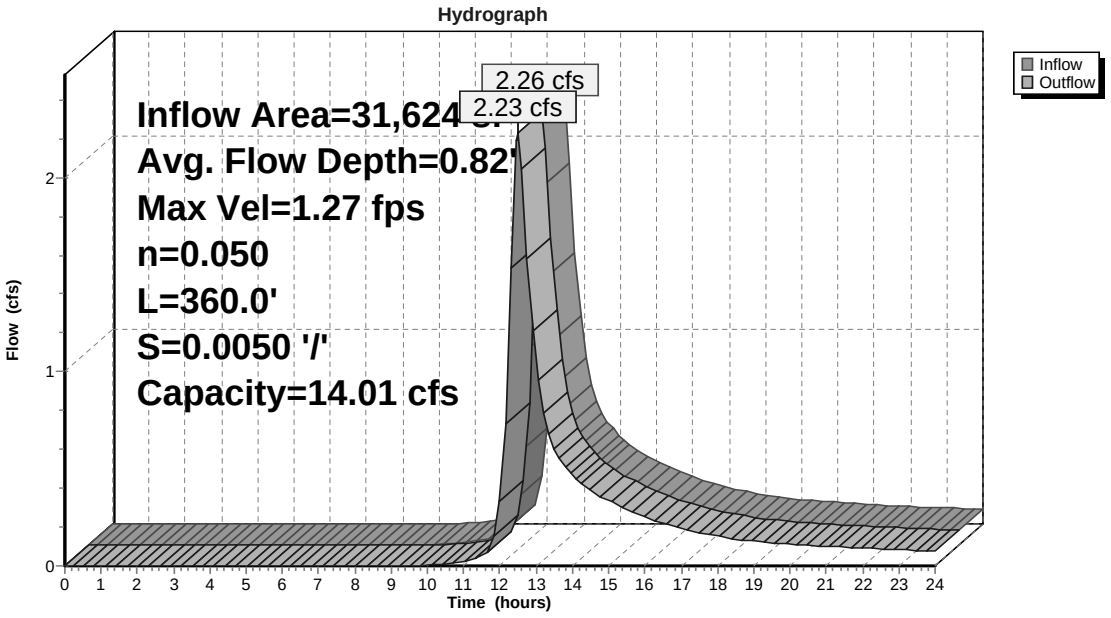
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 1.27 fps, Min. Travel Time= 4.7 min
Avg. Velocity = 0.59 fps, Avg. Travel Time= 10.2 min

Peak Storage= 629 cf @ 12.50 hrs
Average Depth at Peak Storage= 0.82' , Surface Width= 3.20'
Bank-Full Depth= 2.00' Flow Area= 6.7 sf, Capacity= 14.01 cfs

5.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 360.0' Slope= 0.0050 '/'
Inlet Invert= 81.80', Outlet Invert= 80.00'



Reach R-1E: DRAINAGE DITCH



Summary for Pond DMH1: DMH1

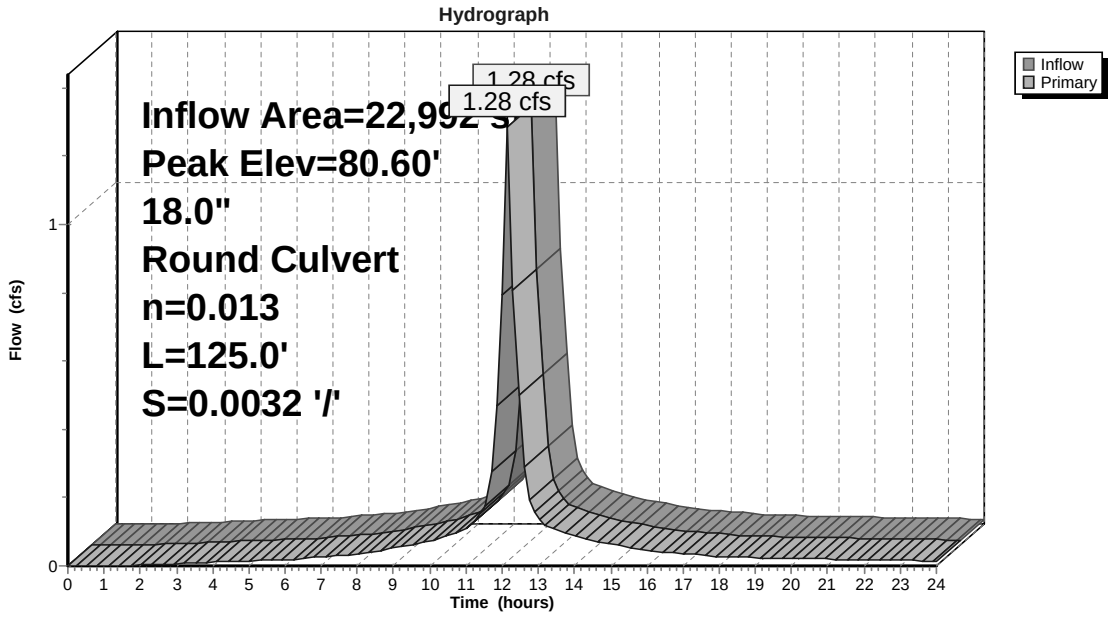
Inflow Area = 22,992 sf,100.00% Impervious, Inflow Depth > 2.87" for 2-Year event
Inflow = 1.28 cfs @ 12.15 hrs, Volume= 5,491 cf
Outflow = 1.28 cfs @ 12.15 hrs, Volume= 5,491 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.28 cfs @ 12.15 hrs, Volume= 5,491 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 80.60' @ 12.72 hrs
Flood Elev= 84.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	79.20'	**18.0" Round EX. 18" VCC** L= 125.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 79.20' / 78.80' S= 0.0032 '/' Cc= 0.900 n= 0.013 Clay tile, Flow Area= 1.77 sf

Primary OutFlow Max=1.28 cfs @ 12.15 hrs HW=80.20' TW=80.07' (Dynamic Tailwater)
1=EX. 18" VCC (Outlet Controls 1.28 cfs @ 1.44 fps)

Pond DMH1: DMH1



Summary for Pond OFFSITE: OFFSITE PONDING AREA IN GRASS

Inflow = 0.13 cfs @ 17.17 hrs, Volume= 2,603 cf
Outflow = 0.12 cfs @ 17.46 hrs, Volume= 2,422 cf, Atten= 1%, Lag= 17.6 min
Primary = 0.12 cfs @ 17.46 hrs, Volume= 2,422 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 82.36' @ 17.46 hrs Surf.Area= 1,588 sf Storage= 233 cf

Plug-Flow detention time= 37.8 min calculated for 2,422 cf (93% of inflow)
Center-of-Mass det. time= 19.3 min (1,190.0 - 1,170.7)

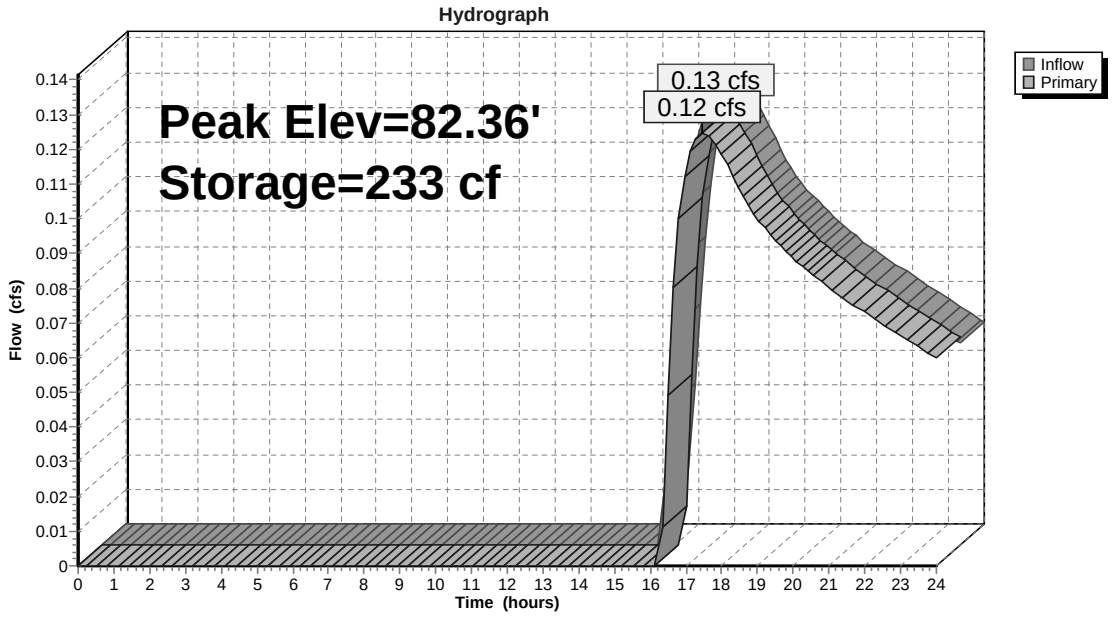
Volume	Invert	Avail.Storage	Storage Description
#1	82.10'	2,354 cf	**OFFSITE PONDING AREA (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.10	230	0	0
83.00	5,000	2,354	2,354

Device	Routing	Invert	Outlet Devices
#1	Primary	82.27'	**8.0" Round (3) 8" HDPE X 3.00 w/ 2.0" inside fill** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.10' / 81.40' S= 0.0333 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.28 sf
#2	Primary	82.60'	**100.0' long x 20.0' breadth WEIR FLOW OVER WALKING PATH** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=0.12 cfs @ 17.46 hrs HW=82.36' TW=0.00' (Dynamic Tailwater)
└─**1=(3) 8" HDPE** (Inlet Controls 0.12 cfs @ 0.75 fps)
└─**2=WEIR FLOW OVER WALKING PATH** (Controls 0.00 cfs)

Pond OFFSITE: OFFSITE PONDING AREA IN GRASS



Summary for Pond P1A: UNDERGROUND CHAMBERS #1

Inflow Area = 212,539 sf, 81.01% Impervious, Inflow Depth > 2.25" for 2-Year event
Inflow = 9.95 cfs @ 12.17 hrs, Volume= 39,883 cf
Outflow = 4.97 cfs @ 12.48 hrs, Volume= 26,555 cf, Atten= 50%, Lag= 18.5 min
Primary = 3.14 cfs @ 12.48 hrs, Volume= 16,123 cf
Secondary = 1.83 cfs @ 12.48 hrs, Volume= 10,433 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 84.87' @ 12.48 hrs Surf.Area= 14,913 sf Storage= 18,126 cf
Flood Elev= 84.00' Surf.Area= 14,913 sf Storage= 12,169 cf

Plug-Flow detention time= 184.4 min calculated for 26,390 cf (66% of inflow)
Center-of-Mass det. time= 91.3 min (895.3 - 804.0)

Volume	Invert	Avail.Storage	Storage Description
#1B	82.64'	5,589 cf	**8.17'W x 935.92'L x 2.33'H Field B** 17,834 cf Overall - 3,862 cf Embedded = 13,972 cf x 40.0% Voids
#2B	83.14'	3,862 cf	**ADS_StormTech SC-310 +Cap** x 262 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 262 Chambers in 2 Rows
#3C	82.64'	2,519 cf	**34.83'W x 102.88'L x 2.33'H Field C** 8,362 cf Overall - 2,064 cf Embedded = 6,298 cf x 40.0% Voids
#4C	83.14'	2,064 cf	**ADS_StormTech SC-310 +Cap** x 140 Inside #3 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 140 Chambers in 10 Rows
#5D	82.64'	2,002 cf	**34.83'W x 81.52'L x 2.33'H Field D** 6,626 cf Overall - 1,622 cf Embedded = 5,004 cf x 40.0% Voids
#6D	83.14'	1,622 cf	**ADS_StormTech SC-310 +Cap** x 110 Inside #5 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 110 Chambers in 10 Rows
#7E	82.64'	602 cf	**21.50'W x 38.80'L x 2.33'H Field E** 1,946 cf Overall - 442 cf Embedded = 1,504 cf x 40.0% Voids
#8E	83.14'	442 cf	**ADS_StormTech SC-310 +Cap** x 30 Inside #7

			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 30 Chambers in 6 Rows
#9	83.14'	50 cf	**4.00'D x 4.00'H DMH**
		18,752 cf	Total Available Storage

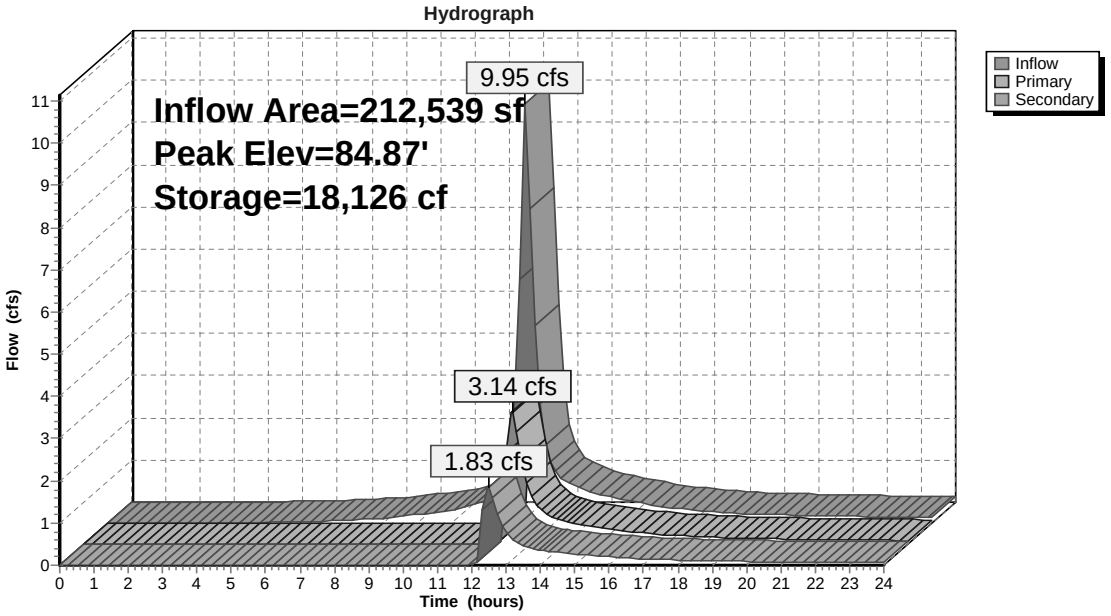
Storage Group B created with Chamber Wizard
Storage Group C created with Chamber Wizard
Storage Group D created with Chamber Wizard
Storage Group E created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	84.00'	**24.0" Round 18" HDPE AT DMH12** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.79' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf
#2	Secondary	84.00'	**12.0" Round 12" HDPE AT DMH8** L= 26.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.74' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.03 cfs @ 12.48 hrs HW=84.86' TW=81.14' (Dynamic Tailwater)
↑**1=18" HDPE AT DMH12** (Barrel Controls 3.03 cfs @ 3.49 fps)

Secondary OutFlow Max=1.77 cfs @ 12.48 hrs HW=84.85' TW=82.61' (Dynamic Tailwater)
↑**2=12" HDPE AT DMH8** (Inlet Controls 1.77 cfs @ 2.48 fps)

Pond P1A: UNDERGROUND CHAMBERS #1



Summary for Pond P1B: UNDERGROUND CHAMBERS #2

Inflow Area = 37,413 sf, 92.13% Impervious, Inflow Depth > 2.68" for 2-Year event
Inflow = 2.01 cfs @ 12.15 hrs, Volume= 8,357 cf
Outflow = 0.82 cfs @ 12.49 hrs, Volume= 4,562 cf, Atten= 59%, Lag= 20.0 min
Primary = 0.82 cfs @ 12.49 hrs, Volume= 4,562 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 84.47' @ 12.49 hrs Surf.Area= 4,282 sf Storage= 4,620 cf
Flood Elev= 84.00' Surf.Area= 4,282 sf Storage= 3,589 cf

Plug-Flow detention time= 242.0 min calculated for 4,562 cf (55% of inflow)
Center-of-Mass det. time= 130.6 min (903.4 - 772.7)

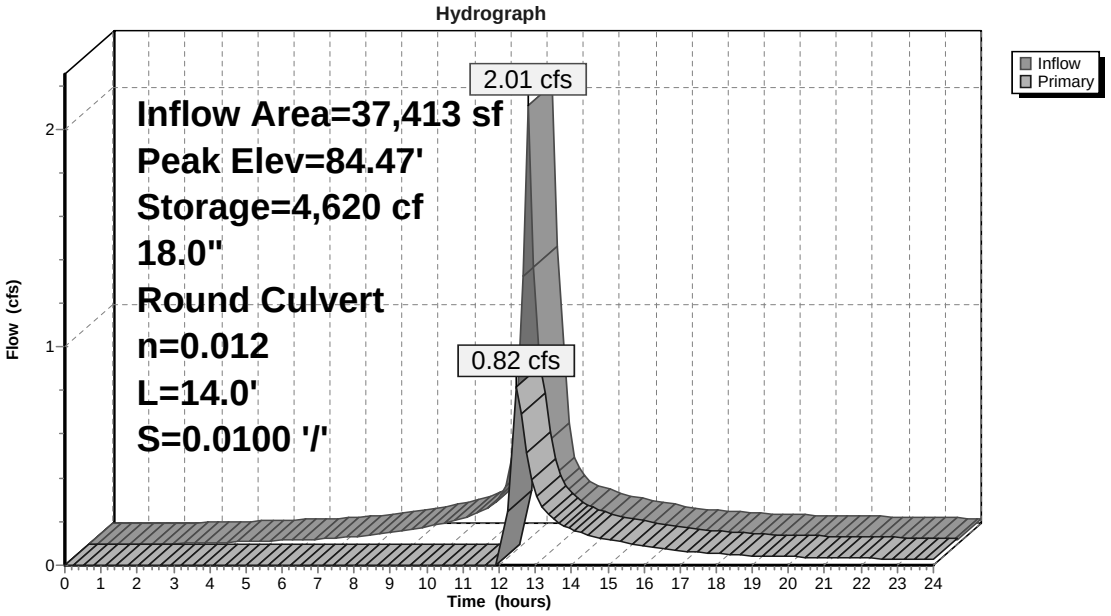
Volume	Invert	Avail.Storage	Storage Description
#1A	82.64'	2,994 cf	**41.50'W x 102.88'L x 2.33'H Field A** 9,962 cf Overall - 2,477 cf Embedded = 7,486 cf x 40.0% Voids
#2A	83.14'	2,477 cf	**ADS_StormTech SC-310 +Cap** x 168 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 168 Chambers in 12 Rows
#3	83.14'	50 cf	**4.00'D x 4.00'H DMH**
		5,521 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	84.00'	**18.0" Round 18" HDPE AT DMH3** L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.86' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=0.78 cfs @ 12.49 hrs HW=84.45' TW=79.69' (Dynamic Tailwater)
↑**1=18" HDPE AT DMH3** (Barrel Controls 0.78 cfs @ 2.61 fps)

Pond P1B: UNDERGROUND CHAMBERS #2



Summary for Pond P1C: BIO.RET. #1

Inflow Area = 48,537 sf, 83.20% Impervious, Inflow Depth > 2.53" for 2-Year event
Inflow = 2.41 cfs @ 12.15 hrs, Volume= 10,248 cf
Outflow = 2.03 cfs @ 12.24 hrs, Volume= 7,696 cf, Atten= 16%, Lag= 5.3 min
Primary = 2.03 cfs @ 12.24 hrs, Volume= 7,696 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 86.65' @ 12.24 hrs Surf.Area= 5,348 sf Storage= 3,310 cf
Flood Elev= 86.50' Surf.Area= 5,219 sf Storage= 2,505 cf

Plug-Flow detention time= 158.1 min calculated for 7,648 cf (75% of inflow)
Center-of-Mass det. time= 75.9 min (844.1 - 768.2)

Volume	Invert	Avail.Storage	Storage Description
#1	86.00'	8,156 cf	**BIORETENTION #1 (Irregular)**Listed below (Recalc)

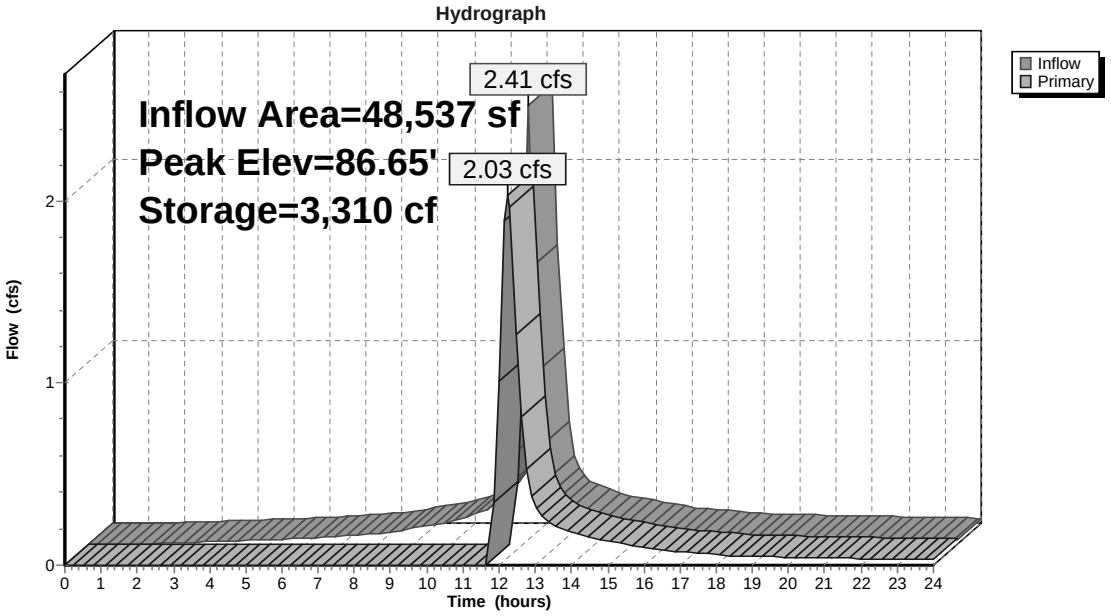
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	4,804	272.0	0	0	4,804
86.50	5,219	282.0	2,505	2,505	5,266
87.00	5,649	291.0	2,716	5,221	5,701
87.50	6,093	301.0	2,935	8,156	6,195

Device	Routing	Invert	Outlet Devices
#1	Primary	83.42'	**12.0" Round 12" HDPE** L= 55.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 83.42' / 83.14' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Device 1	86.50'	**8.0" Horiz. (5) 8" OVERFLOW X 5.00** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.94 cfs @ 12.24 hrs HW=86.65' TW=84.48' (Dynamic Tailwater)
↑**1=12" HDPE** (Passes 1.94 cfs of 4.91 cfs potential flow)
↑**2=(5) 8" OVERFLOW** (Weir Controls 1.94 cfs @ 1.26 fps)

Pond P1C: BIO.RET. #1

Hydrograph



Summary for Pond P1D: BIO.RET. #2

Inflow Area = 29,104 sf, 64.43% Impervious, Inflow Depth > 2.17" for 2-Year event
Inflow = 1.25 cfs @ 12.16 hrs, Volume= 5,269 cf
Outflow = 1.09 cfs @ 12.22 hrs, Volume= 4,104 cf, Atten= 13%, Lag= 4.0 min
Primary = 1.09 cfs @ 12.22 hrs, Volume= 4,104 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 86.62' @ 12.22 hrs Surf.Area= 2,796 sf Storage= 1,464 cf
Flood Elev= 86.50' Surf.Area= 2,628 sf Storage= 1,146 cf

Plug-Flow detention time= 145.4 min calculated for 4,078 cf (77% of inflow)
Center-of-Mass det. time= 66.7 min (846.4 - 779.6)

Volume	Invert	Avail.Storage	Storage Description
#1	86.00'	1,686 cf	**BIORETENTION (Irregular)**Listed below (Recalc)
#2	86.00'	2,826 cf	**BIORETENTION (Irregular)**Listed below (Recalc)
		4,512 cf	Total Available Storage

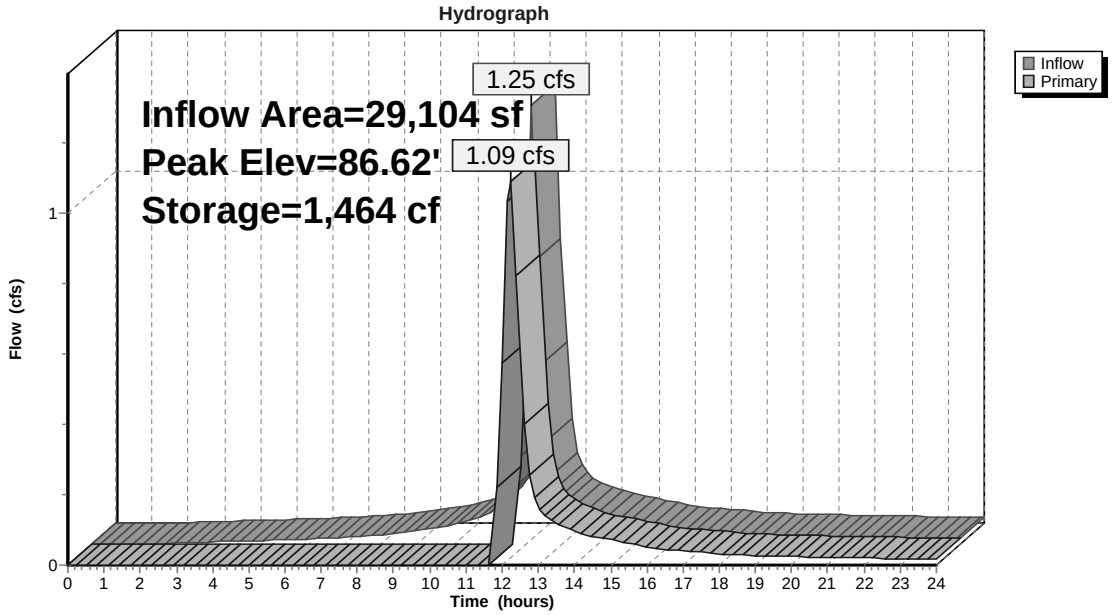
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	688	169.0	0	0	688
87.00	1,281	203.0	969	969	1,712
87.50	1,592	212.0	717	1,686	2,026

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	1,285	193.0	0	0	1,285
87.00	2,098	271.0	1,675	1,675	4,174
87.50	2,512	280.0	1,151	2,826	4,592

Device	Routing	Invert	Outlet Devices
#1	Primary	83.37'	**12.0" Round 12" HDPE** L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 83.37' / 83.14' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Device 1	86.50'	**8.0" Horiz. (4) 8" OVERFLOW X 4.00** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.03 cfs @ 12.22 hrs HW=86.61' TW=84.41' (Dynamic Tailwater)
└**1=12" HDPE** (Passes 1.03 cfs of 4.95 cfs potential flow)
└**2=(4) 8" OVERFLOW** (Weir Controls 1.03 cfs @ 1.10 fps)

Pond P1D: BIO.RET. #2



Summary for Pond P2: SURFACE BASINS

Inflow Area = 143,195 sf, 51.23% Impervious, Inflow Depth > 1.91" for 2-Year event
Inflow = 5.47 cfs @ 12.16 hrs, Volume= 22,755 cf
Outflow = 0.18 cfs @ 17.17 hrs, Volume= 3,619 cf, Atten= 97%, Lag= 300.6 min
Primary = 0.05 cfs @ 17.17 hrs, Volume= 1,016 cf
Secondary = 0.13 cfs @ 17.17 hrs, Volume= 2,603 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 84.52' @ 17.17 hrs Surf.Area= 22,654 sf Storage= 19,352 cf
Flood Elev= 84.50' Surf.Area= 22,488 sf Storage= 18,822 cf

Plug-Flow detention time= 624.6 min calculated for 3,619 cf (16% of inflow)
Center-of-Mass det. time= 378.3 min (1,170.7 - 792.4)

Volume	Invert	Avail.Storage	Storage Description
#1	83.50'	44,950 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

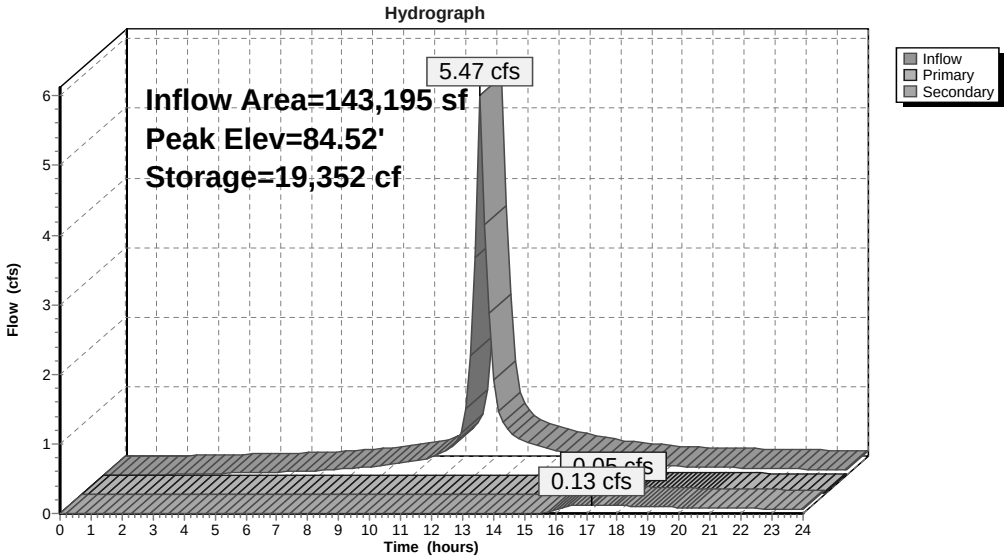
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
83.50	15,229	2,415.0	0	0	15,229
84.00	18,845	2,424.0	8,502	8,502	18,900
84.50	22,488	2,434.0	10,320	18,822	22,952
85.00	26,145	2,443.0	12,147	30,969	26,651
85.50	29,817	2,452.0	13,980	44,950	30,365

Device	Routing	Invert	Outlet Devices
#1	Primary	82.40'	**6.0" Round (2) 6" PVC X 2.00** L= 140.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 82.40' / 81.00' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf
#2	Device 1	84.50'	**8.0" Horiz. (2) 8" OVERFLOW X 2.00** C= 0.600 Limited to weir flow at low heads
#3	Secondary	84.50'	**15.0' long x 5.0' breadth RIP-RAP OVERFLOW** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.05 cfs @ 17.17 hrs HW=84.52' TW=0.00' (Dynamic Tailwater)
└─**1=(2) 6" PVC** (Passes 0.05 cfs of 1.93 cfs potential flow)
└─**2=(2) 8" OVERFLOW** (Weir Controls 0.05 cfs @ 0.50 fps)

Secondary OutFlow Max=0.13 cfs @ 17.17 hrs HW=84.52' TW=82.35' (Dynamic Tailwater)
└─**3=RIP-RAP OVERFLOW** (Weir Controls 0.13 cfs @ 0.36 fps)

Pond P2: SURFACE BASINS

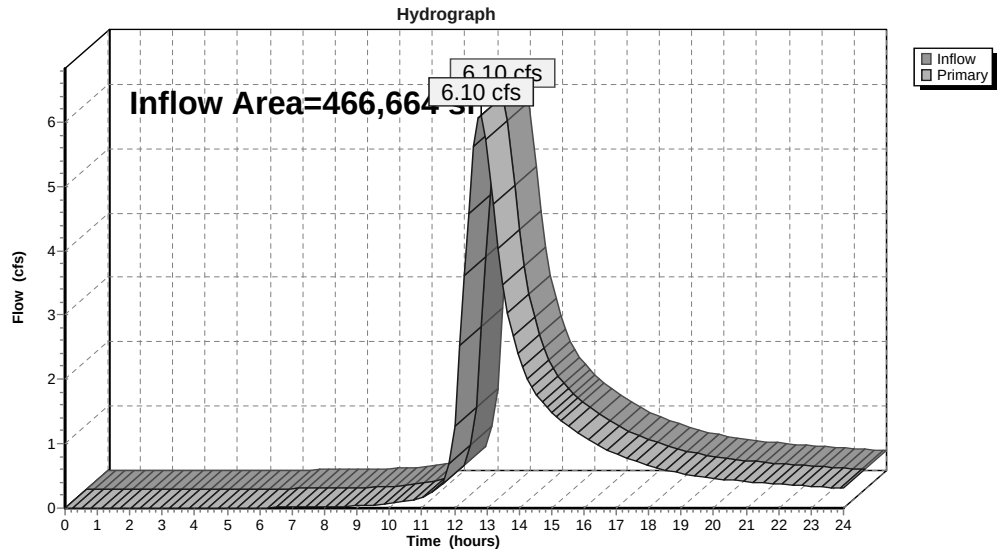


Summary for Link SP-1: STUDY POINT #1

Inflow Area = 466,664 sf, 60.79% Impervious, Inflow Depth > 1.51" for 2-Year event
Inflow = 6.10 cfs @ 12.81 hrs, Volume= 58,579 cf
Primary = 6.10 cfs @ 12.81 hrs, Volume= 58,579 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-1: STUDY POINT #1

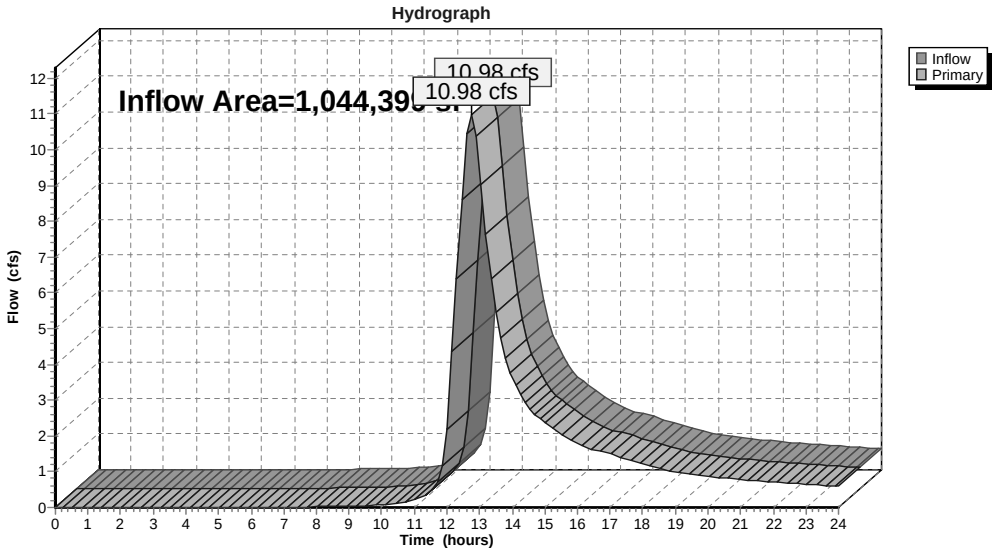


Summary for Link SP-2: STUDY POINT #2

Inflow Area = 1,044,399 sf, 36.80% Impervious, Inflow Depth > 1.14" for 2-Year event
Inflow = 10.98 cfs @ 12.75 hrs, Volume= 99,650 cf
Primary = 10.98 cfs @ 12.75 hrs, Volume= 99,650 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-2: STUDY POINT #2

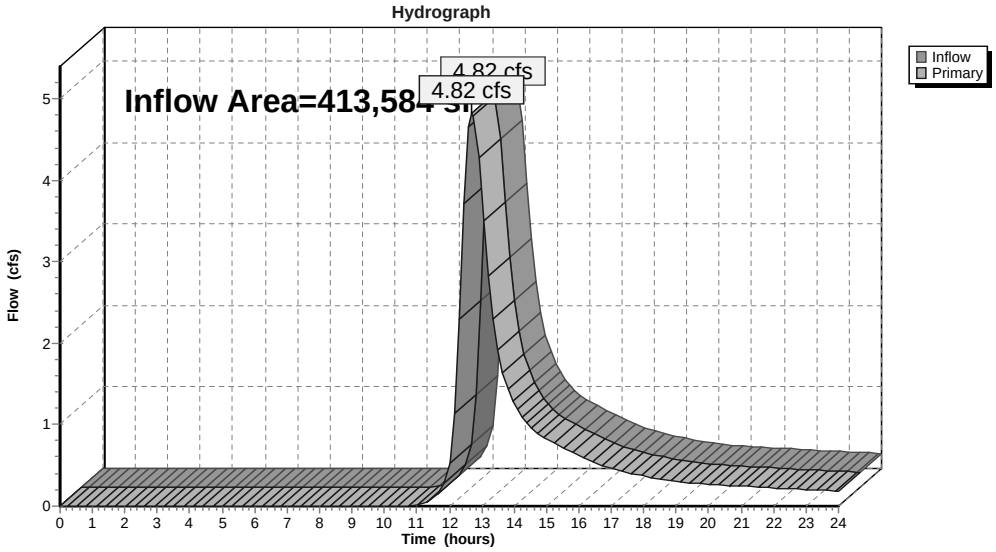


Summary for Link SP-3: STUDY POINT #3

Inflow Area = 413,584 sf, 4.62% Impervious, Inflow Depth > 1.01" for 2-Year event
Inflow = 4.82 cfs @ 12.71 hrs, Volume= 34,968 cf
Primary = 4.82 cfs @ 12.71 hrs, Volume= 34,968 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-3: STUDY POINT #3



Summary for Subcatchment P-1A: PR. WATERSHED

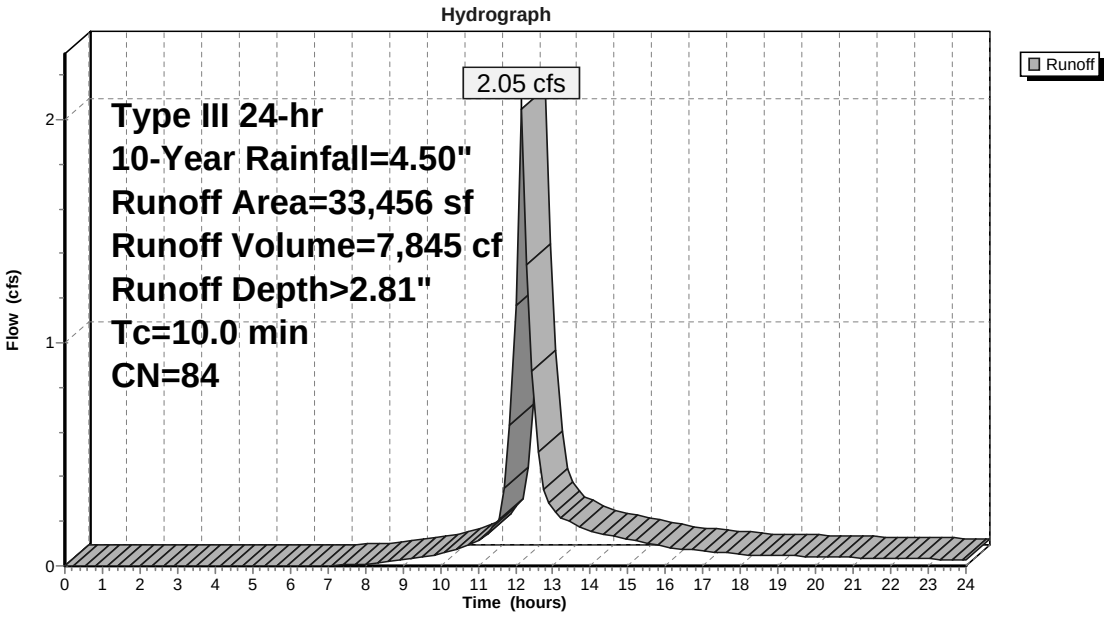
Runoff = 2.05 cfs @ 12.16 hrs, Volume= 7,845 cf, Depth> 2.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	14,724	74	>75% Grass cover, Good, HSG C
	5,389	79	Woods/grass comb., Good, HSG D
*	13,343	98	Impervious
	33,456	84	Weighted Average
	20,113		60.12% Pervious Area
	13,343		39.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1A: PR. WATERSHED



Summary for Subcatchment P-1B: PR. WATERSHED

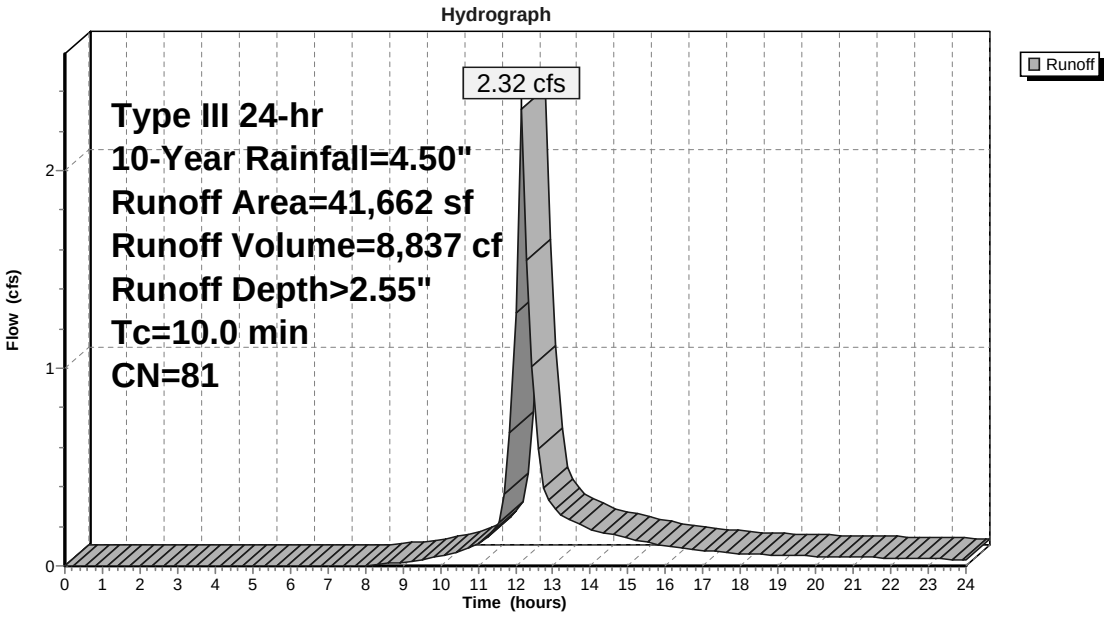
Runoff = 2.32 cfs @ 12.16 hrs, Volume= 8,837 cf, Depth> 2.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	17,448	74	>75% Grass cover, Good, HSG C
	15,311	79	Woods/grass comb., Good, HSG D
*	8,903	98	Impervious
	41,662	81	Weighted Average
	32,759		78.63% Pervious Area
	8,903		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1B: PR. WATERSHED



Summary for Subcatchment P-1C: PR. WATERSHED

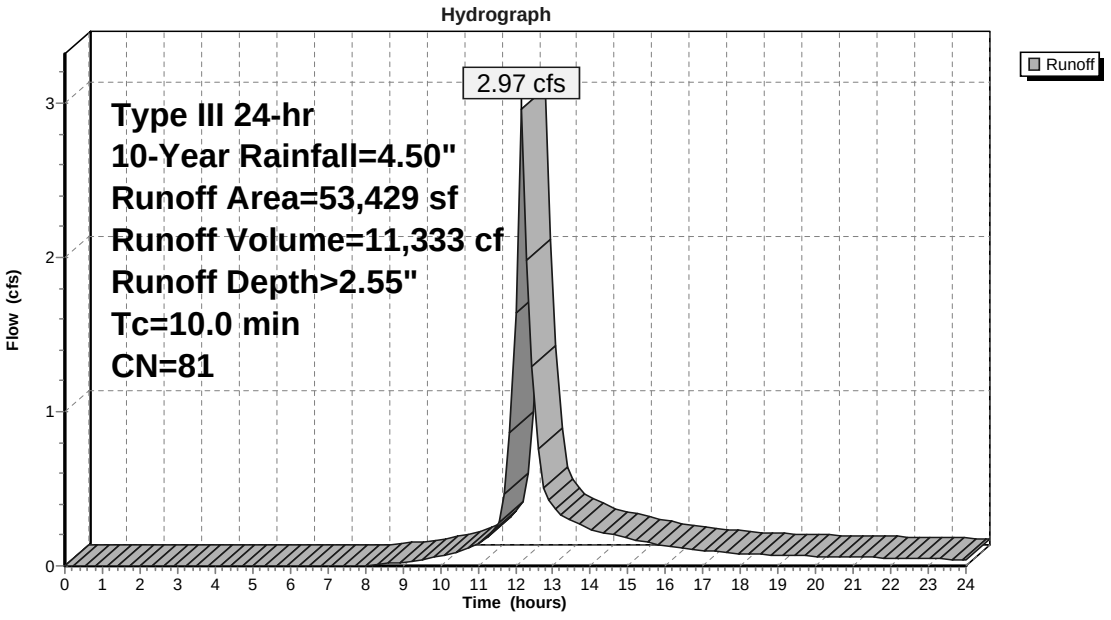
Runoff = 2.97 cfs @ 12.16 hrs, Volume= 11,333 cf, Depth> 2.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	20,039	74	>75% Grass cover, Good, HSG C
	22,761	79	Woods/grass comb., Good, HSG D
*	10,629	98	Impervious
	53,429	81	Weighted Average
	42,800		80.11% Pervious Area
	10,629		19.89% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1C: PR. WATERSHED



Summary for Subcatchment P-1D: PR. WATERSHED

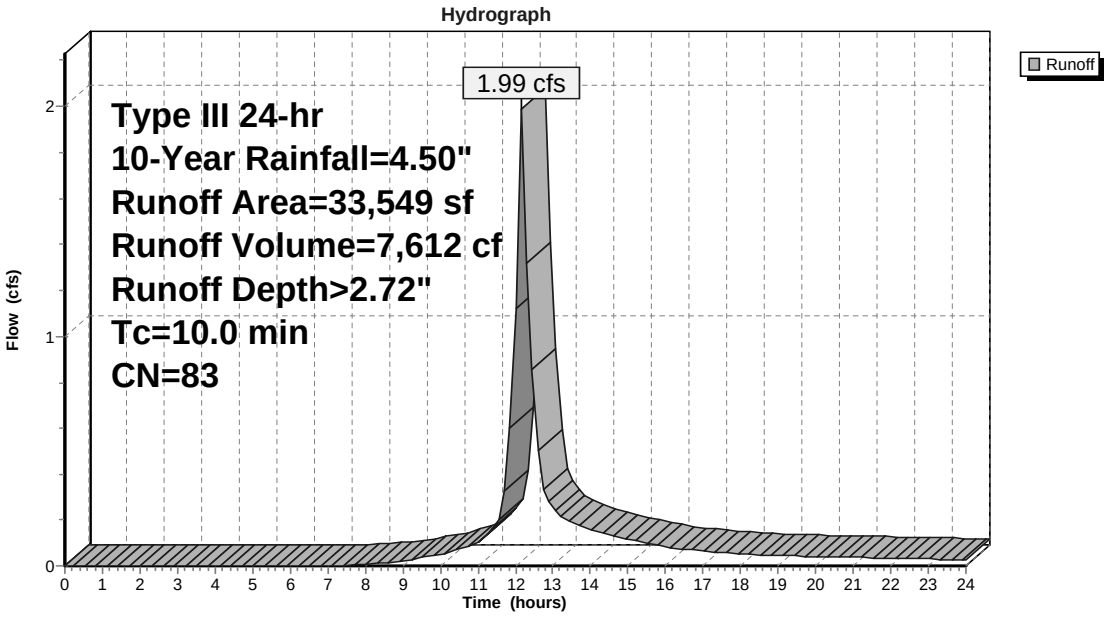
Runoff = 1.99 cfs @ 12.16 hrs, Volume= 7,612 cf, Depth> 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	19,032	74	>75% Grass cover, Good, HSG C
	2,786	79	Woods/grass comb., Good, HSG D
*	11,731	98	Impervious
	33,549	83	Weighted Average
	21,818		65.03% Pervious Area
	11,731		34.97% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1D: PR. WATERSHED



Summary for Subcatchment P-1E: PR. WATERSHED

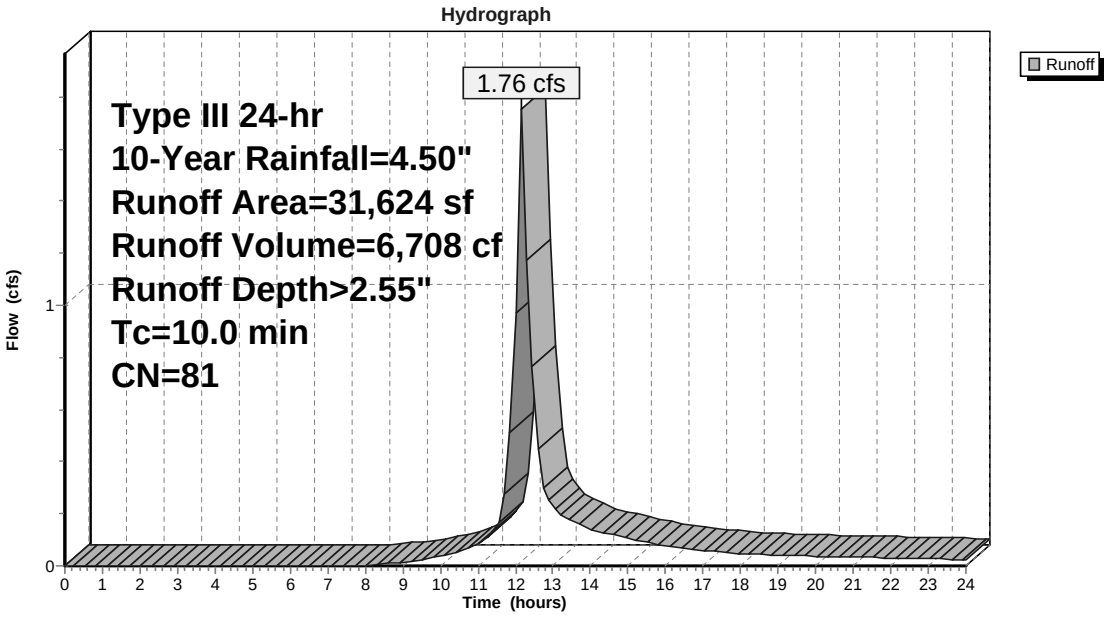
Runoff = 1.76 cfs @ 12.16 hrs, Volume= 6,708 cf, Depth> 2.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	22,158	74	>75% Grass cover, Good, HSG C
*	9,466	98	Impervious
	31,624	81	Weighted Average
	22,158		70.07% Pervious Area
	9,466		29.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1E: PR. WATERSHED



Summary for Subcatchment P-1F: PR. WATERSHED

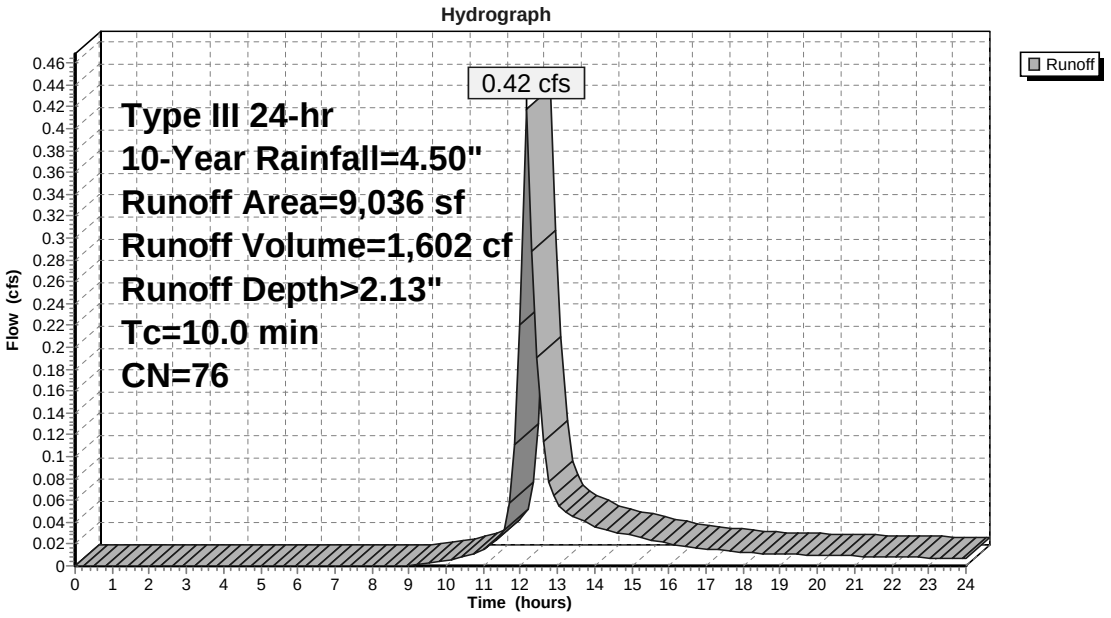
Runoff = 0.42 cfs @ 12.17 hrs, Volume= 1,602 cf, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	8,156	74	>75% Grass cover, Good, HSG C
*	880	98	Impervious
	9,036	76	Weighted Average
	8,156		90.26% Pervious Area
	880		9.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1F: PR. WATERSHED



Summary for Subcatchment P-1G: PR. WATERSHED

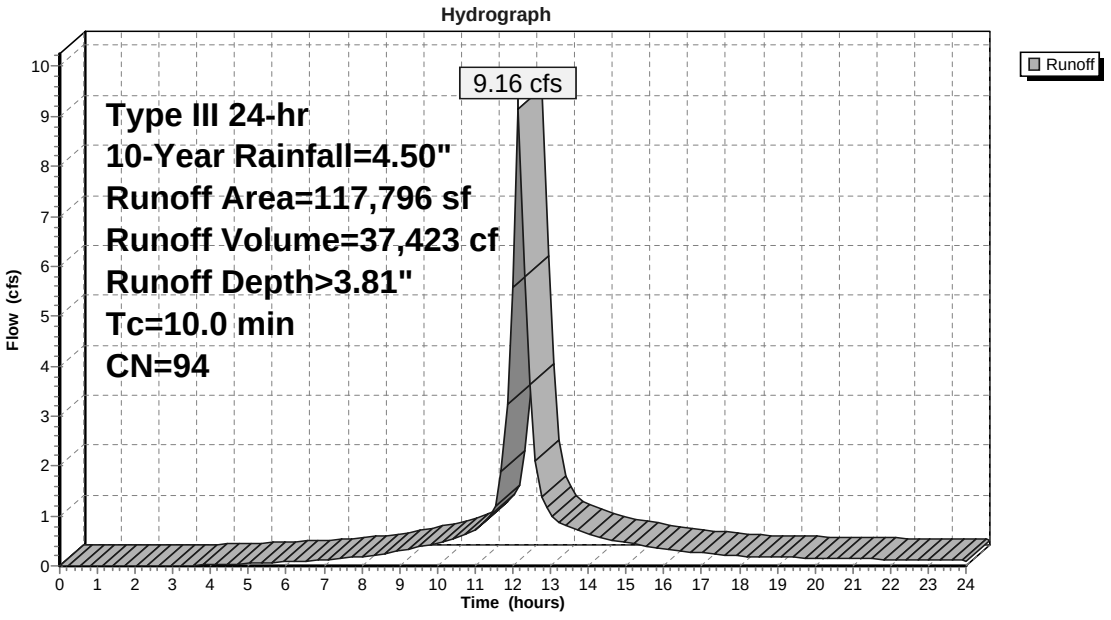
Runoff = 9.16 cfs @ 12.15 hrs, Volume= 37,423 cf, Depth> 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	21,862	74	>75% Grass cover, Good, HSG C
*	95,934	98	Impervious
	117,796	94	Weighted Average
	21,862		18.56% Pervious Area
	95,934		81.44% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1G: PR. WATERSHED



Summary for Subcatchment P-1H: PR. WATERSHED

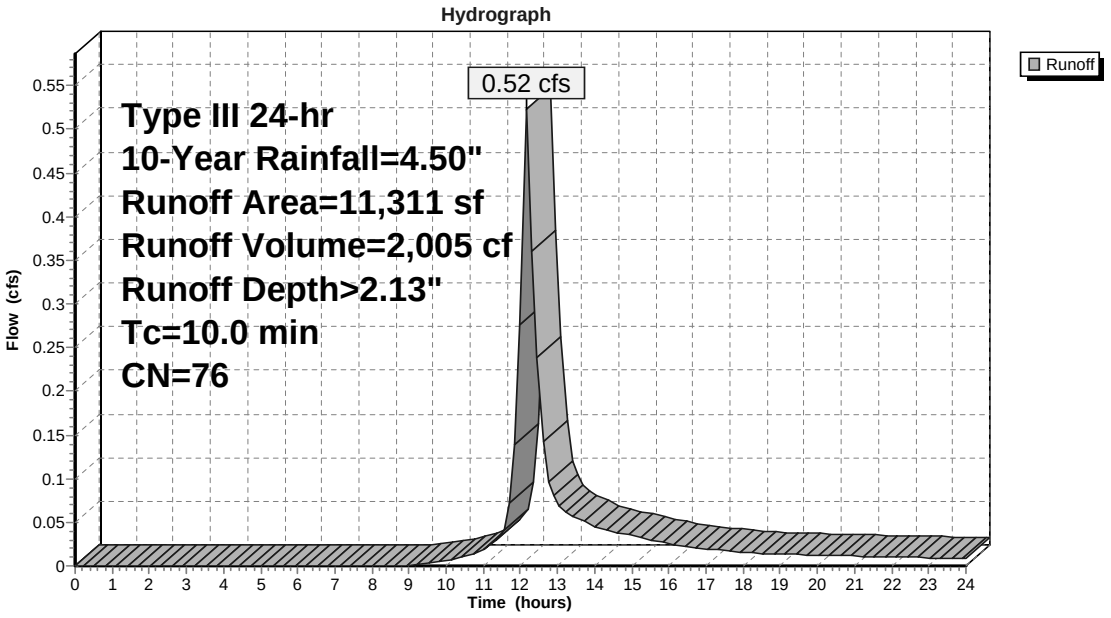
Runoff = 0.52 cfs @ 12.17 hrs, Volume= 2,005 cf, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	10,351	74	>75% Grass cover, Good, HSG C
*	960	98	Impervious
	11,311	76	Weighted Average
	10,351		91.51% Pervious Area
	960		8.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1H: PR. WATERSHED



Summary for Subcatchment P-1I: PR. WATERSHED

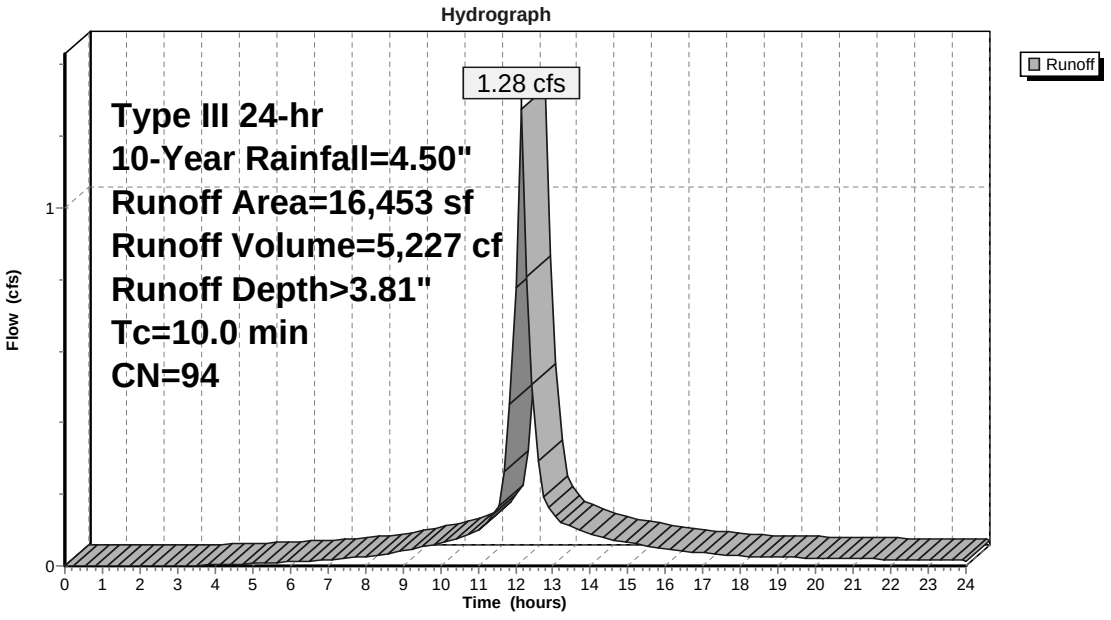
Runoff = 1.28 cfs @ 12.15 hrs, Volume= 5,227 cf, Depth> 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	2,946	74	>75% Grass cover, Good, HSG C
*	13,507	98	Impervious
	16,453	94	Weighted Average
	2,946		17.91% Pervious Area
	13,507		82.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1I: PR. WATERSHED



Summary for Subcatchment P-2A: PR. WATERSHED

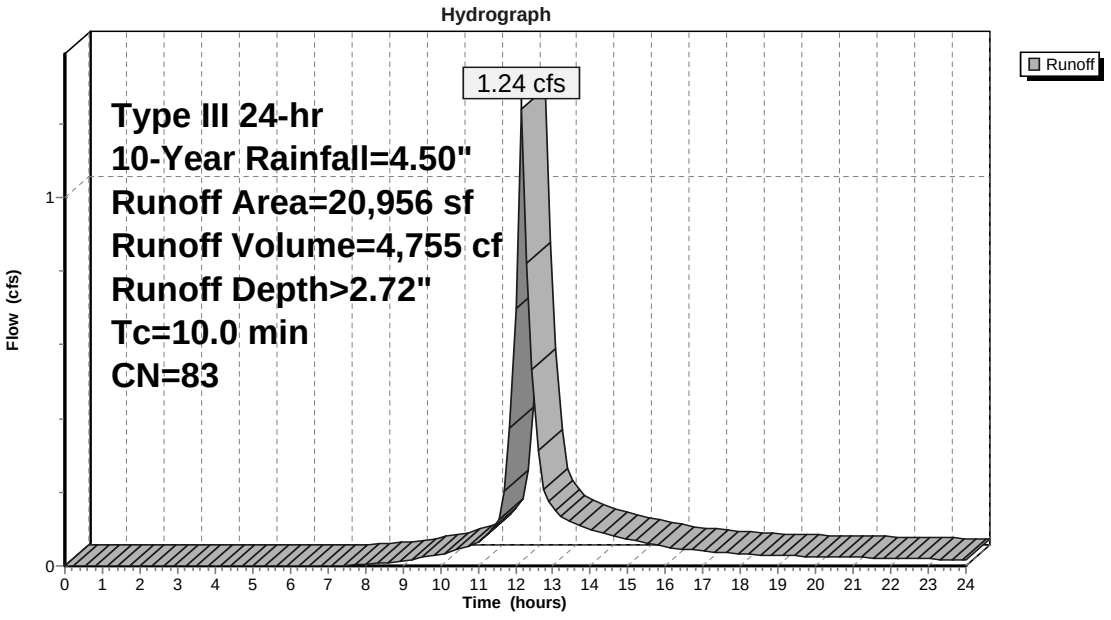
Runoff = 1.24 cfs @ 12.16 hrs, Volume= 4,755 cf, Depth> 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	12,753	74	>75% Grass cover, Good, HSG C
*	8,203	98	Impervious
	20,956	83	Weighted Average
	12,753		60.86% Pervious Area
	8,203		39.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-2A: PR. WATERSHED



Summary for Subcatchment P-2B: PR. WATERSHED

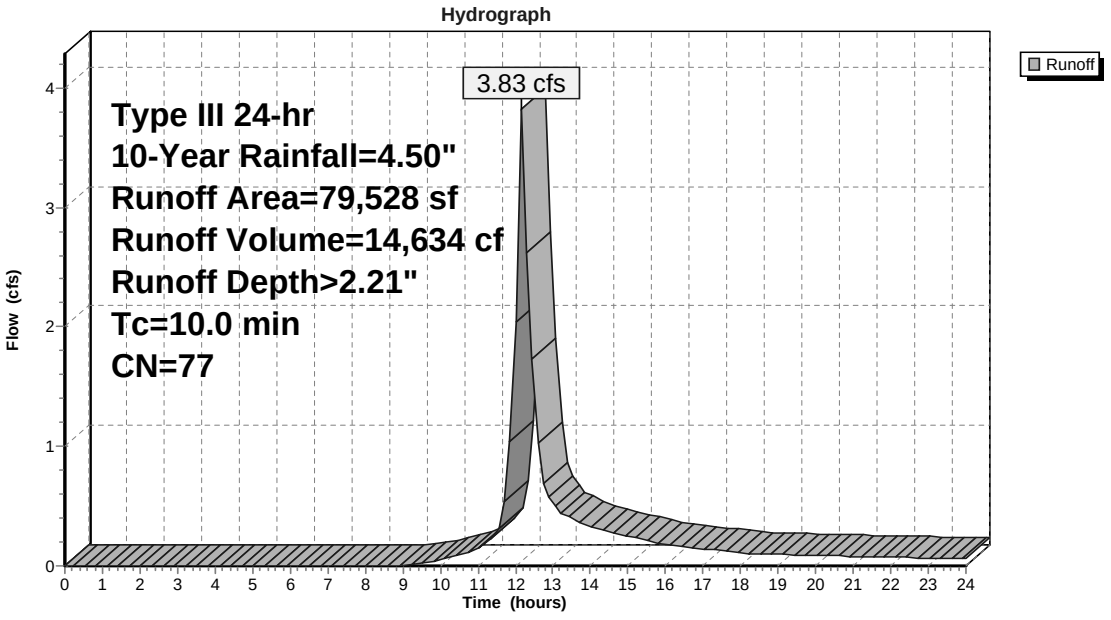
Runoff = 3.83 cfs @ 12.16 hrs, Volume= 14,634 cf, Depth> 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	69,830	74	>75% Grass cover, Good, HSG C
*	9,698	98	Impervious
	79,528	77	Weighted Average
	69,830		87.81% Pervious Area
	9,698		12.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-2B: PR. WATERSHED



Summary for Subcatchment P-3: PR. WATERSHED

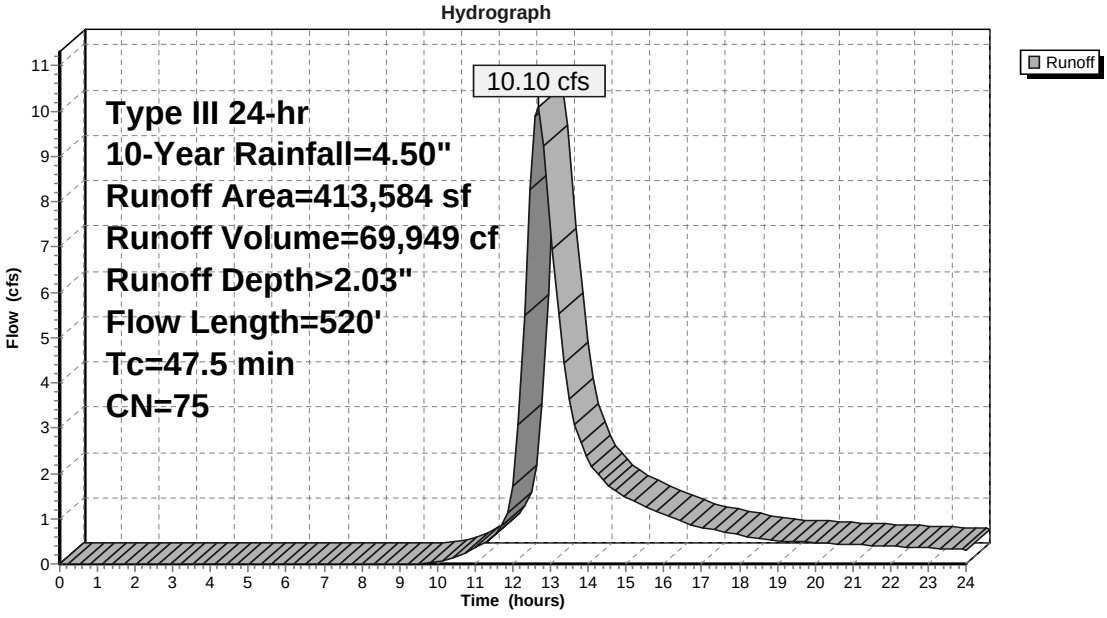
Runoff = 10.10 cfs @ 12.67 hrs, Volume= 69,949 cf, Depth> 2.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
	3,048	74	>75% Grass cover, Good, HSG C
	235,711	77	Woods, Good, HSG D
*	19,107	98	Impervious
	155,718	70	Woods, Good, HSG C
	413,584	75	Weighted Average
	394,477		95.38% Pervious Area
	19,107		4.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	100	0.0100	0.06		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
1.3	40	0.0100	0.50		**Shallow Concentrated Flow, B-C** Woodland Kv= 5.0 fps
17.9	380	0.0050	0.35		**Shallow Concentrated Flow, C-D** Woodland Kv= 5.0 fps
47.5	520	Total			

Subcatchment P-3: PR. WATERSHED



Summary for Subcatchment R-1: BUILDING #3 ROOF

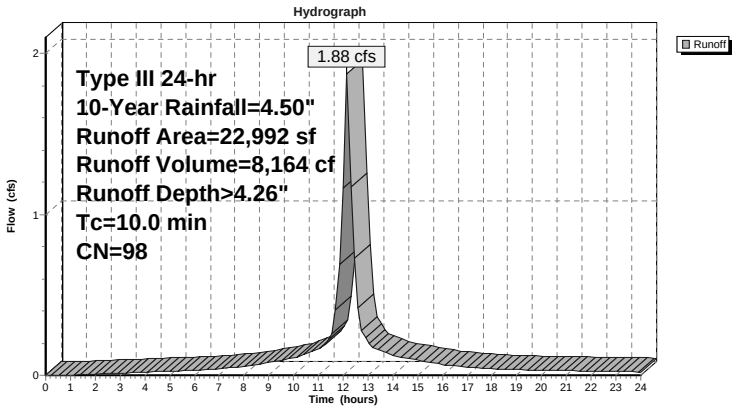
Runoff = 1.88 cfs @ 12.15 hrs, Volume= 8,164 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
*	22,992	98	Roof
	22,992		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1: BUILDING #3 ROOF



Summary for Subcatchment R-1F: BUILDING #3 ROOF

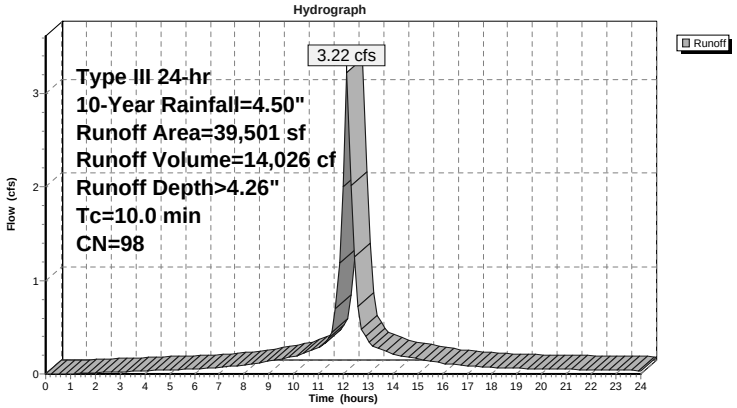
Runoff = 3.22 cfs @ 12.15 hrs, Volume= 14,026 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
*	39,501	98	Roof
	39,501		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1F: BUILDING #3 ROOF



Summary for Subcatchment R-1G: BUILDING #3 ROOF

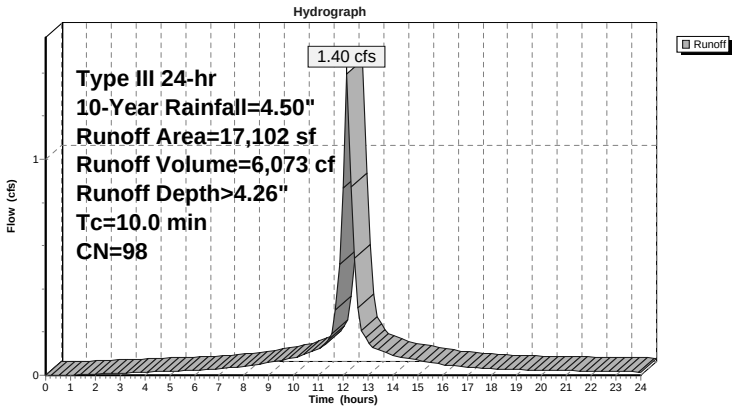
Runoff = 1.40 cfs @ 12.15 hrs, Volume= 6,073 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
*	17,102	98	Roof
	17,102		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1G: BUILDING #3 ROOF



Summary for Subcatchment R-1H: BUILDING #3 ROOF

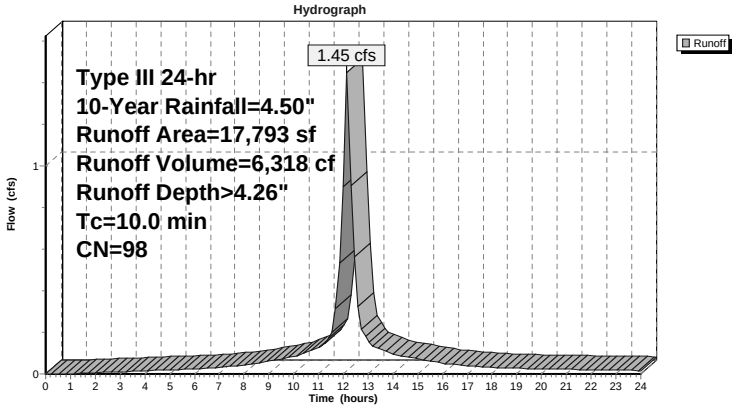
Runoff = 1.45 cfs @ 12.15 hrs, Volume= 6,318 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
*	17,793	98	Roof
	17,793		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1H: BUILDING #3 ROOF



Summary for Subcatchment R-1I: BUILDING #3 ROOF

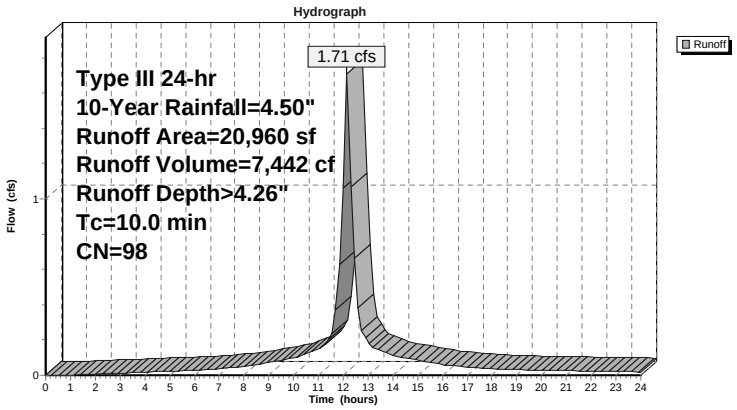
Runoff = 1.71 cfs @ 12.15 hrs, Volume= 7,442 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
*	20,960	98	Roof
	20,960		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1I: BUILDING #3 ROOF



Summary for Subcatchment R-2A: BUILDING #1 ROOF

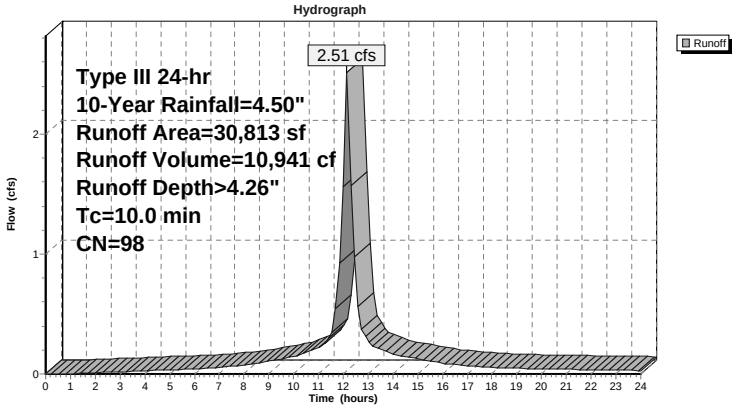
Runoff = 2.51 cfs @ 12.15 hrs, Volume= 10,941 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
*	30,813	98	Roof
	30,813		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-2A: BUILDING #1 ROOF



Summary for Subcatchment R-2B: BUILDING #2 ROOF

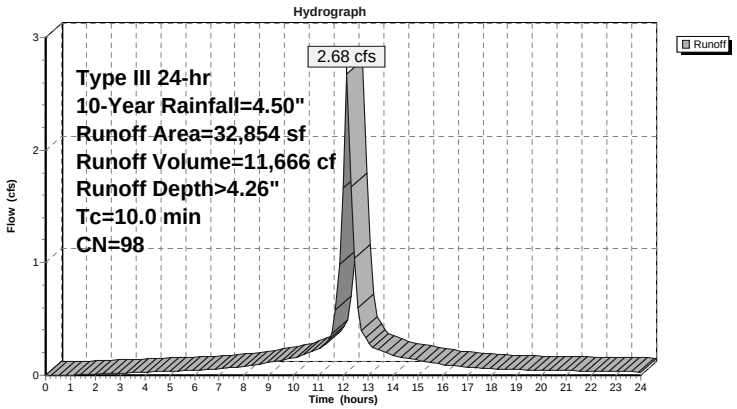
Runoff = 2.68 cfs @ 12.15 hrs, Volume= 11,666 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 10-Year Rainfall=4.50"

	Area (sf)	CN	Description
*	32,854	98	Roof
	32,854		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-2B: BUILDING #2 ROOF



Summary for Reach R-1A: DRAINAGE DITCH

Inflow Area = 466,664 sf, 60.79% Impervious, Inflow Depth > 2.77" for 10-Year event
Inflow = 17.74 cfs @ 12.50 hrs, Volume= 107,710 cf
Outflow = 17.65 cfs @ 12.52 hrs, Volume= 107,632 cf, Atten= 0%, Lag= 0.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 1.25 fps, Min. Travel Time= 0.9 min
Avg. Velocity = 0.42 fps, Avg. Travel Time= 2.8 min

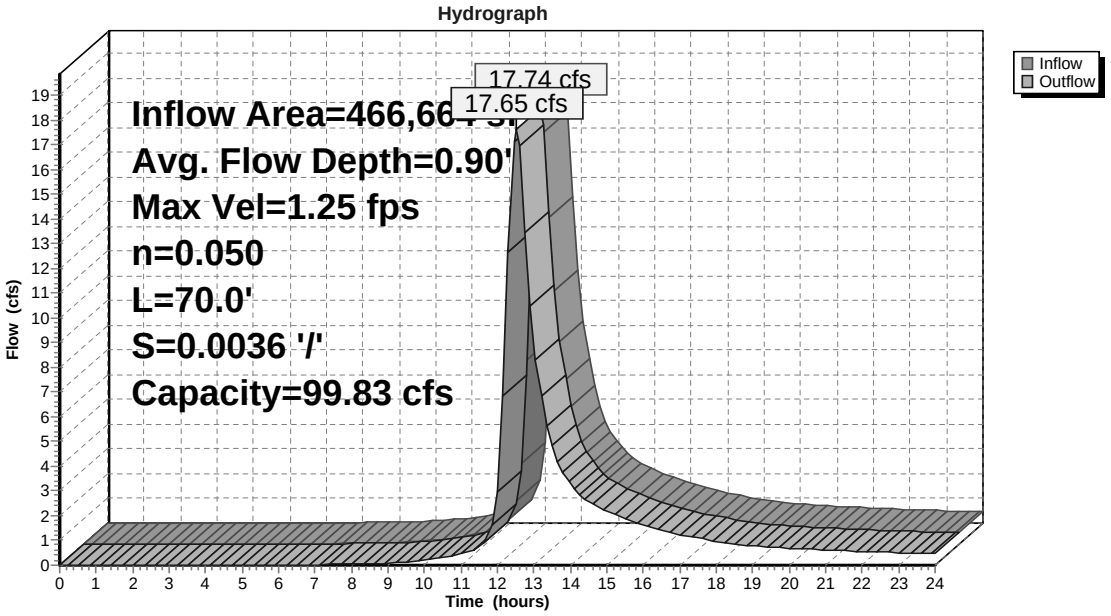
Peak Storage= 983 cf @ 12.52 hrs
Average Depth at Peak Storage= 0.90' , Surface Width= 23.46'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 99.83 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 70.0' Slope= 0.0036 '/'
Inlet Invert= 78.75', Outlet Invert= 78.50'

‡

Reach R-1A: DRAINAGE DITCH

Hydrograph



Summary for Reach R-1B: DRAINAGE DITCH

Inflow Area = 433,208 sf, 62.41% Impervious, Inflow Depth > 2.78" for 10-Year event
Inflow = 18.09 cfs @ 12.42 hrs, Volume= 100,489 cf
Outflow = 17.03 cfs @ 12.51 hrs, Volume= 99,865 cf, Atten= 6%, Lag= 5.8 min

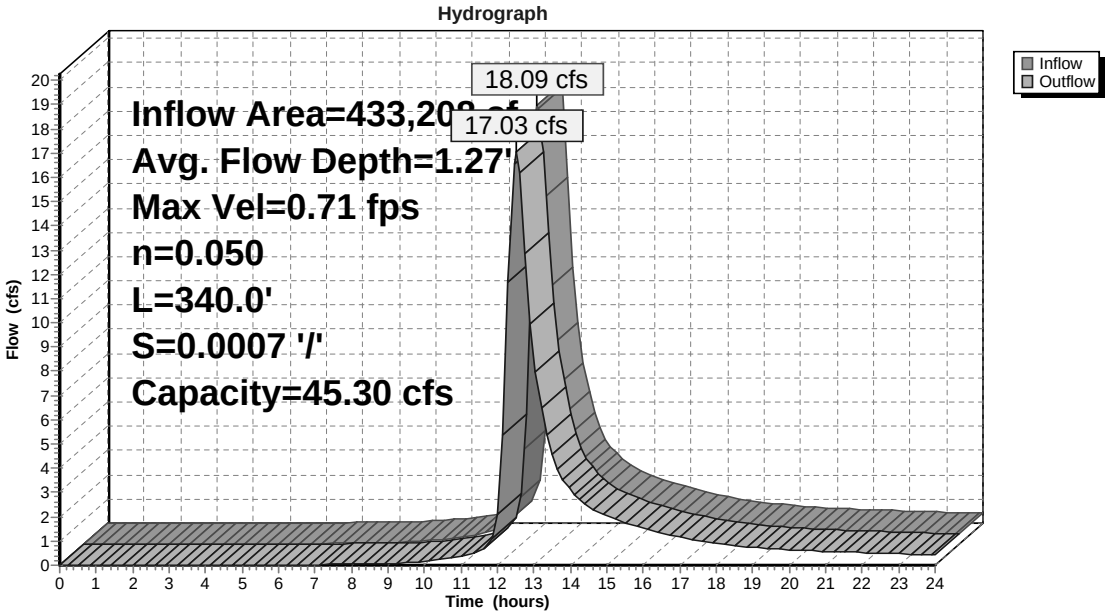
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.71 fps, Min. Travel Time= 7.9 min
Avg. Velocity = 0.23 fps, Avg. Travel Time= 24.4 min

Peak Storage= 8,058 cf @ 12.51 hrs
Average Depth at Peak Storage= 1.27' , Surface Width= 27.93'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 45.30 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 340.0' Slope= 0.0007 '/'
Inlet Invert= 79.00', Outlet Invert= 78.75'

‡

Reach R-1B: DRAINAGE DITCH



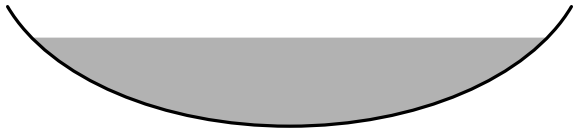
Summary for Reach R-1C: DRAINAGE DITCH

Inflow Area = 354,133 sf, 64.10% Impervious, Inflow Depth > 2.83" for 10-Year event
Inflow = 18.57 cfs @ 12.30 hrs, Volume= 83,528 cf
Outflow = 15.29 cfs @ 12.45 hrs, Volume= 82,819 cf, Atten= 18%, Lag= 8.6 min

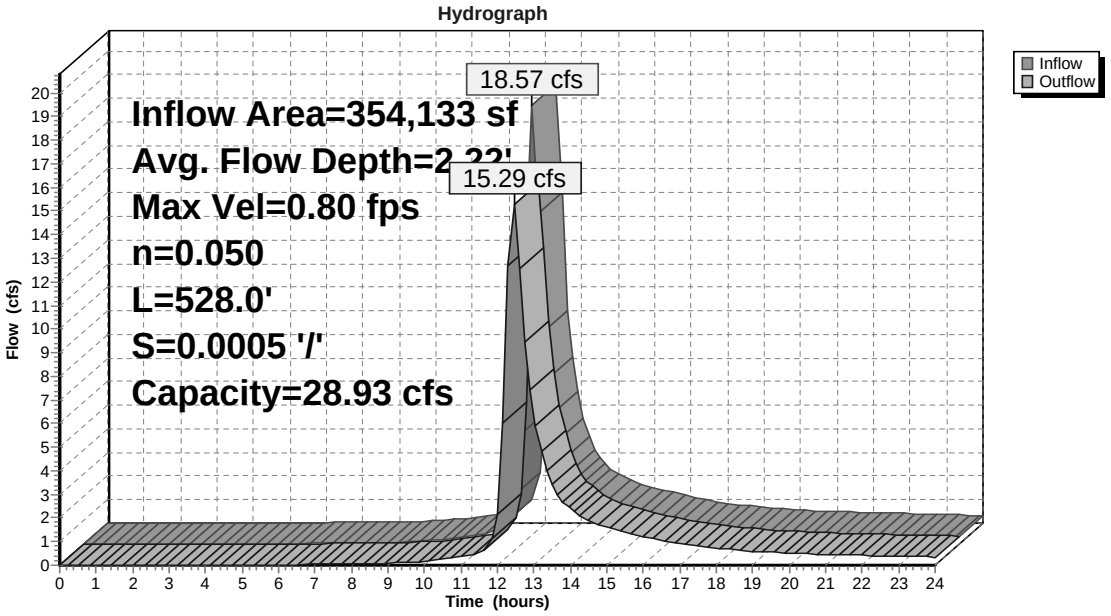
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.80 fps, Min. Travel Time= 11.0 min
Avg. Velocity = 0.26 fps, Avg. Travel Time= 34.0 min

Peak Storage= 10,080 cf @ 12.45 hrs
Average Depth at Peak Storage= 2.22' , Surface Width= 12.90'
Bank-Full Depth= 3.00' Flow Area= 30.0 sf, Capacity= 28.93 cfs

15.00' x 3.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0005 '/'
Inlet Invert= 79.25', Outlet Invert= 79.00'



Reach R-1C: DRAINAGE DITCH



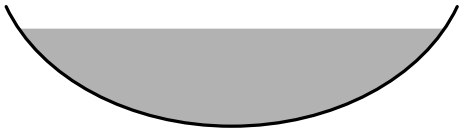
Summary for Reach R-1D: DRAINAGE DITCH

Inflow Area = 277,712 sf, 69.63% Impervious, Inflow Depth > 2.78" for 10-Year event
Inflow = 17.70 cfs @ 12.21 hrs, Volume= 64,402 cf
Outflow = 15.54 cfs @ 12.32 hrs, Volume= 64,031 cf, Atten= 12%, Lag= 6.6 min

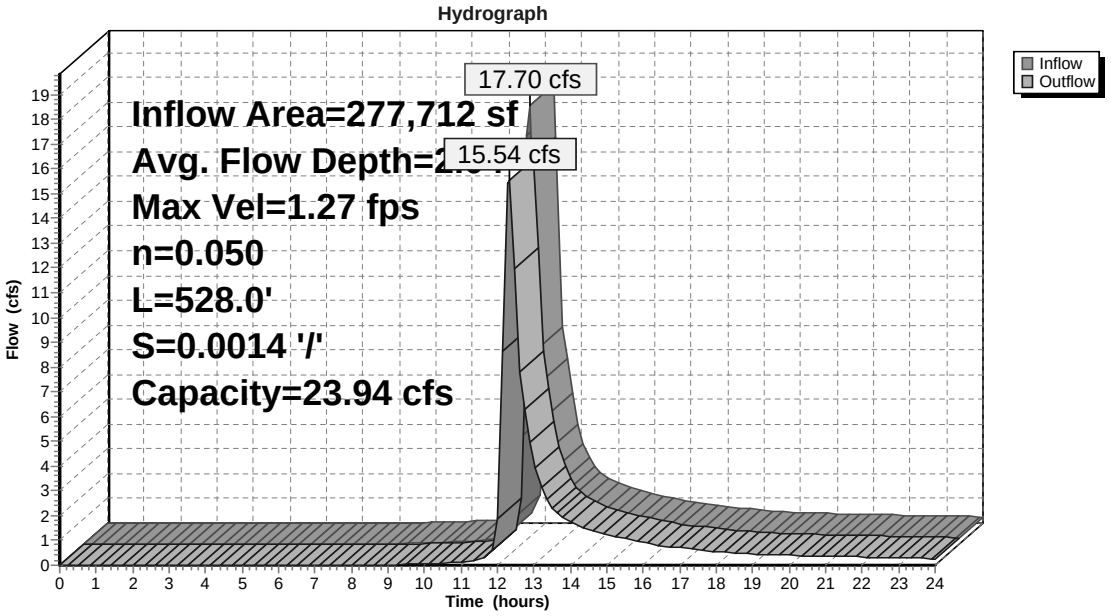
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 1.27 fps, Min. Travel Time= 6.9 min
Avg. Velocity = 0.45 fps, Avg. Travel Time= 19.6 min

Peak Storage= 6,466 cf @ 12.33 hrs
Average Depth at Peak Storage= 2.04' , Surface Width= 9.03'
Bank-Full Depth= 2.50' Flow Area= 16.7 sf, Capacity= 23.94 cfs

10.00' x 2.50' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0014 '/'
Inlet Invert= 80.00', Outlet Invert= 79.25'



Reach R-1D: DRAINAGE DITCH



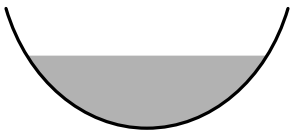
Summary for Reach R-1E: DRAINAGE DITCH

Inflow Area = 31,624 sf, 29.93% Impervious, Inflow Depth > 9.21" for 10-Year event
Inflow = 5.38 cfs @ 12.20 hrs, Volume= 24,266 cf
Outflow = 5.02 cfs @ 12.28 hrs, Volume= 24,191 cf, Atten= 7%, Lag= 5.1 min

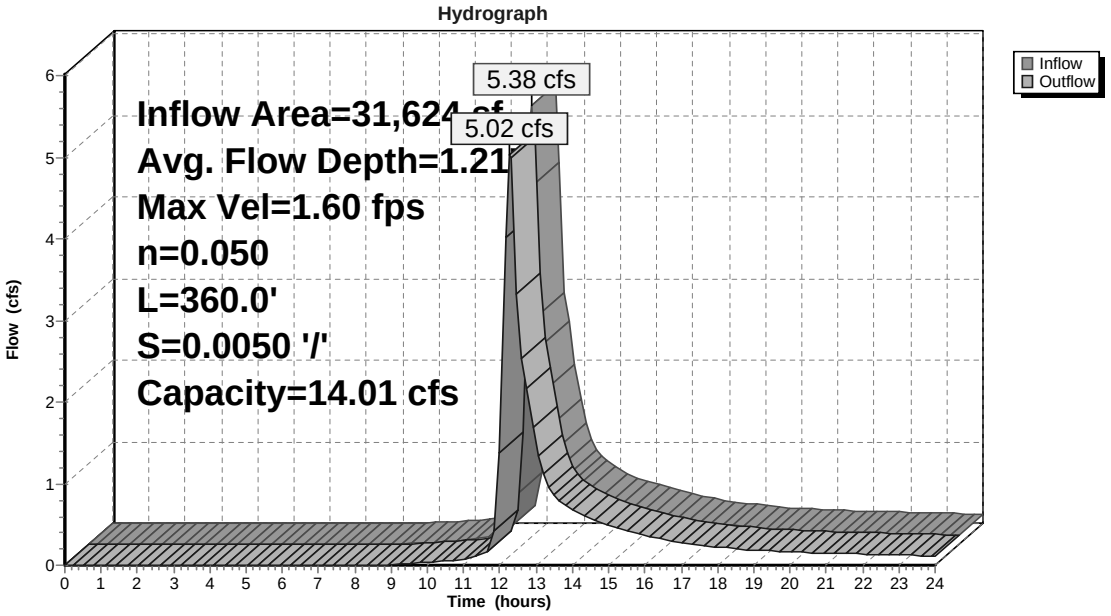
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 1.60 fps, Min. Travel Time= 3.8 min
Avg. Velocity = 0.64 fps, Avg. Travel Time= 9.3 min

Peak Storage= 1,132 cf @ 12.28 hrs
Average Depth at Peak Storage= 1.21' , Surface Width= 3.89'
Bank-Full Depth= 2.00' Flow Area= 6.7 sf, Capacity= 14.01 cfs

5.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 360.0' Slope= 0.0050 '/'
Inlet Invert= 81.80', Outlet Invert= 80.00'



Reach R-1E: DRAINAGE DITCH



Summary for Pond DMH1: DMH1

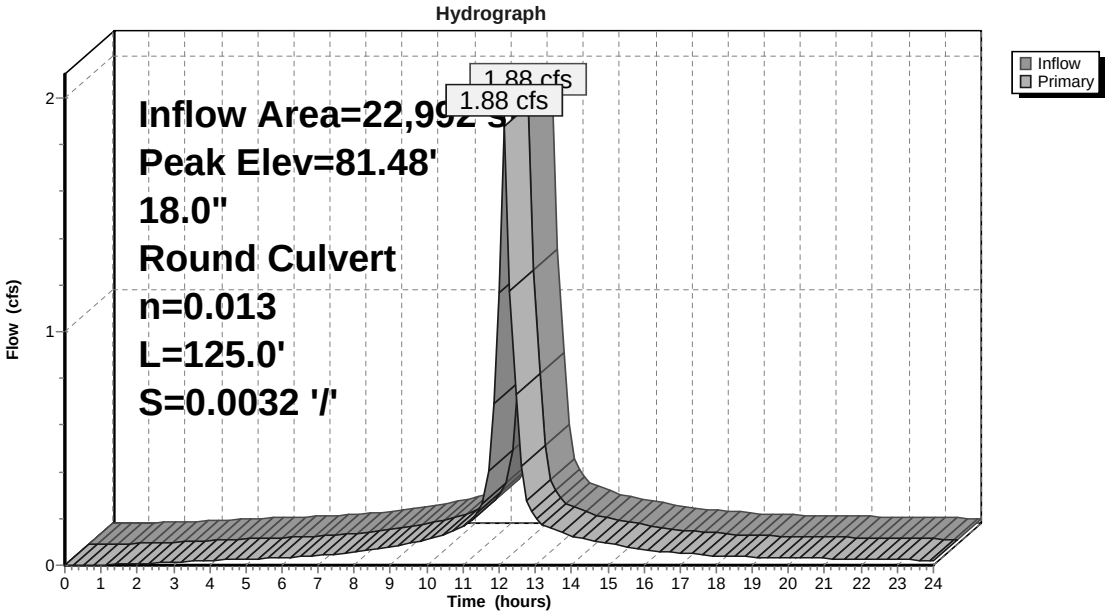
Inflow Area = 22,992 sf,100.00% Impervious, Inflow Depth > 4.26" for 10-Year event
Inflow = 1.88 cfs @ 12.15 hrs, Volume= 8,164 cf
Outflow = 1.88 cfs @ 12.15 hrs, Volume= 8,164 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.88 cfs @ 12.15 hrs, Volume= 8,164 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 81.48' @ 12.44 hrs
Flood Elev= 84.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	79.20'	**18.0" Round EX. 18" VCC** L= 125.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 79.20' / 78.80' S= 0.0032 '/' Cc= 0.900 n= 0.013 Clay tile, Flow Area= 1.77 sf

Primary OutFlow Max=1.88 cfs @ 12.15 hrs HW=80.78' TW=80.69' (Dynamic Tailwater)
↑—**1=EX. 18" VCC** (Outlet Controls 1.88 cfs @ 1.25 fps)

Pond DMH1: DMH1



Summary for Pond OFFSITE: OFFSITE PONDING AREA IN GRASS

Inflow	=	1.50 cfs @	12.66 hrs, Volume=	12,947 cf
Outflow	=	1.48 cfs @	12.79 hrs, Volume=	12,739 cf, Atten= 1%, Lag= 7.6 min
Primary	=	1.48 cfs @	12.79 hrs, Volume=	12,739 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 82.61' @ 12.75 hrs Surf.Area= 2,956 sf Storage= 819 cf

Plug-Flow detention time= 18.8 min calculated for 12,739 cf (98% of inflow)
Center-of-Mass det. time= 10.8 min (946.9 - 936.1)

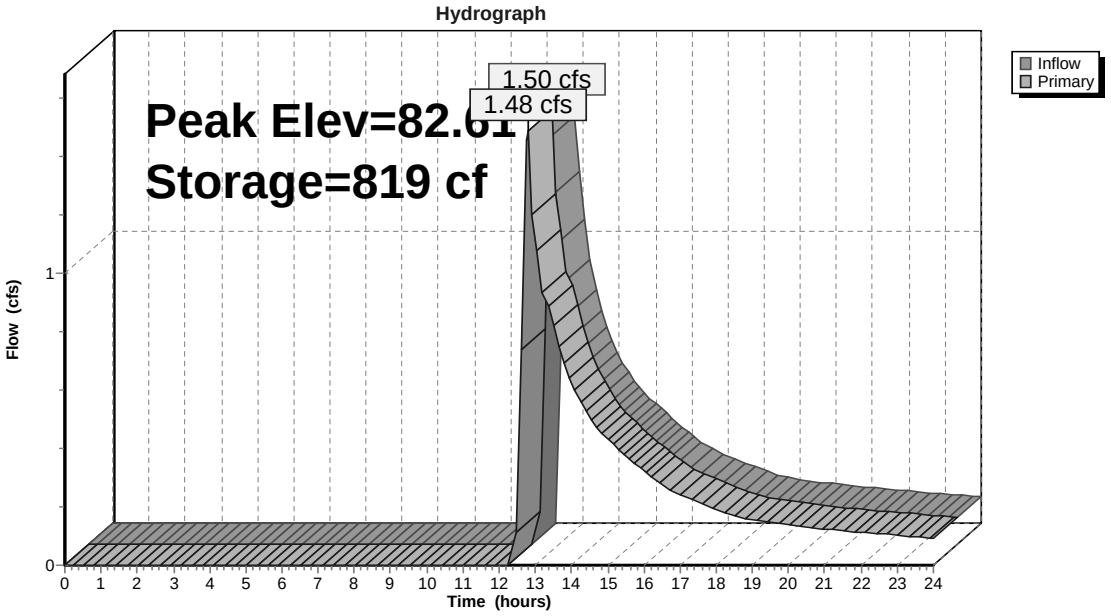
Volume	Invert	Avail.Storage	Storage Description
#1	82.10'	2,354 cf	**OFFSITE PONDING AREA (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.10	230	0	0
83.00	5,000	2,354	2,354

Device	Routing	Invert	Outlet Devices
#1	Primary	82.27'	**8.0" Round (3) 8" HDPE X 3.00 w/ 2.0" inside fill** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.10' / 81.40' S= 0.0333 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.28 sf
#2	Primary	82.60'	**100.0' long x 20.0' breadth WEIR FLOW OVER WALKING PATH** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=1.39 cfs @ 12.79 hrs HW=82.61' TW=0.00' (Dynamic Tailwater)
1=(3) 8" HDPE (Inlet Controls 0.99 cfs @ 1.50 fps)
2=WEIR FLOW OVER WALKING PATH (Weir Controls 0.40 cfs @ 0.31 fps)

Pond OFFSITE: OFFSITE PONDING AREA IN GRASS



Summary for Pond P1A: UNDERGROUND CHAMBERS #1

Inflow Area = 212,539 sf, 81.01% Impervious, Inflow Depth > 3.60" for 10-Year event
Inflow = 15.25 cfs @ 12.17 hrs, Volume= 63,709 cf
Outflow = 14.85 cfs @ 12.21 hrs, Volume= 50,156 cf, Atten= 3%, Lag= 2.5 min
Primary = 11.18 cfs @ 12.21 hrs, Volume= 32,598 cf
Secondary = 3.67 cfs @ 12.21 hrs, Volume= 17,558 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 85.94' @ 12.21 hrs Surf.Area= 14,913 sf Storage= 18,737 cf
Flood Elev= 84.00' Surf.Area= 14,913 sf Storage= 12,169 cf

Plug-Flow detention time= 139.8 min calculated for 50,156 cf (79% of inflow)
Center-of-Mass det. time= 64.0 min (855.6 - 791.6)

Volume	Invert	Avail.Storage	Storage Description
#1B	82.64'	5,589 cf	**8.17'W x 935.92'L x 2.33'H Field B** 17,834 cf Overall - 3,862 cf Embedded = 13,972 cf x 40.0% Voids
#2B	83.14'	3,862 cf	**ADS_StormTech SC-310 +Cap** x 262 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 262 Chambers in 2 Rows
#3C	82.64'	2,519 cf	**34.83'W x 102.88'L x 2.33'H Field C** 8,362 cf Overall - 2,064 cf Embedded = 6,298 cf x 40.0% Voids
#4C	83.14'	2,064 cf	**ADS_StormTech SC-310 +Cap** x 140 Inside #3 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 140 Chambers in 10 Rows
#5D	82.64'	2,002 cf	**34.83'W x 81.52'L x 2.33'H Field D** 6,626 cf Overall - 1,622 cf Embedded = 5,004 cf x 40.0% Voids
#6D	83.14'	1,622 cf	**ADS_StormTech SC-310 +Cap** x 110 Inside #5 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 110 Chambers in 10 Rows
#7E	82.64'	602 cf	**21.50'W x 38.80'L x 2.33'H Field E** 1,946 cf Overall - 442 cf Embedded = 1,504 cf x 40.0% Voids
#8E	83.14'	442 cf	**ADS_StormTech SC-310 +Cap** x 30 Inside #7

			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 30 Chambers in 6 Rows
#9	83.14'	50 cf	**4.00'D x 4.00'H DMH**
		18,752 cf	Total Available Storage

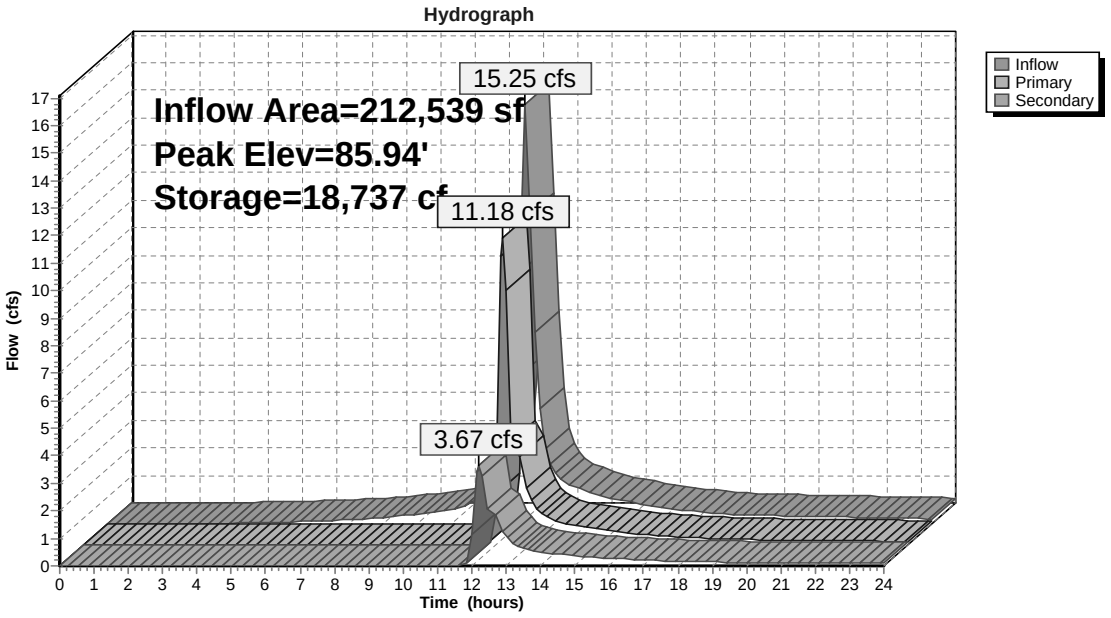
Storage Group B created with Chamber Wizard
Storage Group C created with Chamber Wizard
Storage Group D created with Chamber Wizard
Storage Group E created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	84.00'	**24.0" Round 18" HDPE AT DMH12** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.79' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf
#2	Secondary	84.00'	**12.0" Round 12" HDPE AT DMH8** L= 26.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.74' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=10.02 cfs @ 12.21 hrs HW=85.78' TW=81.72' (Dynamic Tailwater)
└**1=18" HDPE AT DMH12** (Barrel Controls 10.02 cfs @ 4.50 fps)

Secondary OutFlow Max=3.37 cfs @ 12.21 hrs HW=85.77' TW=82.94' (Dynamic Tailwater)
└**2=12" HDPE AT DMH8** (Inlet Controls 3.37 cfs @ 4.29 fps)

Pond P1A: UNDERGROUND CHAMBERS #1



Summary for Pond P1B: UNDERGROUND CHAMBERS #2

Inflow Area = 37,413 sf, 92.13% Impervious, Inflow Depth > 4.06" for 10-Year event
Inflow = 2.99 cfs @ 12.15 hrs, Volume= 12,669 cf
Outflow = 2.31 cfs @ 12.30 hrs, Volume= 8,834 cf, Atten= 23%, Lag= 8.6 min
Primary = 2.31 cfs @ 12.30 hrs, Volume= 8,834 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 84.85' @ 12.29 hrs Surf.Area= 4,282 sf Storage= 5,276 cf
Flood Elev= 84.00' Surf.Area= 4,282 sf Storage= 3,589 cf

Plug-Flow detention time= 186.4 min calculated for 8,834 cf (70% of inflow)
Center-of-Mass det. time= 94.5 min (858.5 - 764.0)

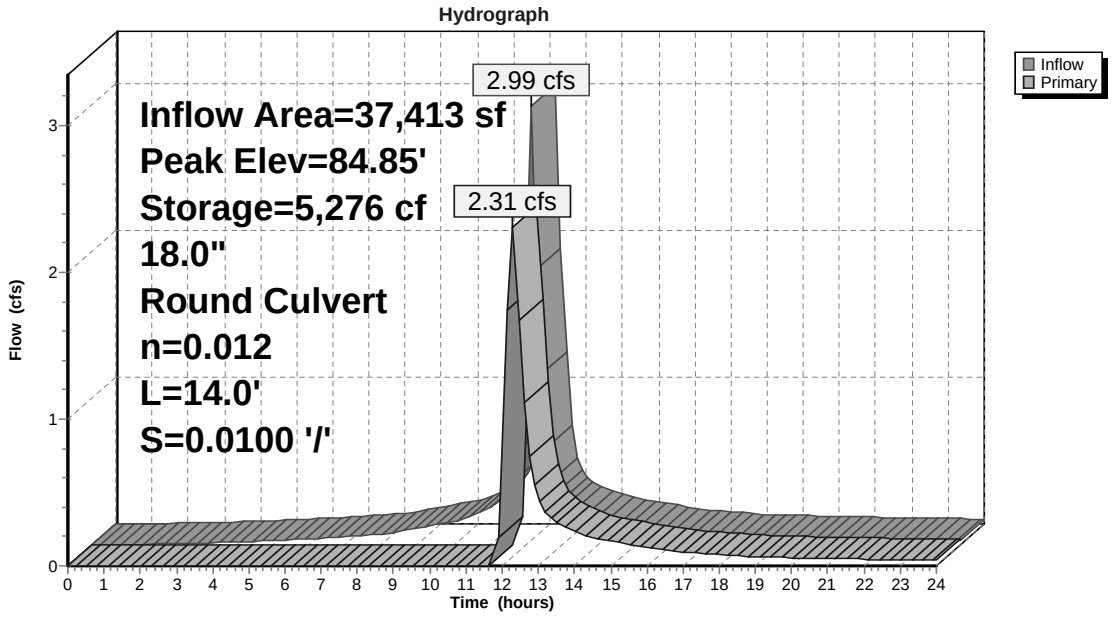
Volume	Invert	Avail.Storage	Storage Description
#1A	82.64'	2,994 cf	**41.50'W x 102.88'L x 2.33'H Field A** 9,962 cf Overall - 2,477 cf Embedded = 7,486 cf x 40.0% Voids
#2A	83.14'	2,477 cf	**ADS_StormTech SC-310 +Cap** x 168 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 168 Chambers in 12 Rows
#3	83.14'	50 cf	**4.00'D x 4.00'H DMH**
		5,521 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	84.00'	**18.0" Round 18" HDPE AT DMH3** L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.86' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=2.29 cfs @ 12.30 hrs HW=84.84' TW=80.06' (Dynamic Tailwater)
1=18" HDPE AT DMH3 (Barrel Controls 2.29 cfs @ 3.24 fps)

Pond P1B: UNDERGROUND CHAMBERS #2



Summary for Pond P1C: BIO.RET. #1

Inflow Area = 48,537 sf, 83.20% Impervious, Inflow Depth > 3.86" for 10-Year event
Inflow = 3.64 cfs @ 12.15 hrs, Volume= 15,628 cf
Outflow = 3.11 cfs @ 12.23 hrs, Volume= 13,062 cf, Atten= 15%, Lag= 4.5 min
Primary = 3.11 cfs @ 12.23 hrs, Volume= 13,062 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 86.70' @ 12.23 hrs Surf.Area= 5,391 sf Storage= 3,580 cf
Flood Elev= 86.50' Surf.Area= 5,219 sf Storage= 2,505 cf

Plug-Flow detention time= 129.7 min calculated for 13,062 cf (84% of inflow)
Center-of-Mass det. time= 62.4 min (824.3 - 762.0)

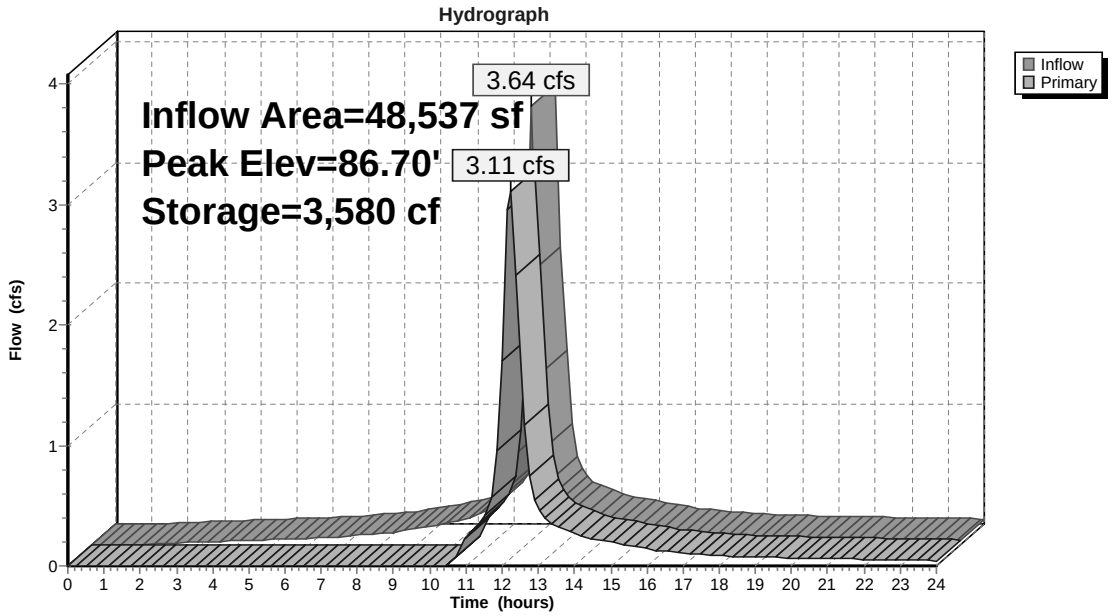
Volume	Invert	Avail.Storage	Storage Description
#1	86.00'	8,156 cf	**BIORETENTION #1 (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	4,804	272.0	0	0	4,804
86.50	5,219	282.0	2,505	2,505	5,266
87.00	5,649	291.0	2,716	5,221	5,701
87.50	6,093	301.0	2,935	8,156	6,195

Device	Routing	Invert	Outlet Devices
#1	Primary	83.42'	**12.0" Round 12" HDPE** L= 55.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 83.42' / 83.14' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Device 1	86.50'	**8.0" Horiz. (5) 8" OVERFLOW X 5.00** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.97 cfs @ 12.23 hrs HW=86.70' TW=85.76' (Dynamic Tailwater)
↑└**1=12" HDPE** (Passes 2.97 cfs of 3.23 cfs potential flow)
↑└**2=(5) 8" OVERFLOW** (Weir Controls 2.97 cfs @ 1.45 fps)

Pond P1C: BIO.RET. #1



Summary for Pond P1D: BIO.RET. #2

Inflow Area = 29,104 sf, 64.43% Impervious, Inflow Depth > 3.43" for 10-Year event
Inflow = 1.97 cfs @ 12.15 hrs, Volume= 8,323 cf
Outflow = 1.75 cfs @ 12.21 hrs, Volume= 7,152 cf, Atten= 11%, Lag= 3.4 min
Primary = 1.75 cfs @ 12.21 hrs, Volume= 7,152 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 86.66' @ 12.21 hrs Surf.Area= 2,859 sf Storage= 1,586 cf
Flood Elev= 86.50' Surf.Area= 2,628 sf Storage= 1,146 cf

Plug-Flow detention time= 112.1 min calculated for 7,108 cf (85% of inflow)
Center-of-Mass det. time= 52.9 min (827.0 - 774.1)

Volume	Invert	Avail.Storage	Storage Description
#1	86.00'	1,686 cf	**BIORETENTION (Irregular)** Listed below (Recalc)
#2	86.00'	2,826 cf	**BIORETENTION (Irregular)** Listed below (Recalc)
		4,512 cf	Total Available Storage

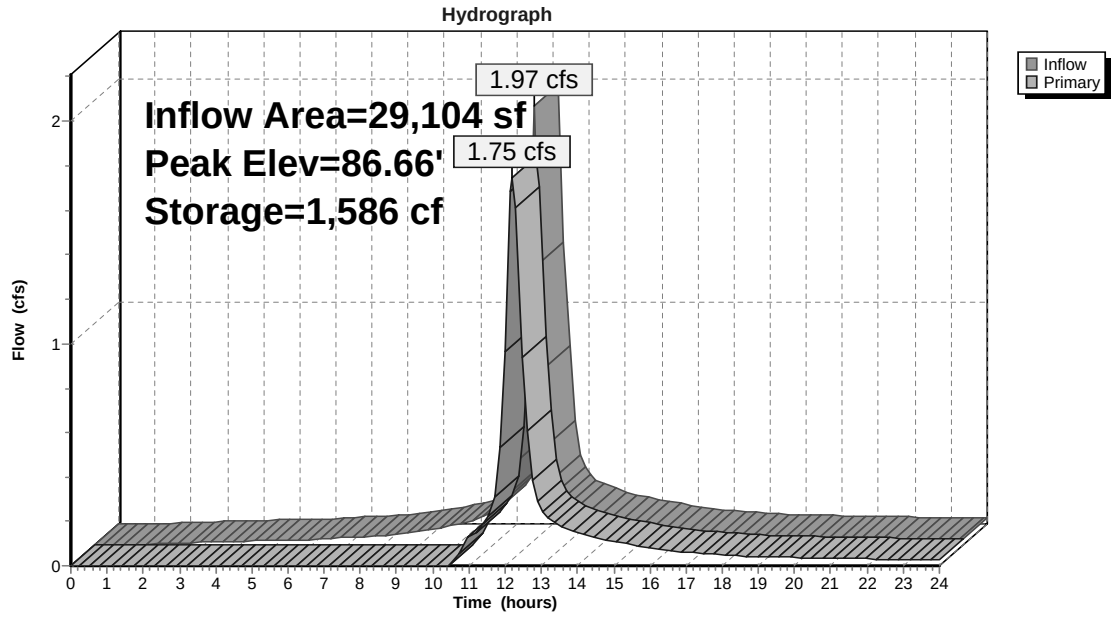
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	688	169.0	0	0	688
87.00	1,281	203.0	969	969	1,712
87.50	1,592	212.0	717	1,686	2,026

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	1,285	193.0	0	0	1,285
87.00	2,098	271.0	1,675	1,675	4,174
87.50	2,512	280.0	1,151	2,826	4,592

Device	Routing	Invert	Outlet Devices
#1	Primary	83.37'	**12.0" Round 12" HDPE** L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 83.37' / 83.14' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Device 1	86.50'	**8.0" Horiz. (4) 8" OVERFLOW X 4.00** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.65 cfs @ 12.21 hrs HW=86.65' TW=85.77' (Dynamic Tailwater)
└─**1=12" HDPE** (Passes 1.65 cfs of 3.13 cfs potential flow)
└─**2=(4) 8" OVERFLOW** (Weir Controls 1.65 cfs @ 1.28 fps)

Pond P1D: BIO.RET. #2



Summary for Pond P2: SURFACE BASINS

Inflow Area = 143,195 sf, 51.23% Impervious, Inflow Depth > 3.12" for 10-Year event
Inflow = 9.02 cfs @ 12.16 hrs, Volume= 37,240 cf
Outflow = 2.09 cfs @ 12.66 hrs, Volume= 17,999 cf, Atten= 77%, Lag= 30.1 min
Primary = 0.59 cfs @ 12.66 hrs, Volume= 5,052 cf
Secondary = 1.50 cfs @ 12.66 hrs, Volume= 12,947 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 84.62' @ 12.66 hrs Surf.Area= 23,359 sf Storage= 21,631 cf
Flood Elev= 84.50' Surf.Area= 22,488 sf Storage= 18,822 cf

Plug-Flow detention time= 273.1 min calculated for 17,887 cf (48% of inflow)
Center-of-Mass det. time= 149.8 min (936.1 - 786.3)

Volume	Invert	Avail.Storage	Storage Description
#1	83.50'	44,950 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

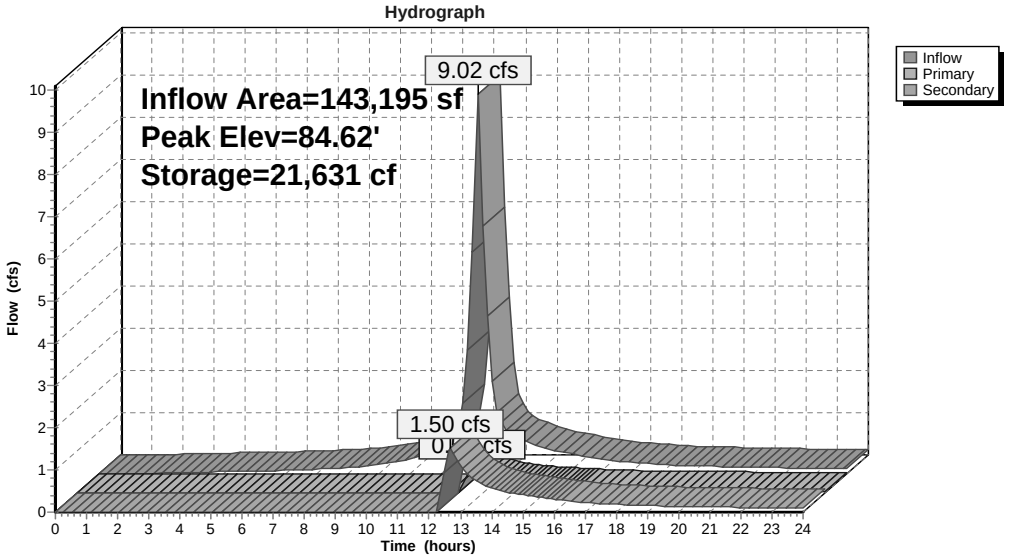
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
83.50	15,229	2,415.0	0	0	15,229
84.00	18,845	2,424.0	8,502	8,502	18,900
84.50	22,488	2,434.0	10,320	18,822	22,952
85.00	26,145	2,443.0	12,147	30,969	26,651
85.50	29,817	2,452.0	13,980	44,950	30,365

Device	Routing	Invert	Outlet Devices
#1	Primary	82.40'	**6.0" Round (2) 6" PVC X 2.00** L= 140.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 82.40' / 81.00' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf
#2	Device 1	84.50'	**8.0" Horiz. (2) 8" OVERFLOW X 2.00** C= 0.600 Limited to weir flow at low heads
#3	Secondary	84.50'	**15.0' long x 5.0' breadth RIP-RAP OVERFLOW** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.56 cfs @ 12.66 hrs HW=84.62' TW=0.00' (Dynamic Tailwater)
└─**1=(2) 6" PVC** (Passes 0.56 cfs of 1.96 cfs potential flow)
└─**2=(2) 8" OVERFLOW** (Weir Controls 0.56 cfs @ 1.13 fps)

Secondary OutFlow Max=1.43 cfs @ 12.66 hrs HW=84.62' TW=82.58' (Dynamic Tailwater)
└─**3=RIP-RAP OVERFLOW** (Weir Controls 1.43 cfs @ 0.81 fps)

Pond P2: SURFACE BASINS

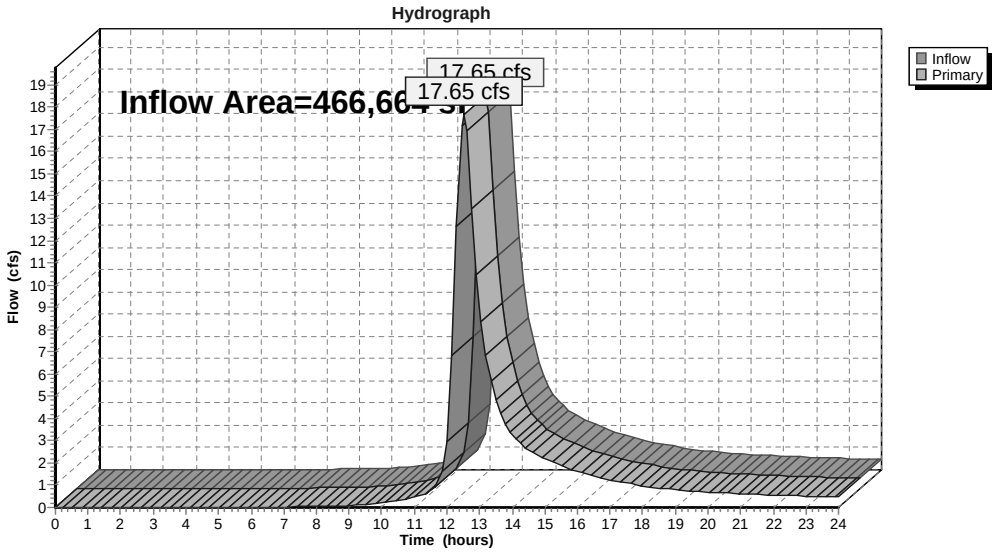


Summary for Link SP-1: STUDY POINT #1

Inflow Area = 466,664 sf, 60.79% Impervious, Inflow Depth > 2.77" for 10-Year event
Inflow = 17.65 cfs @ 12.52 hrs, Volume= 107,632 cf
Primary = 17.65 cfs @ 12.52 hrs, Volume= 107,632 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-1: STUDY POINT #1

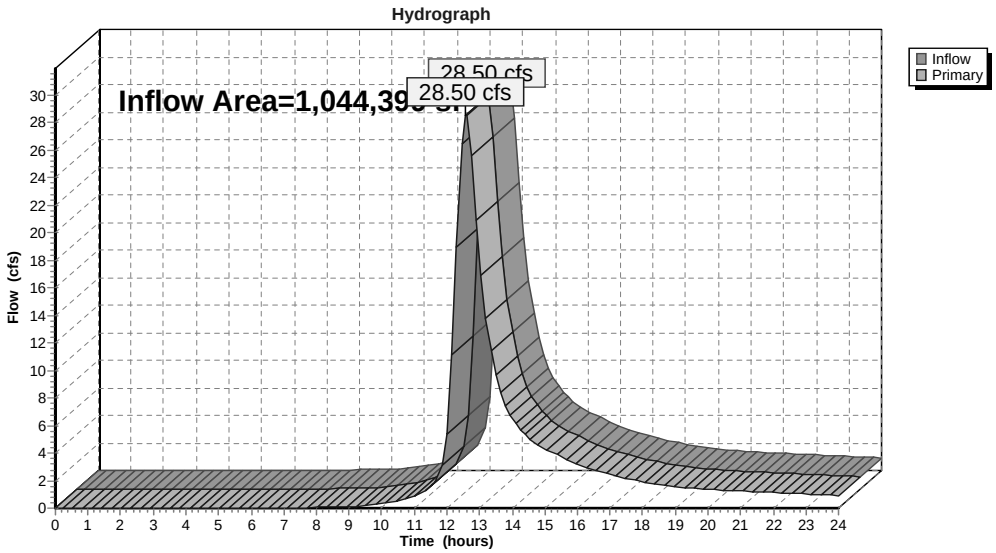


Summary for Link SP-2: STUDY POINT #2

Inflow Area = 1,044,399 sf, 36.80% Impervious, Inflow Depth > 2.30" for 10-Year event
Inflow = 28.50 cfs @ 12.59 hrs, Volume= 200,127 cf
Primary = 28.50 cfs @ 12.59 hrs, Volume= 200,127 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-2: STUDY POINT #2

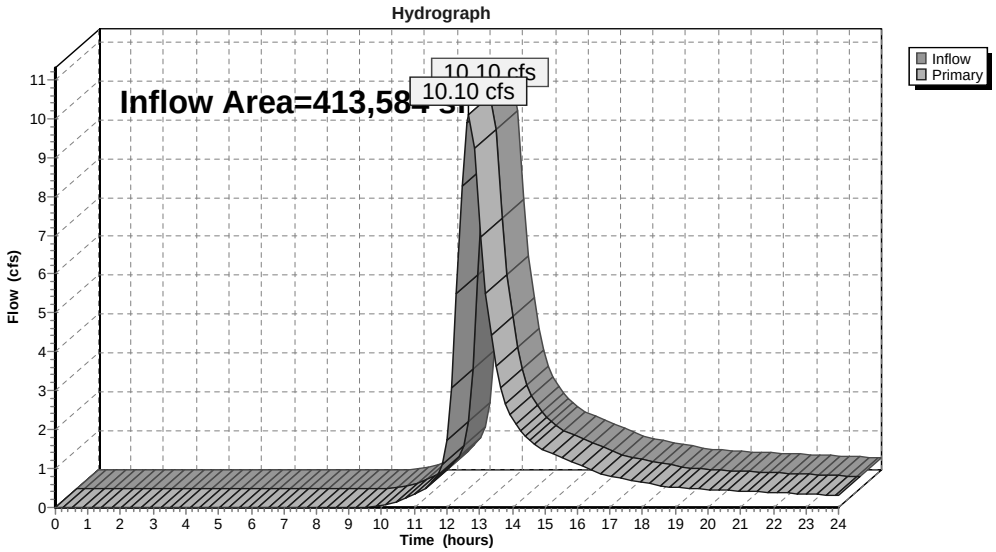


Summary for Link SP-3: STUDY POINT #3

Inflow Area = 413,584 sf, 4.62% Impervious, Inflow Depth > 2.03" for 10-Year event
Inflow = 10.10 cfs @ 12.67 hrs, Volume= 69,949 cf
Primary = 10.10 cfs @ 12.67 hrs, Volume= 69,949 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-3: STUDY POINT #3



Summary for Subcatchment P-1A: PR. WATERSHED

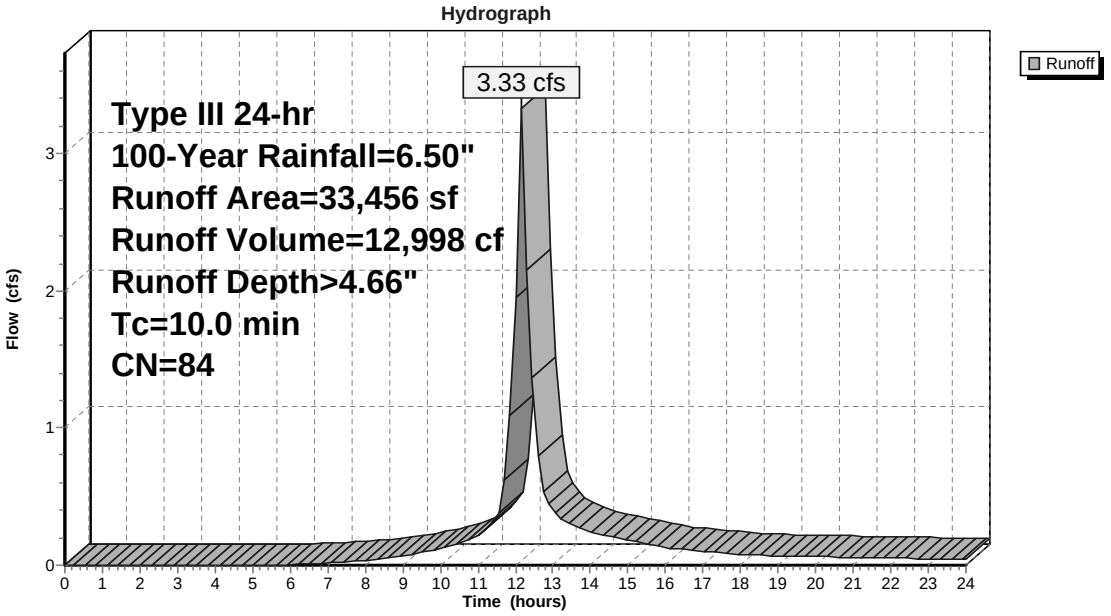
Runoff = 3.33 cfs @ 12.16 hrs, Volume= 12,998 cf, Depth> 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	14,724	74	>75% Grass cover, Good, HSG C
	5,389	79	Woods/grass comb., Good, HSG D
*	13,343	98	Impervious
	33,456	84	Weighted Average
	20,113		60.12% Pervious Area
	13,343		39.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1A: PR. WATERSHED



Summary for Subcatchment P-1B: PR. WATERSHED

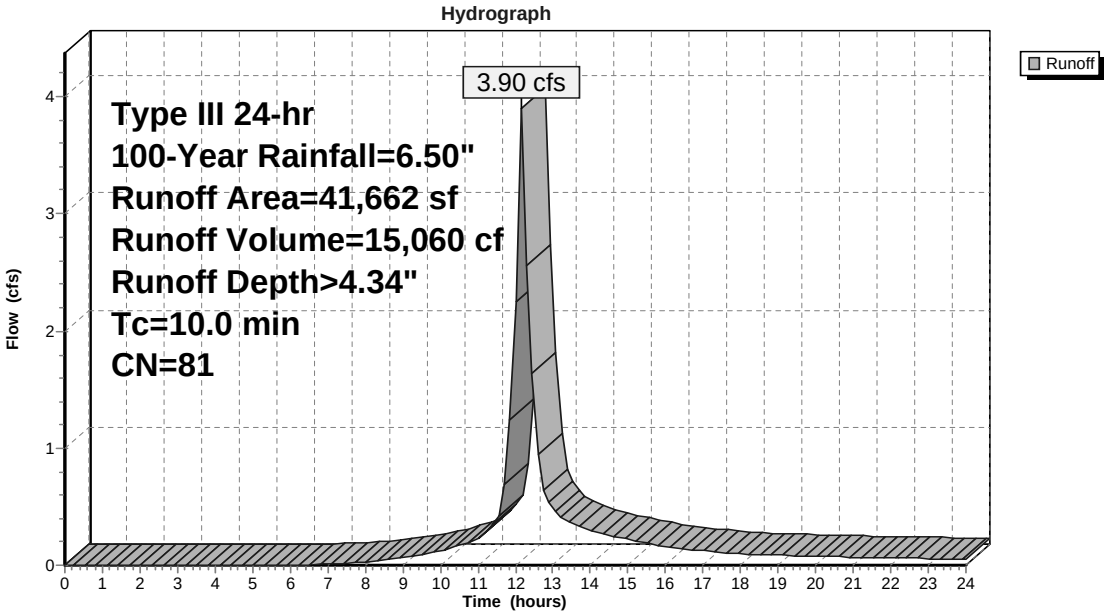
Runoff = 3.90 cfs @ 12.16 hrs, Volume= 15,060 cf, Depth> 4.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	17,448	74	>75% Grass cover, Good, HSG C
	15,311	79	Woods/grass comb., Good, HSG D
*	8,903	98	Impervious
	41,662	81	Weighted Average
	32,759		78.63% Pervious Area
	8,903		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1B: PR. WATERSHED



Summary for Subcatchment P-1C: PR. WATERSHED

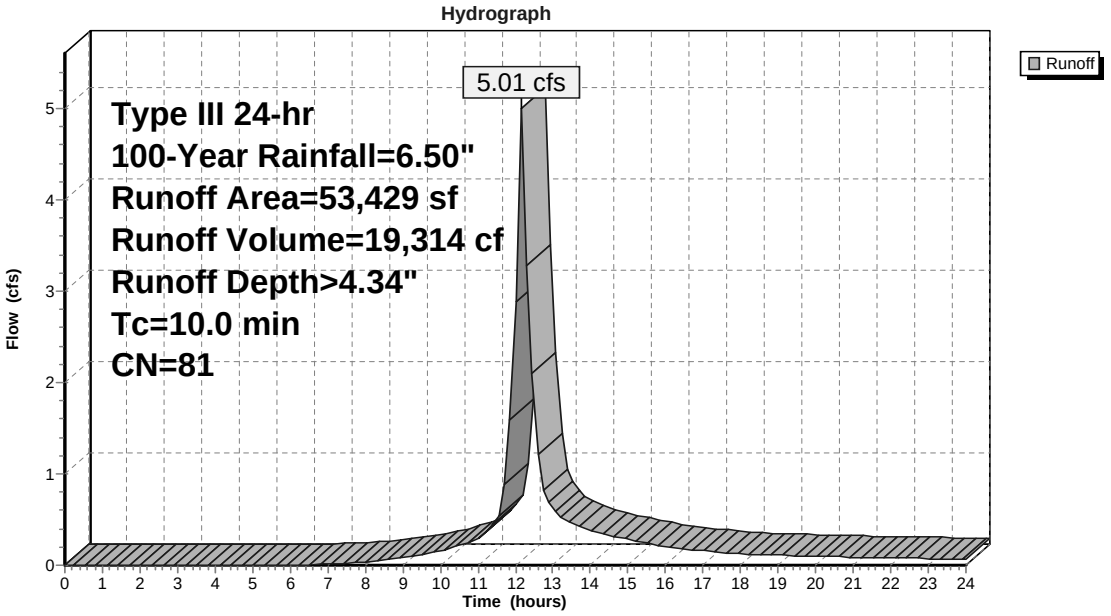
Runoff = 5.01 cfs @ 12.16 hrs, Volume= 19,314 cf, Depth> 4.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	20,039	74	>75% Grass cover, Good, HSG C
	22,761	79	Woods/grass comb., Good, HSG D
*	10,629	98	Impervious
	53,429	81	Weighted Average
	42,800		80.11% Pervious Area
	10,629		19.89% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1C: PR. WATERSHED



Summary for Subcatchment P-1D: PR. WATERSHED

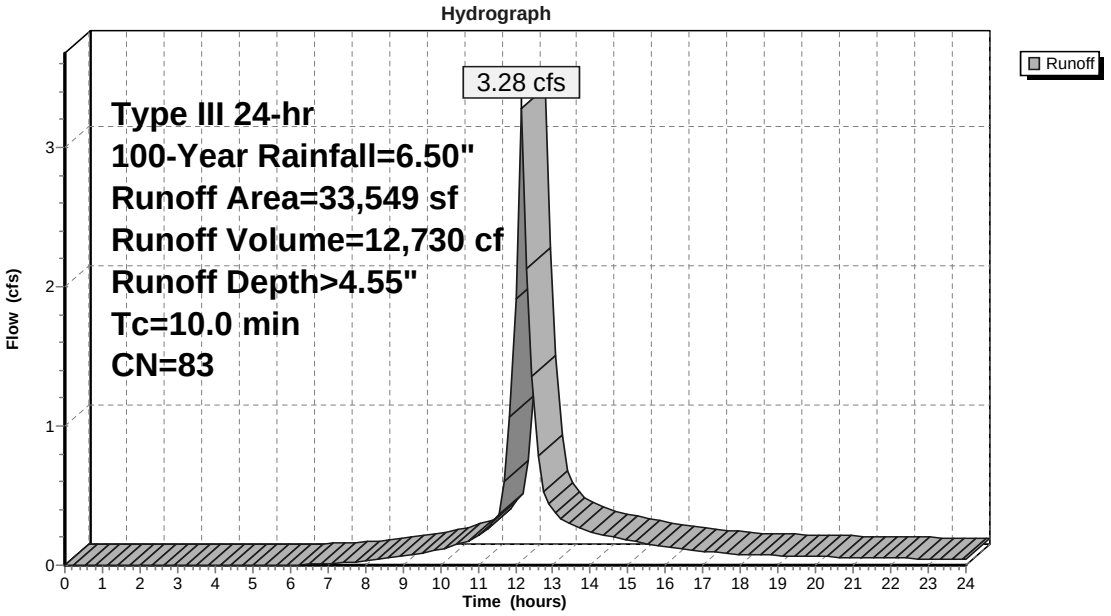
Runoff = 3.28 cfs @ 12.16 hrs, Volume= 12,730 cf, Depth> 4.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	19,032	74	>75% Grass cover, Good, HSG C
	2,786	79	Woods/grass comb., Good, HSG D
*	11,731	98	Impervious
	33,549	83	Weighted Average
	21,818		65.03% Pervious Area
	11,731		34.97% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1D: PR. WATERSHED



Summary for Subcatchment P-1E: PR. WATERSHED

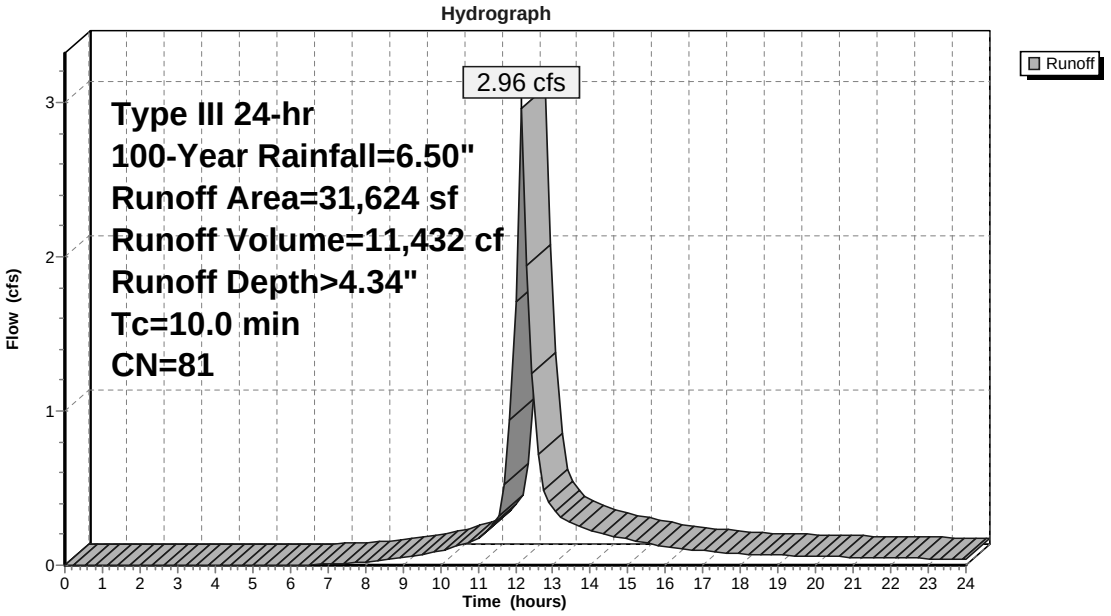
Runoff = 2.96 cfs @ 12.16 hrs, Volume= 11,432 cf, Depth> 4.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	22,158	74	>75% Grass cover, Good, HSG C
*	9,466	98	Impervious
	31,624	81	Weighted Average
	22,158		70.07% Pervious Area
	9,466		29.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1E: PR. WATERSHED



Summary for Subcatchment P-1F: PR. WATERSHED

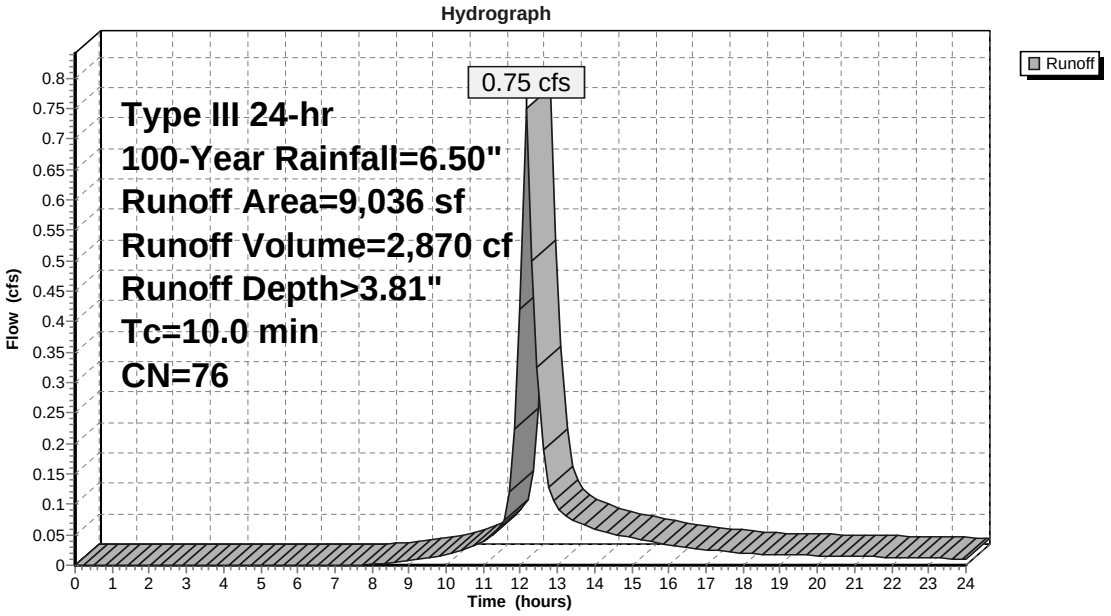
Runoff = 0.75 cfs @ 12.16 hrs, Volume= 2,870 cf, Depth> 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	8,156	74	>75% Grass cover, Good, HSG C
*	880	98	Impervious
	9,036	76	Weighted Average
	8,156		90.26% Pervious Area
	880		9.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1F: PR. WATERSHED



Summary for Subcatchment P-1G: PR. WATERSHED

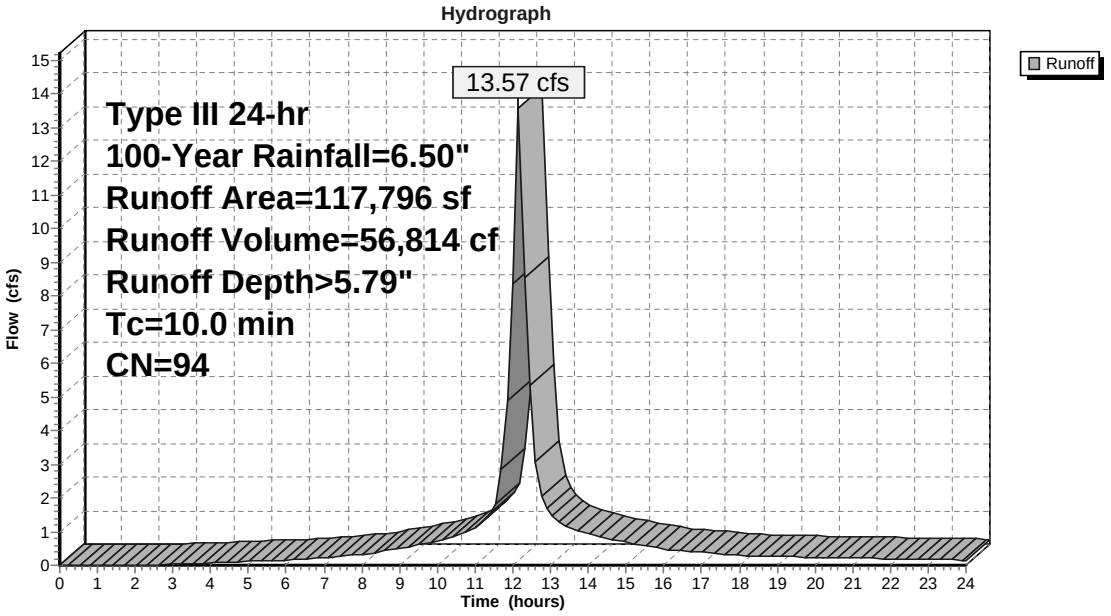
Runoff = 13.57 cfs @ 12.15 hrs, Volume= 56,814 cf, Depth> 5.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	21,862	74	>75% Grass cover, Good, HSG C
*	95,934	98	Impervious
	117,796	94	Weighted Average
	21,862		18.56% Pervious Area
	95,934		81.44% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1G: PR. WATERSHED



Summary for Subcatchment P-1H: PR. WATERSHED

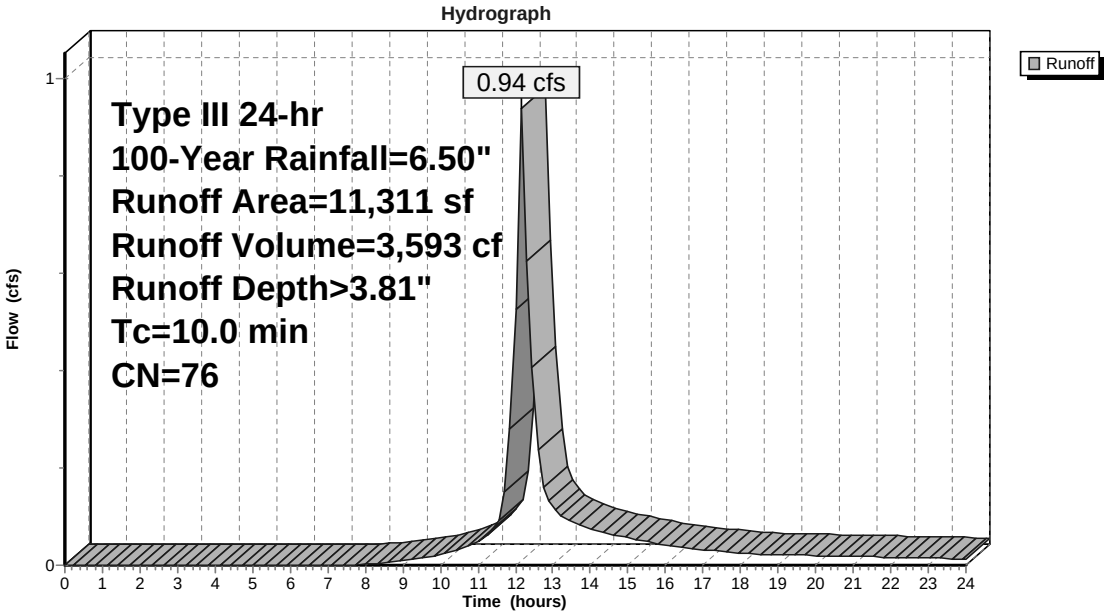
Runoff = 0.94 cfs @ 12.16 hrs, Volume= 3,593 cf, Depth> 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	10,351	74	>75% Grass cover, Good, HSG C
*	960	98	Impervious
	11,311	76	Weighted Average
	10,351		91.51% Pervious Area
	960		8.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1H: PR. WATERSHED



Summary for Subcatchment P-1I: PR. WATERSHED

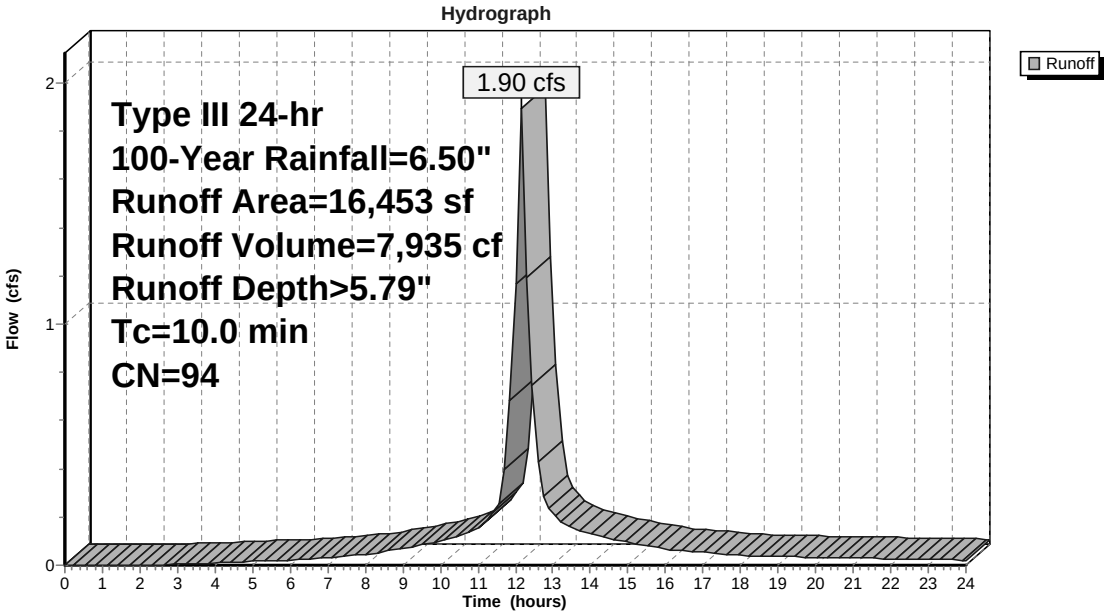
Runoff = 1.90 cfs @ 12.15 hrs, Volume= 7,935 cf, Depth> 5.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	2,946	74	>75% Grass cover, Good, HSG C
*	13,507	98	Impervious
	16,453	94	Weighted Average
	2,946		17.91% Pervious Area
	13,507		82.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-1I: PR. WATERSHED



Summary for Subcatchment P-2A: PR. WATERSHED

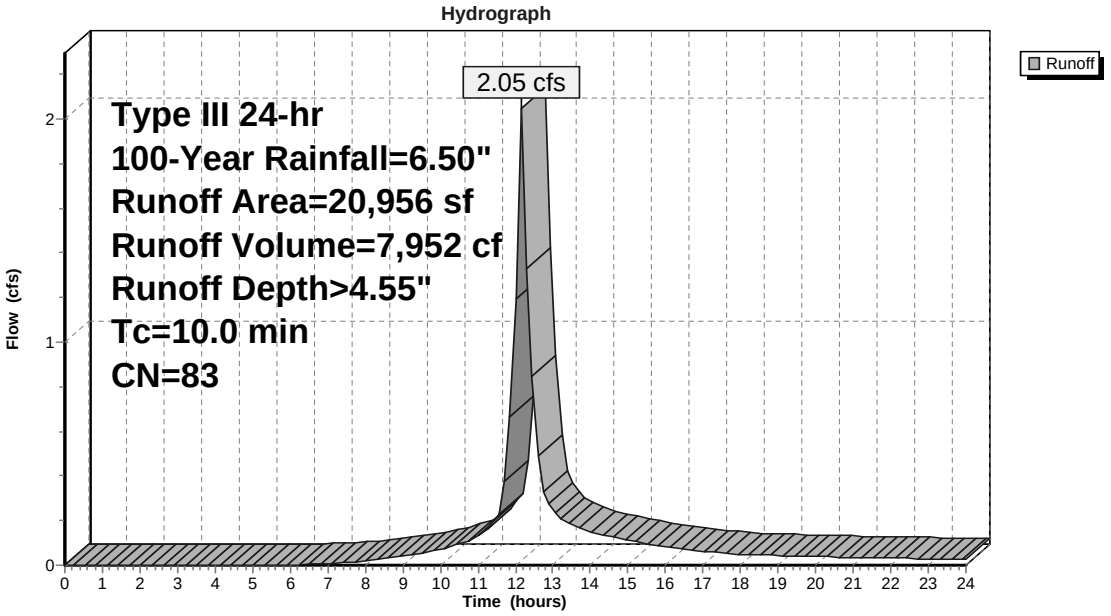
Runoff = 2.05 cfs @ 12.16 hrs, Volume= 7,952 cf, Depth> 4.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	12,753	74	>75% Grass cover, Good, HSG C
*	8,203	98	Impervious
	20,956	83	Weighted Average
	12,753		60.86% Pervious Area
	8,203		39.14% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-2A: PR. WATERSHED



Summary for Subcatchment P-2B: PR. WATERSHED

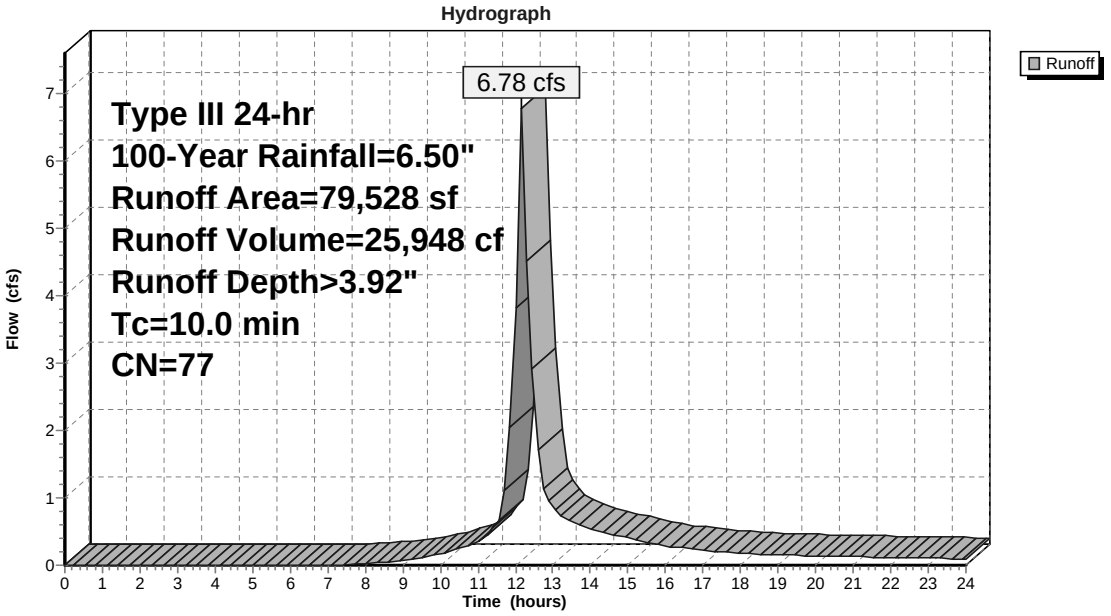
Runoff = 6.78 cfs @ 12.16 hrs, Volume= 25,948 cf, Depth> 3.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	69,830	74	>75% Grass cover, Good, HSG C
*	9,698	98	Impervious
	79,528	77	Weighted Average
	69,830		87.81% Pervious Area
	9,698		12.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment P-2B: PR. WATERSHED



Summary for Subcatchment P-3: PR. WATERSHED

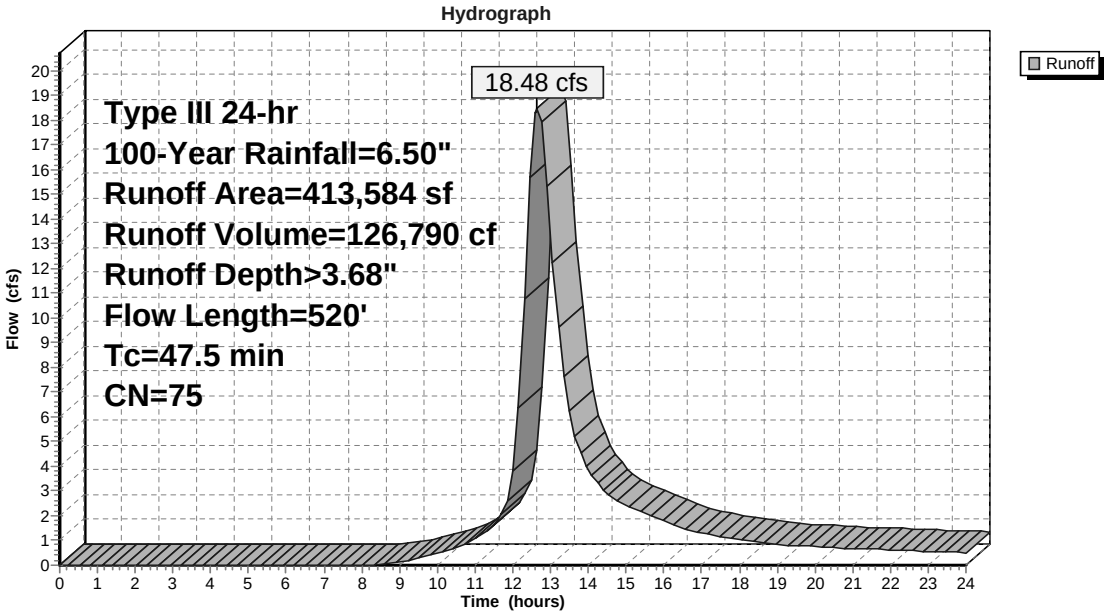
Runoff = 18.48 cfs @ 12.66 hrs, Volume= 126,790 cf, Depth> 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
	3,048	74	>75% Grass cover, Good, HSG C
	235,711	77	Woods, Good, HSG D
*	19,107	98	Impervious
	155,718	70	Woods, Good, HSG C
	413,584	75	Weighted Average
	394,477		95.38% Pervious Area
	19,107		4.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.3	100	0.0100	0.06		**Sheet Flow, A-B** Woods: Light underbrush n= 0.400 P2= 3.20"
1.3	40	0.0100	0.50		**Shallow Concentrated Flow, B-C** Woodland Kv= 5.0 fps
17.9	380	0.0050	0.35		**Shallow Concentrated Flow, C-D** Woodland Kv= 5.0 fps
47.5	520	Total			

Subcatchment P-3: PR. WATERSHED



Summary for Subcatchment R-1: BUILDING #3 ROOF

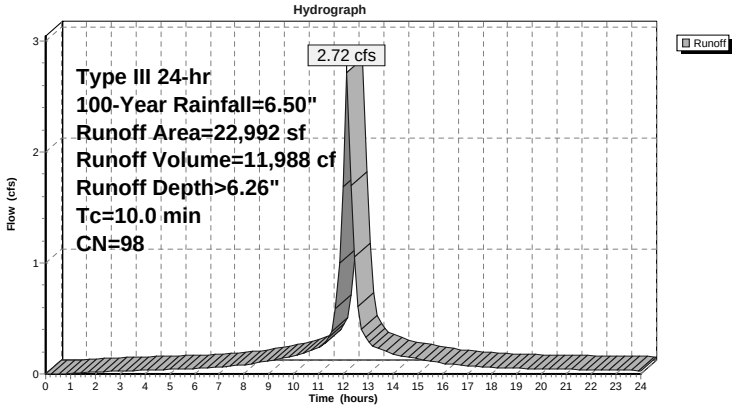
Runoff = 2.72 cfs @ 12.15 hrs, Volume= 11,988 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
*	22,992	98	Roof
	22,992		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1: BUILDING #3 ROOF



Summary for Subcatchment R-1F: BUILDING #3 ROOF

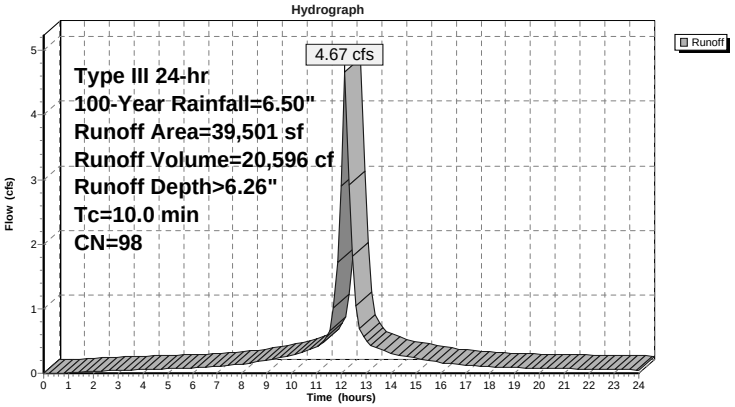
Runoff = 4.67 cfs @ 12.15 hrs, Volume= 20,596 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
*	39,501	98	Roof
	39,501		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1F: BUILDING #3 ROOF



Summary for Subcatchment R-1G: BUILDING #3 ROOF

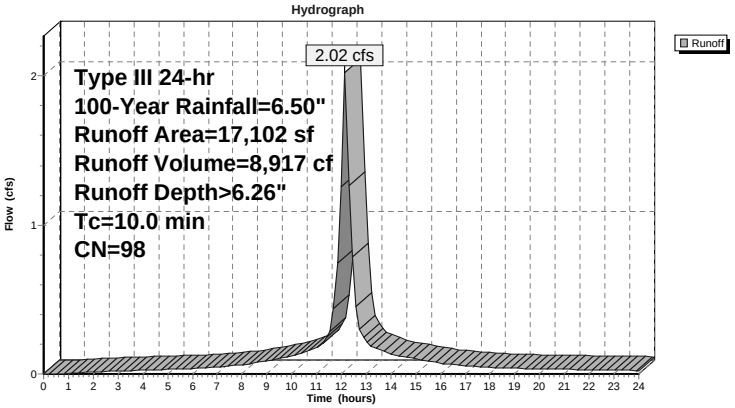
Runoff = 2.02 cfs @ 12.15 hrs, Volume= 8,917 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
*	17,102	98	Roof
	17,102		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1G: BUILDING #3 ROOF



Summary for Subcatchment R-1H: BUILDING #3 ROOF

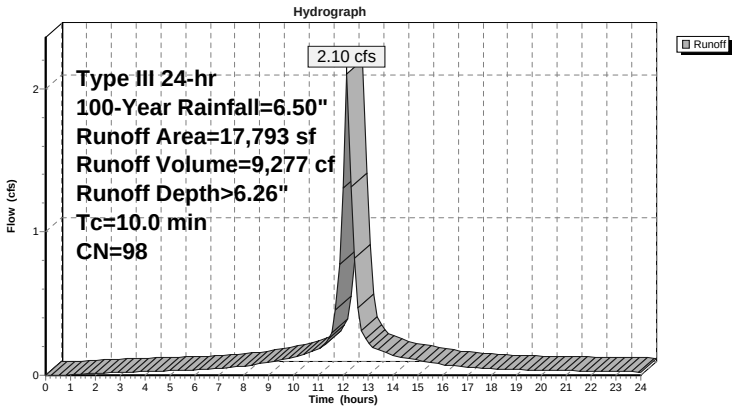
Runoff = 2.10 cfs @ 12.15 hrs, Volume= 9,277 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
*	17,793	98	Roof
	17,793		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1H: BUILDING #3 ROOF



Summary for Subcatchment R-1I: BUILDING #3 ROOF

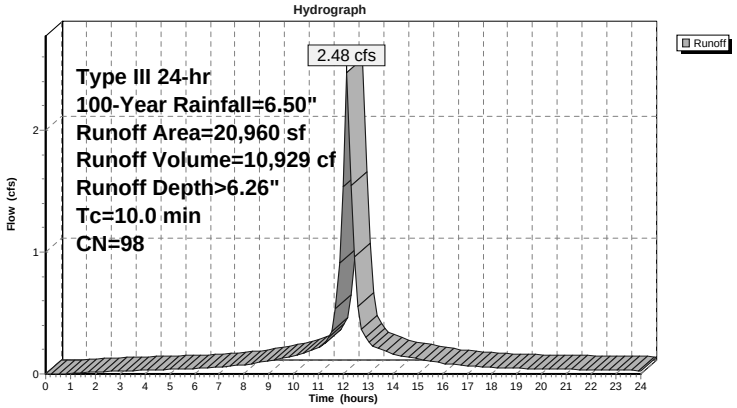
Runoff = 2.48 cfs @ 12.15 hrs, Volume= 10,929 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
*	20,960	98	Roof
	20,960		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-1I: BUILDING #3 ROOF



Summary for Subcatchment R-2A: BUILDING #1 ROOF

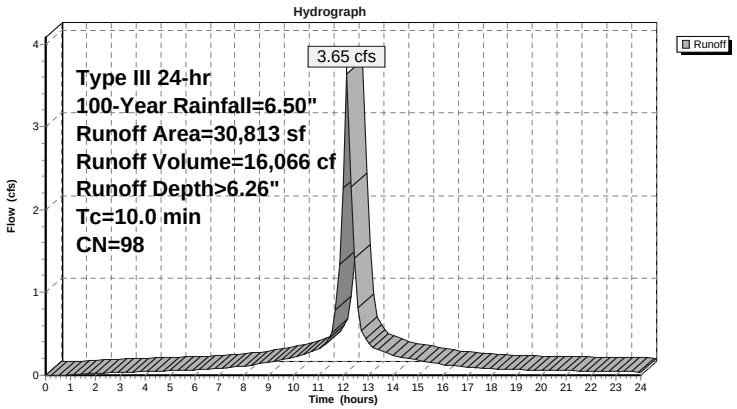
Runoff = 3.65 cfs @ 12.15 hrs, Volume= 16,066 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
*	30,813	98	Roof
	30,813		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-2A: BUILDING #1 ROOF



Summary for Subcatchment R-2B: BUILDING #2 ROOF

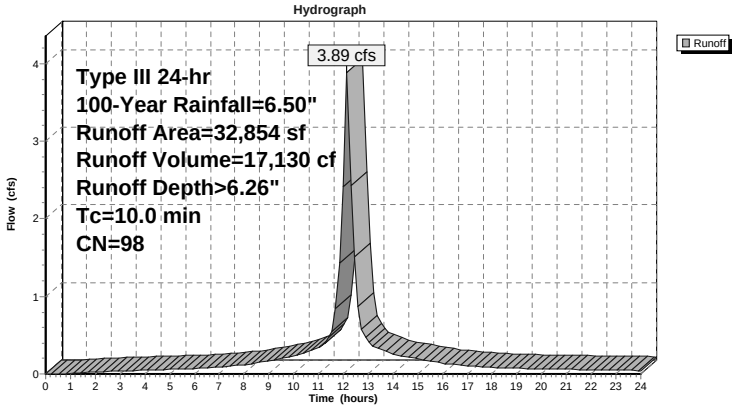
Runoff = 3.89 cfs @ 12.15 hrs, Volume= 17,130 cf, Depth> 6.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs
Type III 24-hr 100-Year Rainfall=6.50"

	Area (sf)	CN	Description
*	32,854	98	Roof
	32,854		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					**Direct Entry, MIN. TC**

Subcatchment R-2B: BUILDING #2 ROOF



Summary for Reach R-1A: DRAINAGE DITCH

Inflow Area = 466,664 sf, 60.79% Impervious, Inflow Depth > 4.65" for 100-Year event
Inflow = 31.11 cfs @ 12.40 hrs, Volume= 180,692 cf
Outflow = 31.08 cfs @ 12.41 hrs, Volume= 180,591 cf, Atten= 0%, Lag= 0.5 min

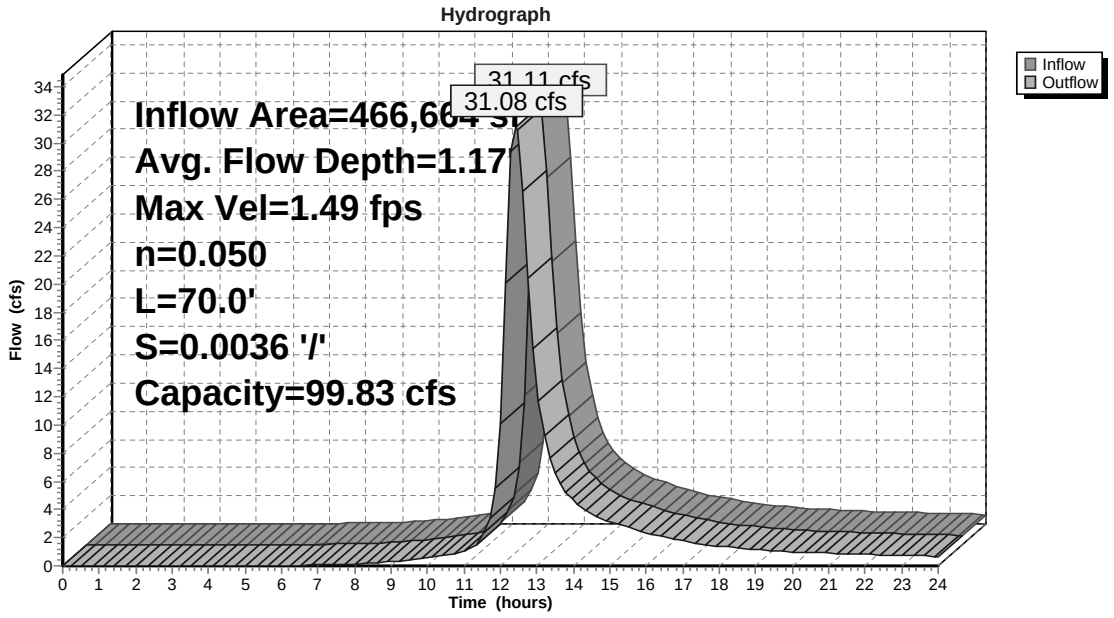
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 1.49 fps, Min. Travel Time= 0.8 min
Avg. Velocity = 0.49 fps, Avg. Travel Time= 2.4 min

Peak Storage= 1,454 cf @ 12.41 hrs
Average Depth at Peak Storage= 1.17' , Surface Width= 26.73'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 99.83 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 70.0' Slope= 0.0036 '/'
Inlet Invert= 78.75', Outlet Invert= 78.50'

‡

Reach R-1A: DRAINAGE DITCH



Summary for Reach R-1B: DRAINAGE DITCH

Inflow Area = 433,208 sf, 62.41% Impervious, Inflow Depth > 4.67" for 100-Year event
Inflow = 30.95 cfs @ 12.33 hrs, Volume= 168,504 cf
Outflow = 29.56 cfs @ 12.42 hrs, Volume= 167,694 cf, Atten= 4%, Lag= 5.8 min

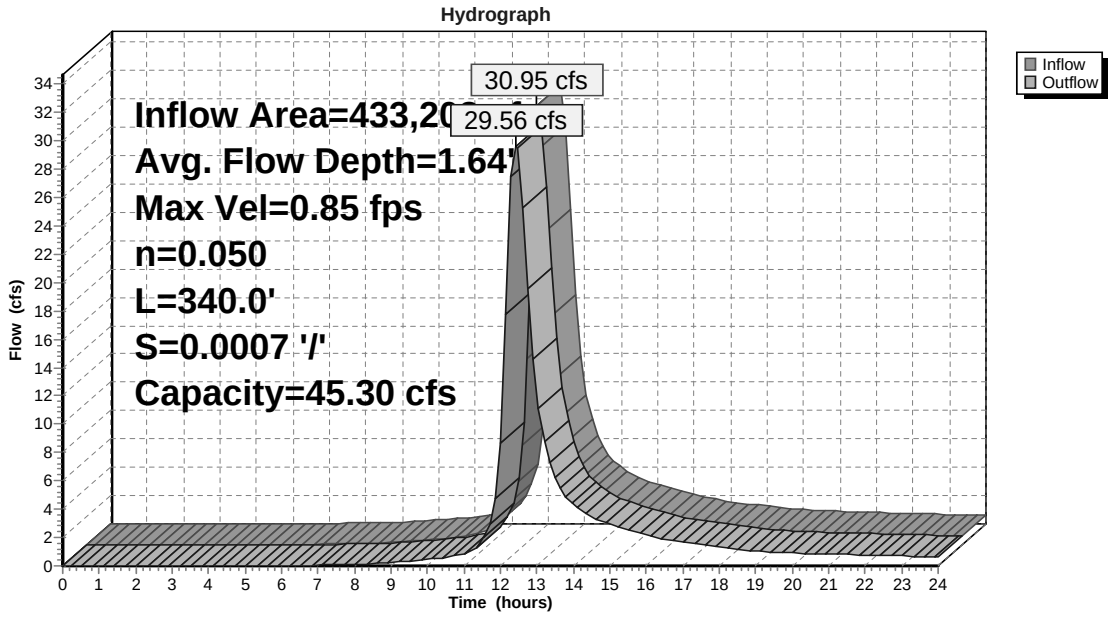
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.85 fps, Min. Travel Time= 6.7 min
Avg. Velocity = 0.27 fps, Avg. Travel Time= 21.0 min

Peak Storage= 11,801 cf @ 12.42 hrs
Average Depth at Peak Storage= 1.64' , Surface Width= 31.71'
Bank-Full Depth= 2.00' Flow Area= 46.7 sf, Capacity= 45.30 cfs

35.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 340.0' Slope= 0.0007 '/'
Inlet Invert= 79.00', Outlet Invert= 78.75'

‡

Reach R-1B: DRAINAGE DITCH



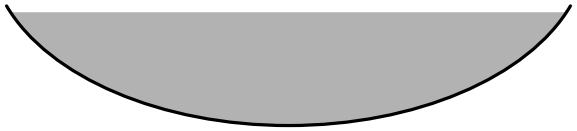
Summary for Reach R-1C: DRAINAGE DITCH

Inflow Area = 354,133 sf, 64.10% Impervious, Inflow Depth > 4.72" for 100-Year event
Inflow = 28.51 cfs @ 12.22 hrs, Volume= 139,382 cf
Outflow = 25.65 cfs @ 12.36 hrs, Volume= 138,462 cf, Atten= 10%, Lag= 8.6 min

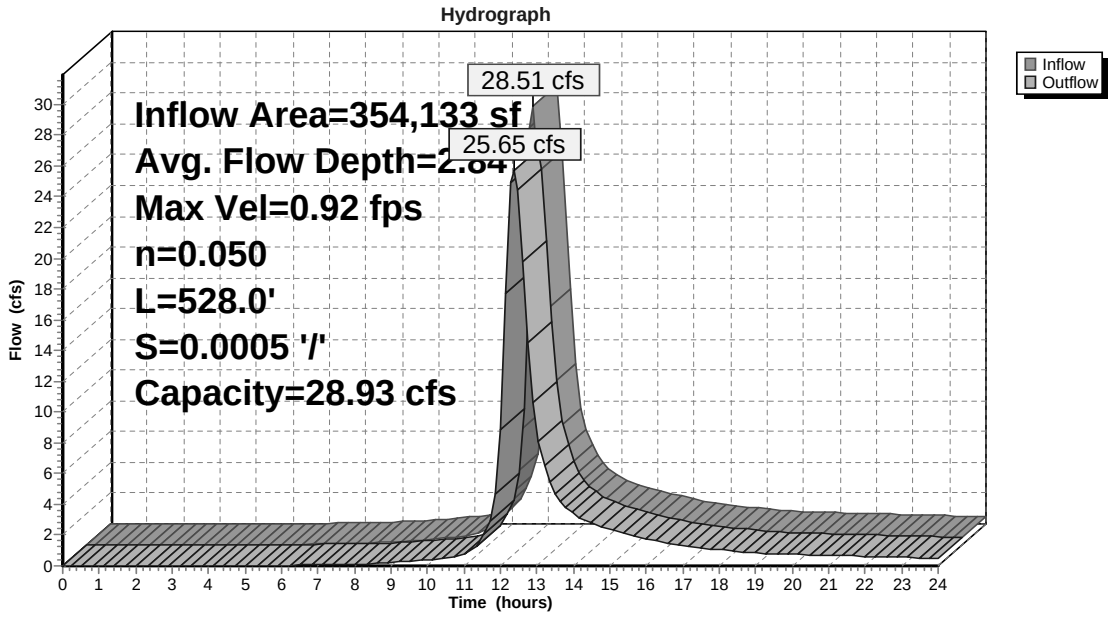
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 0.92 fps, Min. Travel Time= 9.5 min
Avg. Velocity = 0.30 fps, Avg. Travel Time= 29.3 min

Peak Storage= 14,561 cf @ 12.36 hrs
Average Depth at Peak Storage= 2.84' , Surface Width= 14.59'
Bank-Full Depth= 3.00' Flow Area= 30.0 sf, Capacity= 28.93 cfs

15.00' x 3.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0005 '/'
Inlet Invert= 79.25', Outlet Invert= 79.00'



Reach R-1C: DRAINAGE DITCH



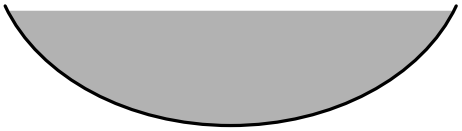
Summary for Reach R-1D: DRAINAGE DITCH

Inflow Area = 277,712 sf, 69.63% Impervious, Inflow Depth > 4.69" for 100-Year event
Inflow = 22.88 cfs @ 12.20 hrs, Volume= 108,560 cf
Outflow = 21.93 cfs @ 12.29 hrs, Volume= 108,080 cf, Atten= 4%, Lag= 5.5 min

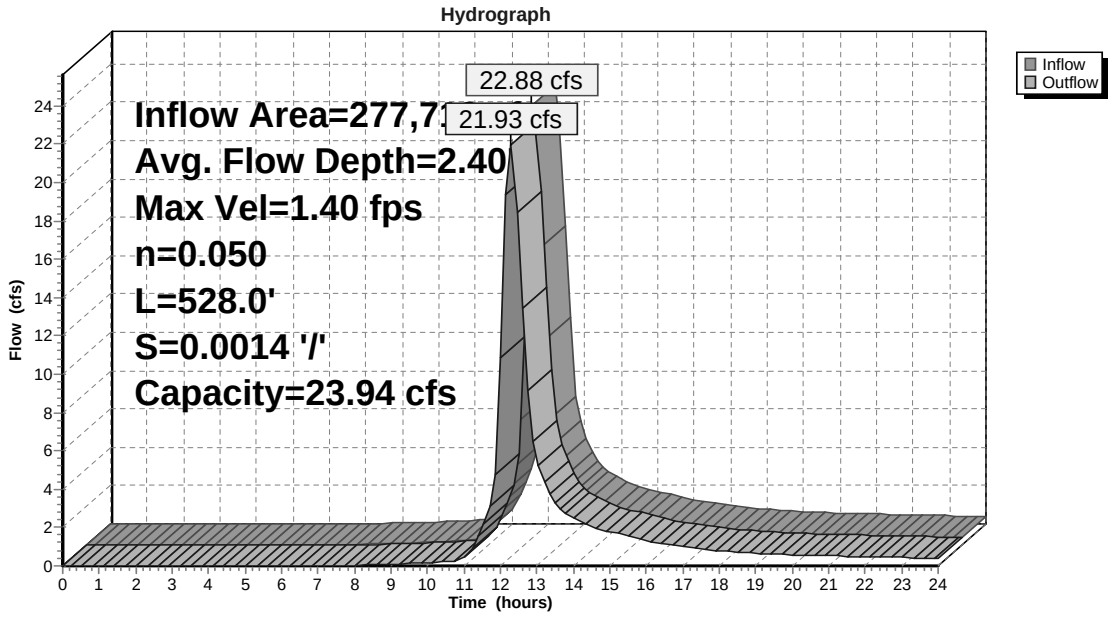
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 1.40 fps, Min. Travel Time= 6.3 min
Avg. Velocity = 0.50 fps, Avg. Travel Time= 17.5 min

Peak Storage= 8,264 cf @ 12.29 hrs
Average Depth at Peak Storage= 2.40' , Surface Width= 9.79'
Bank-Full Depth= 2.50' Flow Area= 16.7 sf, Capacity= 23.94 cfs

10.00' x 2.50' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 528.0' Slope= 0.0014 '/'
Inlet Invert= 80.00', Outlet Invert= 79.25'



Reach R-1D: DRAINAGE DITCH



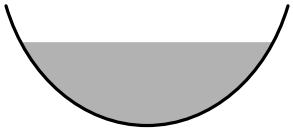
Summary for Reach R-1E: DRAINAGE DITCH

Inflow Area = 31,624 sf, 29.93% Impervious, Inflow Depth > 14.81" for 100-Year event
Inflow = 6.82 cfs @ 12.17 hrs, Volume= 39,040 cf
Outflow = 6.65 cfs @ 12.21 hrs, Volume= 38,943 cf, Atten= 2%, Lag= 2.8 min

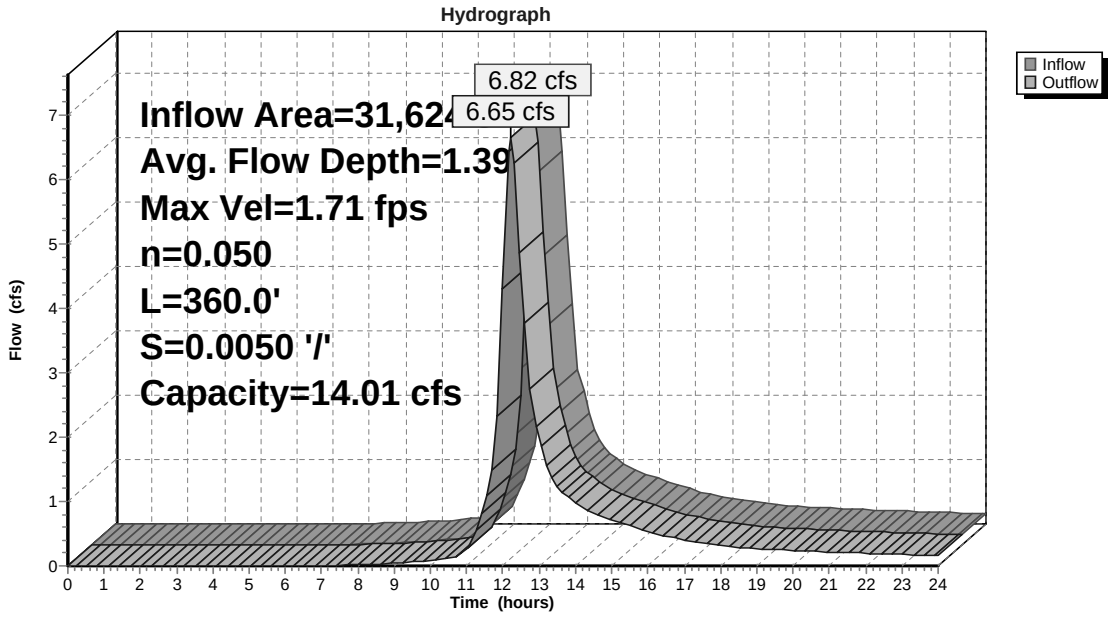
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Max. Velocity= 1.71 fps, Min. Travel Time= 3.5 min
Avg. Velocity = 0.71 fps, Avg. Travel Time= 8.5 min

Peak Storage= 1,391 cf @ 12.21 hrs
Average Depth at Peak Storage= 1.39' , Surface Width= 4.17'
Bank-Full Depth= 2.00' Flow Area= 6.7 sf, Capacity= 14.01 cfs

5.00' x 2.00' deep Parabolic Channel, n= 0.050 Scattered brush, heavy weeds
Length= 360.0' Slope= 0.0050 '/'
Inlet Invert= 81.80', Outlet Invert= 80.00'



Reach R-1E: DRAINAGE DITCH



Summary for Pond DMH1: DMH1

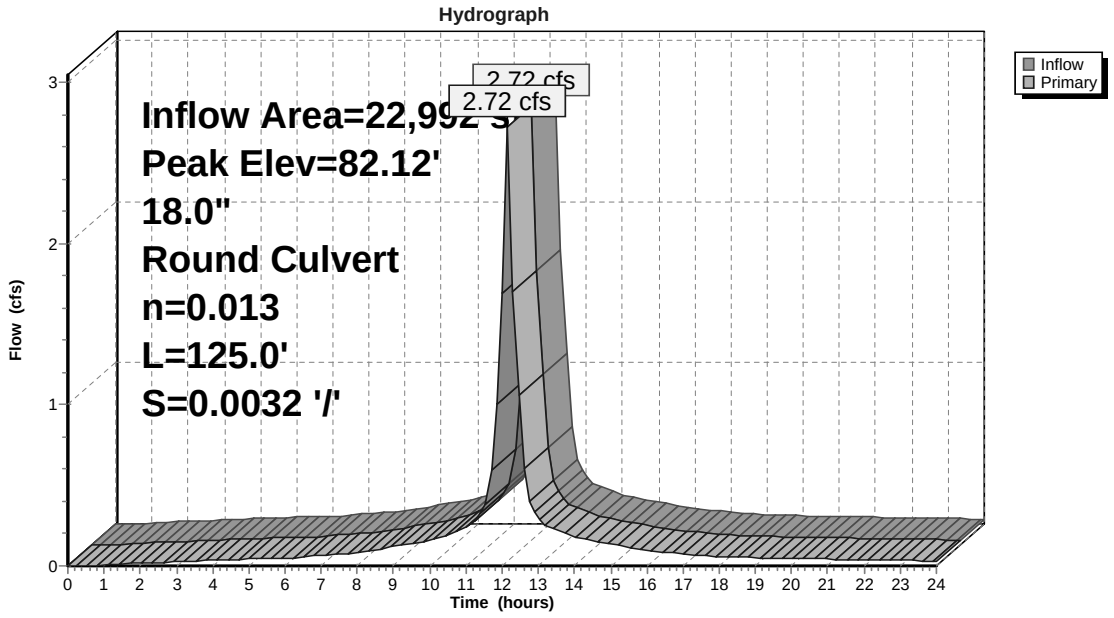
Inflow Area = 22,992 sf,100.00% Impervious, Inflow Depth > 6.26" for 100-Year event
Inflow = 2.72 cfs @ 12.15 hrs, Volume= 11,988 cf
Outflow = 2.72 cfs @ 12.15 hrs, Volume= 11,988 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.72 cfs @ 12.15 hrs, Volume= 11,988 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 82.12' @ 12.34 hrs
Flood Elev= 84.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	79.20'	**18.0" Round EX. 18" VCC** L= 125.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 79.20' / 78.80' S= 0.0032 '/' Cc= 0.900 n= 0.013 Clay tile, Flow Area= 1.77 sf

Primary OutFlow Max=3.06 cfs @ 12.15 hrs HW=81.84' TW=81.63' (Dynamic Tailwater)
↑**1=EX. 18" VCC** (Inlet Controls 3.06 cfs @ 1.73 fps)

Pond DMH1: DMH1



Summary for Pond OFFSITE: OFFSITE PONDING AREA IN GRASS

Inflow = 6.69 cfs @ 12.36 hrs, Volume= 29,058 cf
Outflow = 7.36 cfs @ 12.35 hrs, Volume= 28,816 cf, Atten= 0%, Lag= 0.0 min
Primary = 7.36 cfs @ 12.35 hrs, Volume= 28,816 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 82.68' @ 12.30 hrs Surf.Area= 3,288 sf Storage= 1,015 cf

Plug-Flow detention time= 10.8 min calculated for 28,637 cf (99% of inflow)
Center-of-Mass det. time= 6.3 min (878.9 - 872.7)

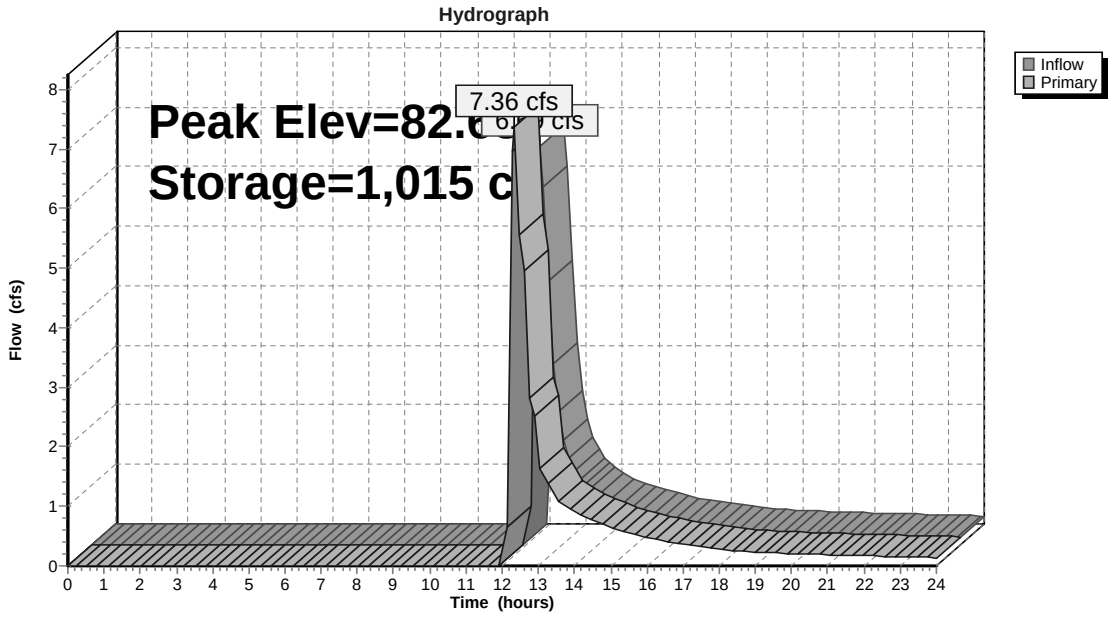
Volume	Invert	Avail.Storage	Storage Description
#1	82.10'	2,354 cf	**OFFSITE PONDING AREA (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.10	230	0	0
83.00	5,000	2,354	2,354

Device	Routing	Invert	Outlet Devices
#1	Primary	82.27'	**8.0" Round (3) 8" HDPE X 3.00 w/ 2.0" inside fill** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.10' / 81.40' S= 0.0333 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.28 sf
#2	Primary	82.60'	**100.0' long x 20.0' breadth WEIR FLOW OVER WALKING PATH** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=6.51 cfs @ 12.35 hrs HW=82.67' TW=0.00' (Dynamic Tailwater)
└─**1=(3) 8" HDPE** (Inlet Controls 1.24 cfs @ 1.65 fps)
└─**2=WEIR FLOW OVER WALKING PATH** (Weir Controls 5.27 cfs @ 0.72 fps)

Pond OFFSITE: OFFSITE PONDING AREA IN GRASS



Summary for Pond P1A: UNDERGROUND CHAMBERS #1

Inflow Area = 212,539 sf, 81.01% Impervious, Inflow Depth > 5.55" for 100-Year event
Inflow = 17.54 cfs @ 12.18 hrs, Volume= 98,305 cf
Outflow = 17.27 cfs @ 12.23 hrs, Volume= 84,495 cf, Atten= 2%, Lag= 2.7 min
Primary = 13.34 cfs @ 12.23 hrs, Volume= 56,887 cf
Secondary = 3.93 cfs @ 12.23 hrs, Volume= 27,609 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 86.22' @ 12.23 hrs Surf.Area= 14,913 sf Storage= 18,740 cf
Flood Elev= 84.00' Surf.Area= 14,913 sf Storage= 12,169 cf

Plug-Flow detention time= 111.1 min calculated for 84,495 cf (86% of inflow)
Center-of-Mass det. time= 52.0 min (833.2 - 781.2)

Volume	Invert	Avail.Storage	Storage Description
#1B	82.64'	5,589 cf	**8.17'W x 935.92'L x 2.33'H Field B** 17,834 cf Overall - 3,862 cf Embedded = 13,972 cf x 40.0% Voids
#2B	83.14'	3,862 cf	**ADS_StormTech SC-310 +Cap** x 262 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 262 Chambers in 2 Rows
#3C	82.64'	2,519 cf	**34.83'W x 102.88'L x 2.33'H Field C** 8,362 cf Overall - 2,064 cf Embedded = 6,298 cf x 40.0% Voids
#4C	83.14'	2,064 cf	**ADS_StormTech SC-310 +Cap** x 140 Inside #3 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 140 Chambers in 10 Rows
#5D	82.64'	2,002 cf	**34.83'W x 81.52'L x 2.33'H Field D** 6,626 cf Overall - 1,622 cf Embedded = 5,004 cf x 40.0% Voids
#6D	83.14'	1,622 cf	**ADS_StormTech SC-310 +Cap** x 110 Inside #5 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 110 Chambers in 10 Rows
#7E	82.64'	602 cf	**21.50'W x 38.80'L x 2.33'H Field E** 1,946 cf Overall - 442 cf Embedded = 1,504 cf x 40.0% Voids
#8E	83.14'	442 cf	**ADS_StormTech SC-310 +Cap** x 30 Inside #7

			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 30 Chambers in 6 Rows
#9	83.14'	50 cf	**4.00'D x 4.00'H DMH**
		18,752 cf	Total Available Storage

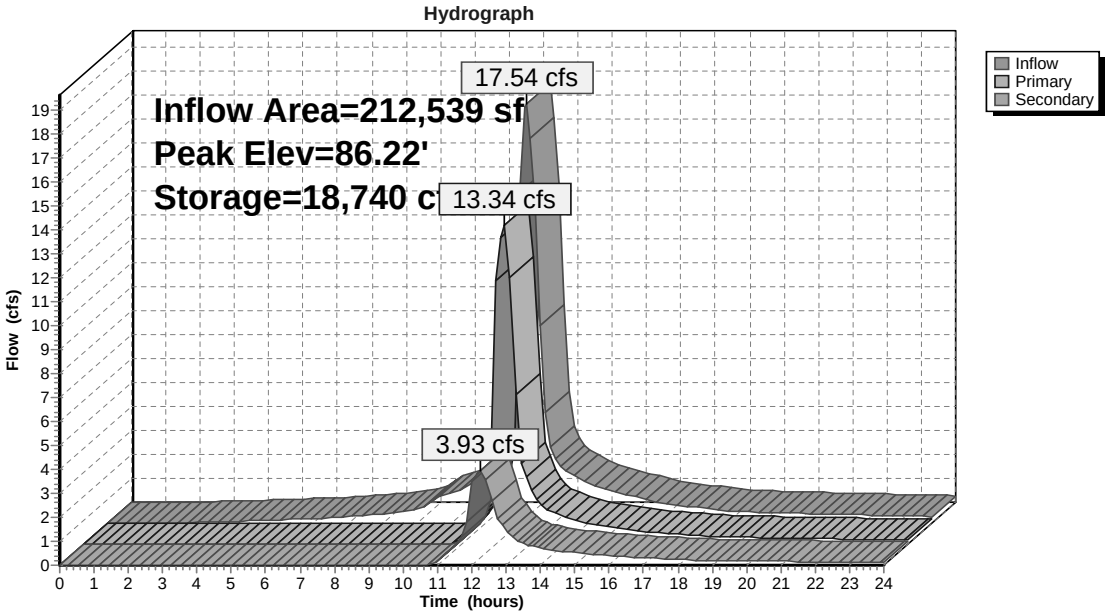
Storage Group B created with Chamber Wizard
Storage Group C created with Chamber Wizard
Storage Group D created with Chamber Wizard
Storage Group E created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	84.00'	**24.0" Round 18" HDPE AT DMH12** L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.79' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf
#2	Secondary	84.00'	**12.0" Round 12" HDPE AT DMH8** L= 26.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.74' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=12.88 cfs @ 12.23 hrs HW=86.16' TW=82.33' (Dynamic Tailwater)
↑**1=18" HDPE AT DMH12** (Inlet Controls 12.88 cfs @ 4.10 fps)

Secondary OutFlow Max=3.85 cfs @ 12.23 hrs HW=86.16' TW=83.16' (Dynamic Tailwater)
↑**2=12" HDPE AT DMH8** (Inlet Controls 3.85 cfs @ 4.90 fps)

Pond P1A: UNDERGROUND CHAMBERS #1



Summary for Pond P1B: UNDERGROUND CHAMBERS #2

Inflow Area = 37,413 sf, 92.13% Impervious, Inflow Depth > 6.05" for 100-Year event
Inflow = 4.38 cfs @ 12.15 hrs, Volume= 18,864 cf
Outflow = 3.89 cfs @ 12.20 hrs, Volume= 14,982 cf, Atten= 11%, Lag= 2.8 min
Primary = 3.89 cfs @ 12.20 hrs, Volume= 14,982 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 85.17' @ 12.20 hrs Surf.Area= 4,282 sf Storage= 5,496 cf
Flood Elev= 84.00' Surf.Area= 4,282 sf Storage= 3,589 cf

Plug-Flow detention time= 153.5 min calculated for 14,982 cf (79% of inflow)
Center-of-Mass det. time= 77.5 min (834.2 - 756.6)

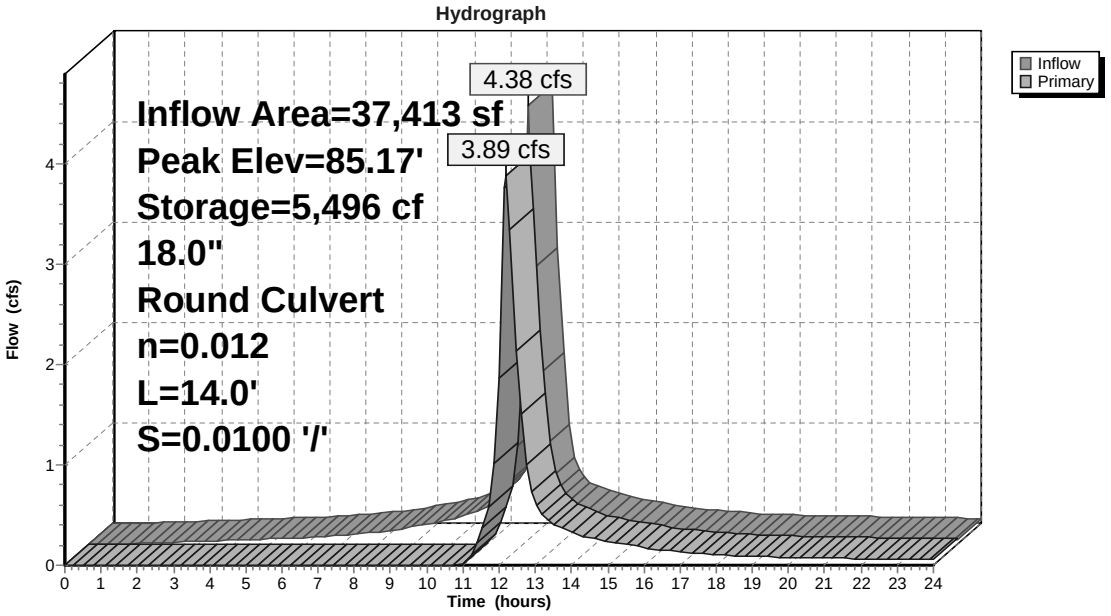
Volume	Invert	Avail.Storage	Storage Description
#1A	82.64'	2,994 cf	**41.50'W x 102.88'L x 2.33'H Field A** 9,962 cf Overall - 2,477 cf Embedded = 7,486 cf x 40.0% Voids
#2A	83.14'	2,477 cf	**ADS_StormTech SC-310 +Cap** x 168 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 168 Chambers in 12 Rows
#3	83.14'	50 cf	**4.00'D x 4.00'H DMH**
		5,521 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	84.00'	**18.0" Round 18" HDPE AT DMH3** L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 84.00' / 83.86' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.64 cfs @ 12.20 hrs HW=85.12' TW=80.39' (Dynamic Tailwater)
↑**1=18" HDPE AT DMH3** (Barrel Controls 3.64 cfs @ 3.59 fps)

Pond P1B: UNDERGROUND CHAMBERS #2



Summary for Pond P1C: BIO.RET. #1

Inflow Area = 48,537 sf, 83.20% Impervious, Inflow Depth > 5.80" for 100-Year event
Inflow = 5.42 cfs @ 12.15 hrs, Volume= 23,466 cf
Outflow = 4.46 cfs @ 12.53 hrs, Volume= 20,883 cf, Atten= 18%, Lag= 22.7 min
Primary = 4.46 cfs @ 12.53 hrs, Volume= 20,883 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 87.15' @ 12.33 hrs Surf.Area= 5,782 sf Storage= 6,086 cf
Flood Elev= 86.50' Surf.Area= 5,219 sf Storage= 2,505 cf

Plug-Flow detention time= 105.6 min calculated for 20,883 cf (89% of inflow)
Center-of-Mass det. time= 53.7 min (810.2 - 756.5)

Volume	Invert	Avail.Storage	Storage Description
#1	86.00'	8,156 cf	**BIORETENTION #1 (Irregular)**Listed below (Recalc)

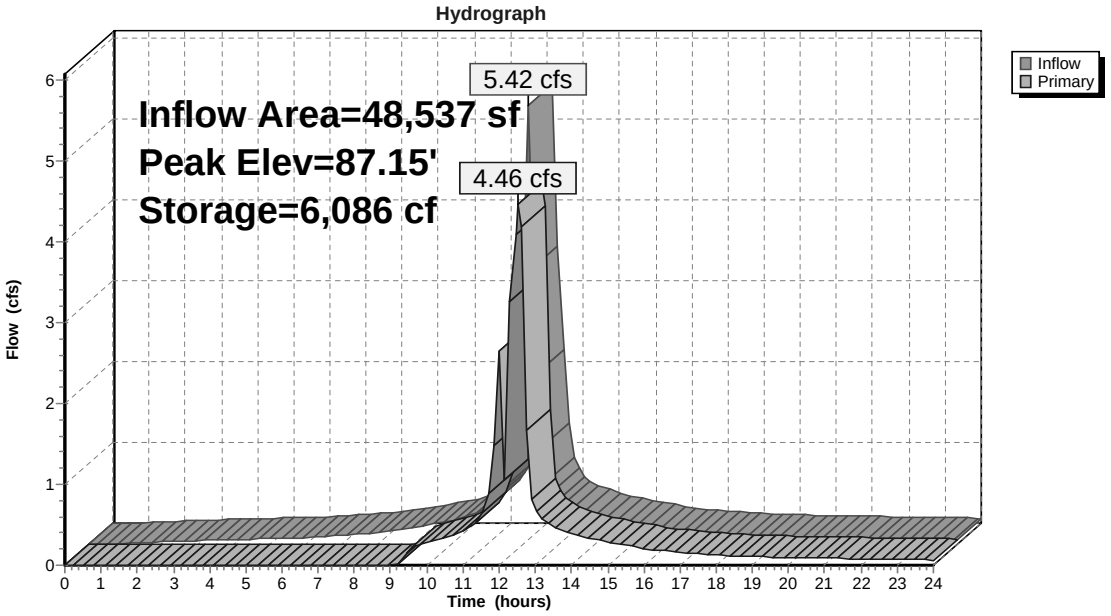
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	4,804	272.0	0	0	4,804
86.50	5,219	282.0	2,505	2,505	5,266
87.00	5,649	291.0	2,716	5,221	5,701
87.50	6,093	301.0	2,935	8,156	6,195

Device	Routing	Invert	Outlet Devices
#1	Primary	83.42'	**12.0" Round 12" HDPE** L= 55.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 83.42' / 83.14' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Device 1	86.50'	**8.0" Horiz. (5) 8" OVERFLOW X 5.00** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=4.00 cfs @ 12.53 hrs HW=86.93' TW=85.49' (Dynamic Tailwater)
↑**1=12" HDPE** (Inlet Controls 4.00 cfs @ 5.10 fps)
↑**2=(5) 8" OVERFLOW** (Passes 4.00 cfs of 5.53 cfs potential flow)

Pond P1C: BIO.RET. #1

Hydrograph



Summary for Pond P1D: BIO.RET. #2

Inflow Area = 29,104 sf, 64.43% Impervious, Inflow Depth > 5.31" for 100-Year event
Inflow = 3.04 cfs @ 12.15 hrs, Volume= 12,870 cf
Outflow = 3.37 cfs @ 12.38 hrs, Volume= 11,692 cf, Atten= 0%, Lag= 13.7 min
Primary = 3.37 cfs @ 12.38 hrs, Volume= 11,692 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 86.99' @ 12.27 hrs Surf.Area= 3,363 sf Storage= 2,610 cf
Flood Elev= 86.50' Surf.Area= 2,628 sf Storage= 1,146 cf

Plug-Flow detention time= 86.7 min calculated for 11,619 cf (90% of inflow)
Center-of-Mass det. time= 43.1 min (811.7 - 768.5)

Volume	Invert	Avail.Storage	Storage Description
#1	86.00'	1,686 cf	**BIORETENTION (Irregular)**Listed below (Recalc)
#2	86.00'	2,826 cf	**BIORETENTION (Irregular)**Listed below (Recalc)
		4,512 cf	Total Available Storage

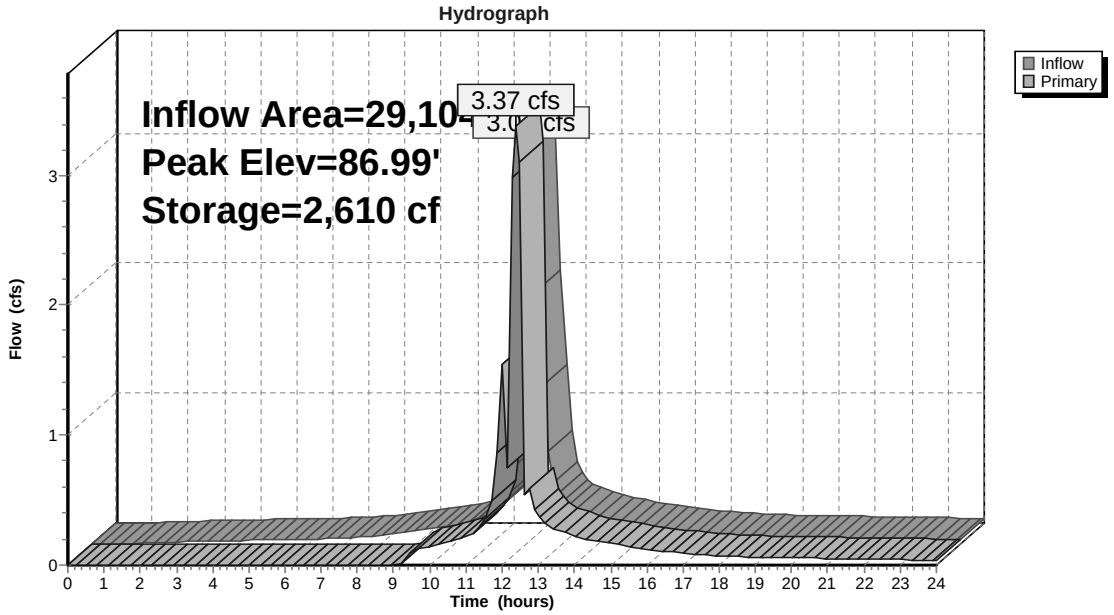
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	688	169.0	0	0	688
87.00	1,281	203.0	969	969	1,712
87.50	1,592	212.0	717	1,686	2,026

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
86.00	1,285	193.0	0	0	1,285
87.00	2,098	271.0	1,675	1,675	4,174
87.50	2,512	280.0	1,151	2,826	4,592

Device	Routing	Invert	Outlet Devices
#1	Primary	83.37'	**12.0" Round 12" HDPE** L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 83.37' / 83.14' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Device 1	86.50'	**8.0" Horiz. (4) 8" OVERFLOW X 4.00** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=3.23 cfs @ 12.38 hrs HW=86.85' TW=85.91' (Dynamic Tailwater)
└**1=12" HDPE** (Inlet Controls 3.23 cfs @ 4.11 fps)
└**2=(4) 8" OVERFLOW** (Passes 3.23 cfs of 3.96 cfs potential flow)

Pond P1D: BIO.RET. #2



Summary for Pond P2: SURFACE BASINS

Inflow Area = 143,195 sf, 51.23% Impervious, Inflow Depth > 4.96" for 100-Year event
Inflow = 14.31 cfs @ 12.15 hrs, Volume= 59,144 cf
Outflow = 8.66 cfs @ 12.36 hrs, Volume= 39,772 cf, Atten= 39%, Lag= 12.4 min
Primary = 1.96 cfs @ 12.37 hrs, Volume= 10,715 cf
Secondary = 6.69 cfs @ 12.36 hrs, Volume= 29,058 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs / 2
Peak Elev= 84.82' @ 12.36 hrs Surf.Area= 24,831 sf Storage= 26,499 cf
Flood Elev= 84.50' Surf.Area= 22,488 sf Storage= 18,822 cf

Plug-Flow detention time= 190.5 min calculated for 39,525 cf (67% of inflow)
Center-of-Mass det. time= 95.0 min (874.7 - 779.7)

Volume	Invert	Avail.Storage	Storage Description
#1	83.50'	44,950 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

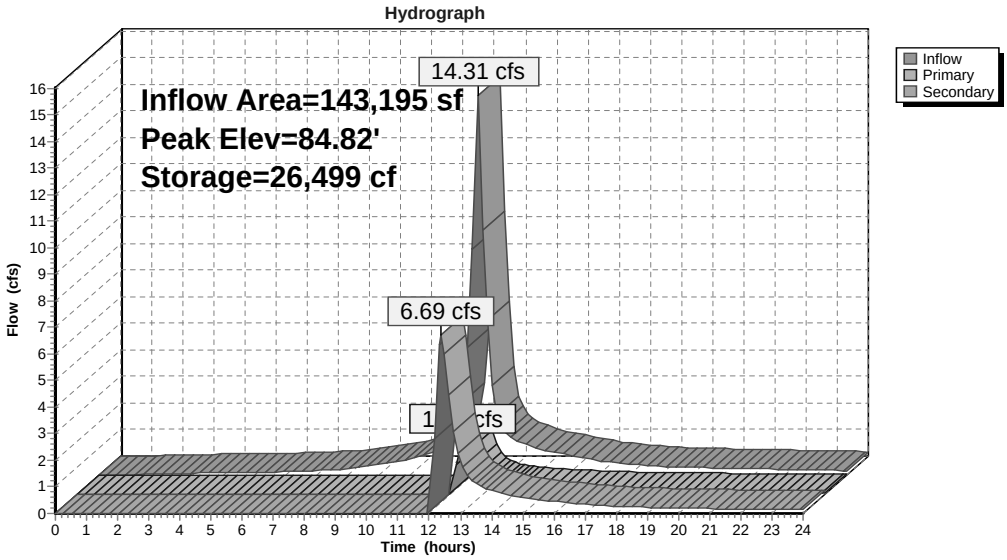
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
83.50	15,229	2,415.0	0	0	15,229
84.00	18,845	2,424.0	8,502	8,502	18,900
84.50	22,488	2,434.0	10,320	18,822	22,952
85.00	26,145	2,443.0	12,147	30,969	26,651
85.50	29,817	2,452.0	13,980	44,950	30,365

Device	Routing	Invert	Outlet Devices
#1	Primary	82.40'	**6.0" Round (2) 6" PVC X 2.00** L= 140.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 82.40' / 81.00' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf
#2	Device 1	84.50'	**8.0" Horiz. (2) 8" OVERFLOW X 2.00** C= 0.600 Limited to weir flow at low heads
#3	Secondary	84.50'	**15.0' long x 5.0' breadth RIP-RAP OVERFLOW** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=1.86 cfs @ 12.37 hrs HW=84.81' TW=0.00' (Dynamic Tailwater)
└─**1=(2) 6" PVC** (Passes 1.86 cfs of 2.02 cfs potential flow)
└─**2=(2) 8" OVERFLOW** (Orifice Controls 1.86 cfs @ 2.67 fps)

Secondary OutFlow Max=6.22 cfs @ 12.36 hrs HW=84.81' TW=82.67' (Dynamic Tailwater)
└─**3=RIP-RAP OVERFLOW** (Weir Controls 6.22 cfs @ 1.35 fps)

Pond P2: SURFACE BASINS

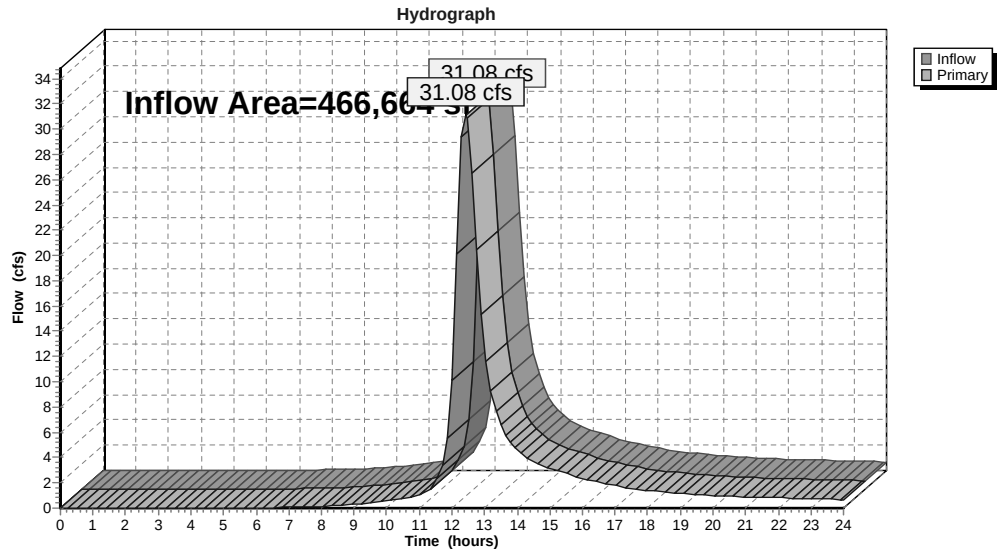


Summary for Link SP-1: STUDY POINT #1

Inflow Area = 466,664 sf, 60.79% Impervious, Inflow Depth > 4.64" for 100-Year event
Inflow = 31.08 cfs @ 12.41 hrs, Volume= 180,591 cf
Primary = 31.08 cfs @ 12.41 hrs, Volume= 180,591 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-1: STUDY POINT #1

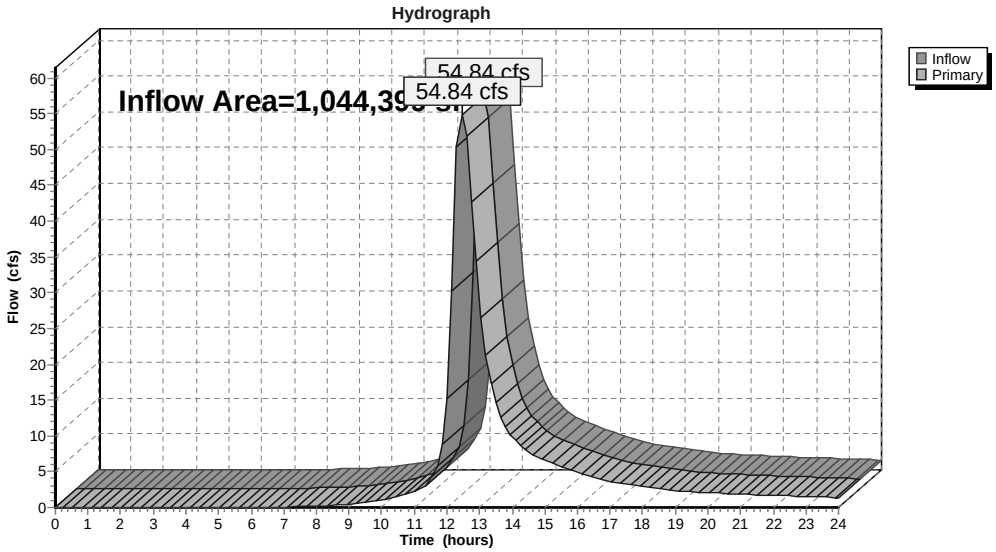


Summary for Link SP-2: STUDY POINT #2

Inflow Area = 1,044,399 sf, 36.80% Impervious, Inflow Depth > 4.08" for 100-Year event
Inflow = 54.84 cfs @ 12.47 hrs, Volume= 354,864 cf
Primary = 54.84 cfs @ 12.47 hrs, Volume= 354,864 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-2: STUDY POINT #2

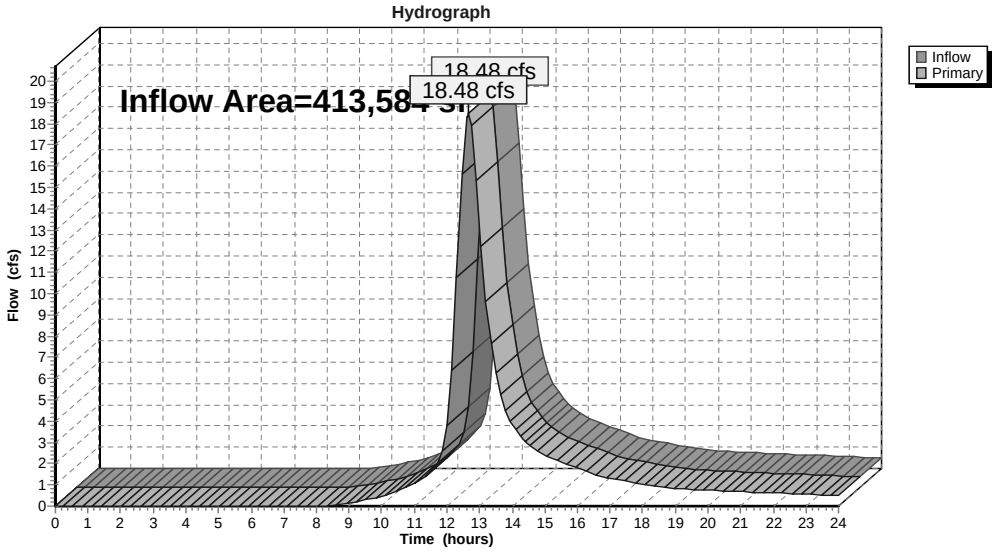


Summary for Link SP-3: STUDY POINT #3

Inflow Area = 413,584 sf, 4.62% Impervious, Inflow Depth > 3.68" for 100-Year event
Inflow = 18.48 cfs @ 12.66 hrs, Volume= 126,790 cf
Primary = 18.48 cfs @ 12.66 hrs, Volume= 126,790 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.15 hrs

Link SP-3: STUDY POINT #3



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1611-08-Proposed Conditions
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Printed 3/25/2021

SECTION 5.0 -
APPENDIX

Title	***MA DEP Standard Calculations***
Project	*200 Quannapowitt*
Location	200 Quannapowitt Pkwy Wakefield, MA
Date	March 16, 2021
Revised	March 31, 2021

Computation Sheet

By	NCD
Chk'd	TJW
Apprv'd	TJW

Stormwater Recharge/Water Quality Volume Table

$Rv = F*A_{IMP}$

$A_{WQ} = D_{WQ}*A_{IMP}$

Rv *= Required Recharge Volume, expressed in ft ^{3}, cubic yards or acre-feet*

F *= Target Depth Factor associated with each Hydraulic Soil Group*

A_{WQ} *= Required Water Quality Treatment Volume, expressed in ft ^{3}*

D_{WQ} *= Water Quality Depth*

A_{IMP} *= Impervious Area (pavement & rooftop area on site)*

Watershed	Area (Sq. Ft.)	Landscaped	Impervious Area (Square Feet)			Recharge Required			Water Quality Volume Required	
			HSG B (F=0.35)	HSG C (F=0.25)	HSG D (F=0.10)	*F Avg. (Inches)*	*Impervious Area (Feet)*	*Rv (ft ^{3})*	D_{WQ} *(Inch)*	A_{WQ}
P-1A	33,456	20,113	0	13,343	0	0.25	13,343	278	1.0	1,112
P-1B	41,622	32,719	0	8,903	0	0.25	8,903	185	1.0	742
P-1C	53,429	42,800	0	10,629	0	0.25	10,629	221	1.0	886
P-1D	33,549	21,818	0	11,731	0	0.25	11,731	244	1.0	978
P-1E	31,624	22,158	0	9,466	0	0.25	9,466	197	1.0	789
P-1F	9,036	8,156	0	880	0	0.25	880	18	1.0	73
P-1G	117,796	21,862	0	95,934	0	0.25	95,934	1,999	1.0	7,995
P-1H	11,311	10,351	0	960	0	0.25	960	20	1.0	80
P-1I	16,453	2,946	0	13,507	0	0.25	13,507	281	1.0	1,126
R-1	22,992	0	0	22,992	0	0.25	22,992	479	1.0	1,916
R-1F	39,501	0	0	39,501	0	0.25	39,501	823	1.0	3,292
R-1G	17,102	0	0	17,102	0	0.25	17,102	356	1.0	1,425
R-1H	17,793	0	0	17,793	0	0.25	17,793	371	1.0	1,483
R-1I	20,960	0	0	20,960	0	0.25	20,960	437	1.0	1,747
P-2A	20,956	12,753	0	8,203	0	0.25	8,203	171	1.0	684
P-2B	79,528	69,830	0	9,698	0	0.25	9,698	202	1.0	808
R-2A	30,813	0	0	30,813	0	0.25	30,813	642	1.0	2,568
R-2B	32,854	0	0	32,854	0	0.25	32,854	684	1.0	2,738
P-3	413,584	394,477	0	19,107	0	0.25	19,107	398	1.0	1,592
Total	**1,044,359**	**659,983**	**0**	**384,376**	**0**		**384,376**	**8,008**		**32,031**

			Computation Sheet
Title	***MA DEP Standard Calculations***		By NCD
Project	*200 Quannapowitt*		Chk'd TJW
Location	200 Quannapowitt Pkwy Wakefield, MA		Apprv'd TJW
Date	March 16, 2021		

Stormwater Recharge Summary

	Required (cf)	***Provided (cf)***	
ARv =	***2,355***	***12,169***	***Infiltration Chambers #1 (Below Outlet Inv.=84.00) [P-1G, R-1G]***
ARv =	***843***	***3,589***	***Infiltration Chambers#2 (Below Outlet Inv.=84.00) [P-1I, R-1I]***
ARv =	***841***	***2,505***	***Bioretention #1(Below Outlet Inv.=86.50) [P-1F, R-1F]***
ARv =	***391***	***1,146***	***Bioretention #2 (Below Outlet Inv.=86.50) [P-1H, R-1H]***
ARv =	***1,528***	***18,822***	***Surface Infiltration Basin (Below Outlet Inv.=84.50) [P-2B, R-2A, R-2B]***
ARv =	***5,958***	***38,231***	***Total***
Capture Area Adjustment * =	***10,993***		

Water Quality Summary

	Required (cf)	***Provided (cf)***	
A_{WQ} =	***9,420***	***12,169***	***Infiltration Chambers #1 (Below Outlet Inv.=84.00) [P-1G, R-1G]***
A_{WQ} =	***2,872***	***3,589***	***Infiltration Chambers #2 (Below Outlet Inv.=84.00) [P-1I, R-1I]***
A_{WQ} =	***3,365***	***2,505***	***Bioretention #1(Below Outlet Inv.=86.50) [P-1F, R-1F]***
A_{WQ} =	***1,563***	***1,146***	***Bioretention #2 (Below Outlet Inv.=86.50) [P-1H, R-1H]***
A_{WQ} =	***6,114***	***18,822***	***Surface Infiltration Basin (Below Outlet Inv.=84.50) [P-2B, R-2A, R-2B]***
A_{WQ} =	***23,334***	***38,231***	***Total***
Capture Area Adjustment * =	***43,971***		

***Capture Area Adjustment**	
Total Impervious Area	384,376
Site Impervious area draining to recharge facilities	280,002
Ratio	1.37
Adjusted ARv	10,993
Adjusted Awq	43,971

Computation Sheet

Title	***MA DEP Standard Calculations***	By	NCD
Project	*200 Quannapowitt*	Chk'd	TJW
Location	200 Quannapowitt Pkwy Wakefield, MA	Apprv'd	TJW
Date	March 16, 2021		

Draindown Within 72 Hours

$Time_{drawdown}$=(Rv) (1/Design Infiltration Rate in inches per hour) (Conversion for inches to feet) (1/bottom area in feet)

Infiltration Chambers #1 (HSG C - Silty Loam)	
Infiltration Rate (in/Hr)=	0.27
Bottom Area (ft^2) =	15,749
Infiltration Volume (ft^3) =	12,169
$Time_{drawdown}$ (Hours)=	34.34

Infiltration Chambers #2 (HSG C - Silty Loam)	
Infiltration Rate (in/Hr)=	0.27
Bottom Area (ft^2) =	4,284
Infiltration Volume (ft^3) =	3,589
$Time_{drawdown}$ (Hours)=	37.23

Bioretention #1 (HSG C - Silty Loam)	
Infiltration Rate (in/Hr)=	0.27
Bottom Area (ft^2) =	4,804
Infiltration Volume (ft^3) =	2,505
$Time_{drawdown}$ (Hours)=	23.18

Bioretention #2 (HSG C - Silty Loam)	
Infiltration Rate (in/Hr)=	0.27
Bottom Area (ft^2) =	1,972
Infiltration Volume (ft^3) =	1,146
$Time_{drawdown}$ (Hours)=	25.83

Surface Infiltration Basin (HSG C - Silty Loam)	
Infiltration Rate (in/Hr)=	0.27
Bottom Area (ft^2) =	15,229
Infiltration Volume (ft^3) =	18,822
$Time_{drawdown}$ (Hours)=	54.93

INSTRUCTIONS:
1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: 200 Quannapowitt Parkway Wakefield, MA

TSS Removal Calculation Worksheet

B BMP[1]	C TSS Removal Rate[1]	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Street Sweeping - 5%	**0.05**	**1.00**	**0.05**	**0.95**
Deep Sump and Hooded Catch Basin	**0.25**	**0.95**	**0.24**	**0.71**
Subsurface Infiltration Structure	**0.80**	**0.71**	**0.57**	**0.14**
	0.00	**0.14**	**0.00**	**0.14**
	0.00	**0.14**	**0.00**	**0.14**

Total TSS Removal = **86%**

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project: **200 Quannapowitt**
Prepared By: **NCD**
Date: **16-Mar-21**

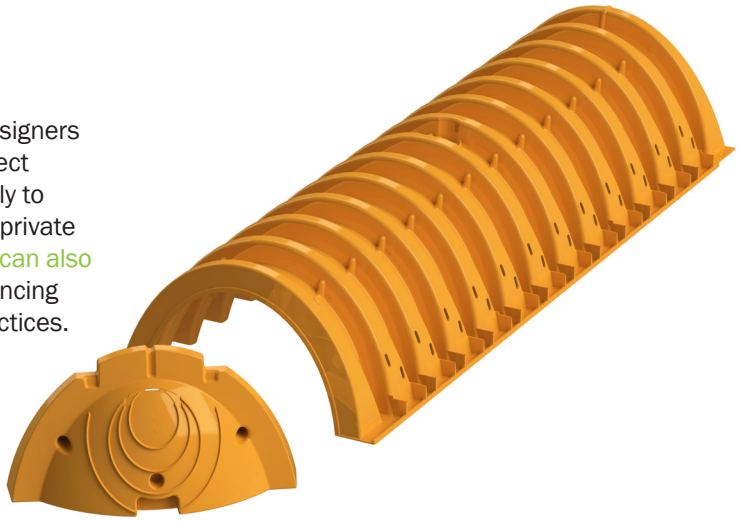
*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

SC-310 CHAMBER

Designed to meet the most stringent industry performance standards for superior structural integrity while providing designers with a cost-effective method to save valuable land and protect water resources. The StormTech system is designed primarily to be used under parking lots, thus maximizing land usage for private (commercial) and public applications. StormTech chambers can also be used in conjunction with Green Infrastructure, thus enhancing the performance and extending the service life of these practices.



STORMTECH SC-310 CHAMBER
(not to scale)

Nominal Chamber Specifications

Size (L x W x H)
85.4" x 34.0" x 16.0"
2,170 mm x 864 mm x 406 mm

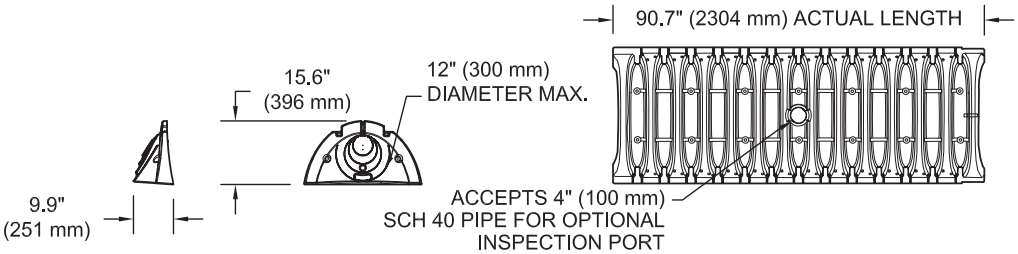
Chamber Storage
14.7 ft³ (0.42 m³)

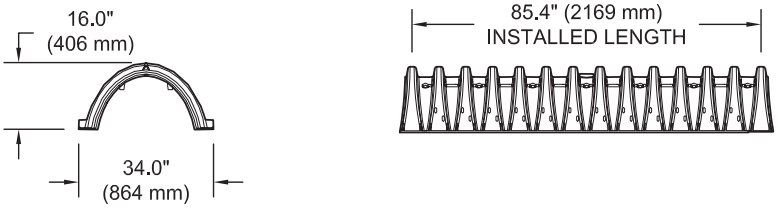
Min. Installed Storage*
31.0 ft³ (0.88 m³)

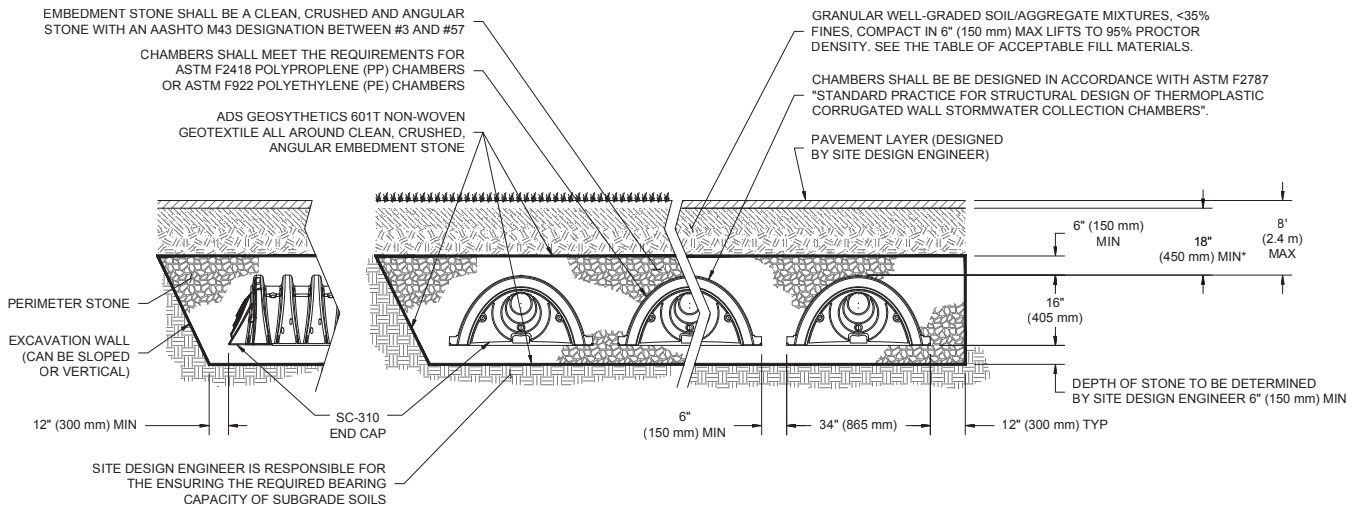
Weight
37.0 lbs (16.8 kg)

Shipping
41 chambers/pallet
108 end caps/pallet
18 pallets/truck

*Assumes 6" (150 mm) stone above and below chambers and 40% stone porosity.







*MINIMUM COVER TO BOTTOM OF FLEXIBLE PAVEMENT. FOR UNPAVED INSTALLATIONS WHERE RUTTING FROM VEHICLES MAY OCCUR, INCREASE COVER TO 24" (600 mm).

SC-310 CUMULATIVE STORAGE VOLUMES PER CHAMBER

Assumes 40% Stone Porosity. Calculations are Based Upon a 6" (150 mm) Stone Base Under Chambers.

Depth of Water in System Inches (mm)		Cumulative Chamber Storage ft³ (m³)	Total System Cumulative Storage ft³ (m³)
28 (711)		14.70 (0.416)	31.00 (0.878)
27 (686)		14.70 (0.416)	30.21 (0.855)
26 (680)	Stone	14.70 (0.416)	29.42 (0.833)
25 (610)	Cover	14.70 (0.416)	28.63 (0.811)
24 (609)		14.70 (0.416)	27.84 (0.788)
23 (584)		14.70 (0.416)	27.05 (0.766)
22 (559)		14.70 (0.416)	26.26 (0.748)
21 (533)		14.64 (0.415)	25.43 (0.720)
20 (508)		14.49 (0.410)	24.54 (0.695)
19 (483)		14.22 (0.403)	23.58 (0.668)
18 (457)		13.68 (0.387)	22.47 (0.636)
17 (432)		12.99 (0.368)	21.25 (0.602)
16 (406)		12.17 (0.345)	19.97 (0.566)
15 (381)		11.25 (0.319)	18.62 (0.528)
14 (356)		10.23 (0.290)	17.22 (0.488)
13 (330)		9.15 (0.260)	15.78 (0.447)
12 (305)		7.99 (0.227)	14.29 (0.425)
11 (279)		6.78 (0.192)	12.77 (0.362)
10 (254)		5.51 (0.156)	11.22 (0.318)
9 (229)		4.19 (0.119)	9.64 (0.278)
8 (203)		2.83 (0.081)	8.03 (0.227)
7 (178)		1.43 (0.041)	6.40 (0.181)
6 (152)		0	4.74 (0.134)
5 (127)		0	3.95 (0.112)
4(102)	Stone Foundation	0	3.16 (0.090)
3 (76)		0	2.37 (0.067)
2 (51)		0	1.58 (0.046)
1 (25)		0	0.79 (0.022)

Note: Add 0.79 ft³ (0.022 m³) of storage for each additional inch. (25 mm) of stone foundation.

STORAGE VOLUME PER CHAMBER FT³ (M³)

	Bare Chamber Storage ft³ (m³)	Chamber and Stone Foundation Depth in. (mm)		
		6 (150)	12 (300)	18 (450)
StormTech SC-310	14.7 (0.4)	31.0 (0.9)	35.7 (1.0)	40.4 (1.1)

Note: Assumes 6" (150 mm) of stone above chambers, 6" (150 mm) row spacing and 40% stone porosity.

AMOUNT OF STONE PER CHAMBER

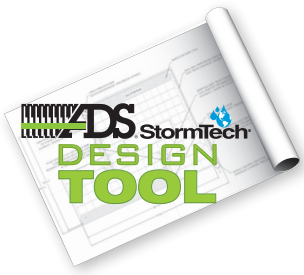
ENGLISH TONS (yds³)	Stone Foundation Depth		
	6"	12"	18"
StormTech SC-310	2.1 (1.5 yd³)	2.7 (1.9 yd³)	3.4 (2.4 yd³)
METRIC KILOGRAMS (m³)	**150 mm**	**300 mm**	**450 mm**
StormTech SC-310	1830 (1.1 m³)	2490 (1.5 m³)	2990 (1.8 m³)

Note: Assumes 6" (150 mm) of stone above, and between chambers.

VOLUME EXCAVATION PER CHAMBER YD³ (M³)

	Stone Foundation Depth		
	6" (150 mm)	12" (300 mm)	18" (450 mm)
StormTech SC-310	2.9 (2.2)	3.4 (2.6)	3.8 (2.9)

Note: Assumes 6" (150 mm) of row separation and 18" (450 mm) of cover. The volume of excavation will vary as the depth of the cover increases.



Working on a project?
Visit us at www.stormtech.com
and utilize the StormTech Design Tool

For more information on the StormTech SC-310 Chamber and other ADS products, please contact our Customer Service Representatives at 1-800-821-6710

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Division of ADS

STORMTECH ISOLATOR ROW SIZING CHART					
	SC-310	SC-740	DC-780	MC-3500	MC-4500
Chamber Area (Sq.Ft.)	20	27.8	27.8	43.2	30.1
Treated Flow Rate per chamber (CFS)	0.11	0.15	0.15	0.24	0.17

NOTE: *Testing of the Isolator Row completed by Tennesse Tech has been verified by NJCAT and it has shown to have a TSS removal efficiency of 84% for SIL-CO-SIL 250*

NJCAT verified Treated Flow Rate (GPM / Sq.Ft.) 2.5

StormTech®
Detention • Retention • Recharge
Subsurface Stormwater Management℠

Isolator Row™ Performance Test Results

as reported by Tennessee Technological University

StormTech chambers are the only chambers that meet stringent AASHTO safety factors for traffic load and deep burial applications.

Four SC-740 chambers in test apparatus at Tennessee Tech.

Uniform sediment distribution (US Silica OK-110 SG=2.65).

The Isolator Row is an innovative yet simple system that inexpensively removes total suspended solids (TSS) from storm water and provides easy access for inspection and maintenance. In the Isolator Row, StormTech chambers are completely enclosed by geotextile fabrics. Sediment is captured in the Isolator Row as storm water passes through the fabric to the stone and adjacent chambers.

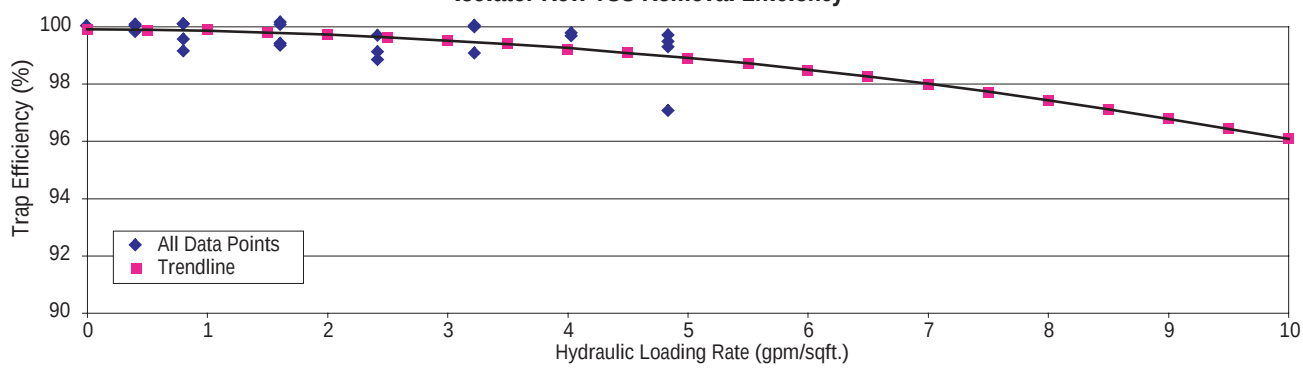
The recent completion of TSS removal testing at Tennessee Tech provides design engineers and regulators solid data that can be used to estimate the maintenance free interval and establishes the Isolator Row as a best management practice (BMP) for TSS removal.

For additional information on the Isolator Row (patent pending), contact StormTech at (888) 892-2694.

Performance Summary:

- 97% Overall TSS Removal
- 80% TSS Captured in the Isolator Row
- Estimated Maintenance Interval – 3 years

Isolator Row TSS Removal Efficiency



Another Success Story for the Isolator Row

After One Year of Operation, Harvey Industries Inspected and Cleaned Their Isolator Row in Portland, Maine

150 StormTech SC-740 chambers were installed in April, 2003. On July 7, 2004, after one year in service, StormTech inspected the Isolator Row and observed maintenance procedures.

Vactor trucks are typically equipped with both jetting and vacuum equipment.

During maintenance, the jetting nozzle propels itself down the Isolator Row scouring up sediment and washing it down to the access manhole where it is vacuumed into the truck.

After four passes of the jetting nozzle at pressures up to 1900 psi, the bottom fabric was scoured clean.

StormTech®
Detention • Retention • Recharge
Subsurface Stormwater Management℠

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860.529.8188 | 888.892.2694 | fax 866.328.8401 | www.stormtech.com

F-1. Rainfall Data for Massachusetts from *Rainfall Frequency Atlas of the United States* (TP-40)

- Users of this Handbook should note that current MA DEP written guidance (see DEP Waterlines newsletter -- Fall 2000) requires the use of TP-40 Rainfall Data for calculations under the Wetlands Protection Regulations and the Stormwater Management Policy. More stringent design storms may be used under a local bylaw or ordinance. However, DEP will continue to require the use of TP-40 in any case it reviews under the Wetlands Protection Act and Stormwater Management Policy.

Adjusted Technical Paper 40 Design Storms for 24-hour Event by County

County Name	1-yr 24-hr	2-yr 24-hr	5-yr 24-hr	10-yr 24-hr	25-yr 24-hr	50-yr 24-hr	100-yr 24-hr
Barnstable	2.5	3.6	4.5	4.8	5.7	6.4	7.1
Berkshire	2.5	2.9	3.8	4.4	5.1	5.9	6.4
Bristol	2.5	3.4	4.3	4.8	5.6	6.3	7.0
Dukes	2.5	3.6	4.6	4.9	5.8	6.5	7.2
Essex	2.5	3.1	3.9	4.5	5.4	5.9	6.5
Franklin	2.5	2.9	3.8	4.3	5.1	5.8	6.2
Hampden	2.5	3.0	4.0	4.6	5.3	6.0	6.5
Hampshire	2.5	3.0	3.9	4.5	5.2	5.9	6.4
Middlesex	2.5	3.1	4.0	4.5	5.3	5.9	6.5
Nantucket	2.5	3.6	4.6	4.9	5.8	6.5	7.2
Norfolk	2.5	3.2	4.1	4.7	5.5	6.1	6.7
Plymouth	2.5	3.4	4.3	4.7	5.6	6.2	7.0
Suffolk	2.5	3.2	4.0	4.6	5.5	6.0	6.6
Worcester	2.5	3.0	4.0	4.5	5.3	5.9	6.5

VALUES OF THE ROUGHNESS COEFFICIENT n
(Boldface figures are values generally recommended in design)

Type of channel and description	Minimum	Normal	Maximum
A. CLOSED CONDUITS FLOWING PARTLY FULL			
A-1. Metal			
a. Brass, smooth	0.009	**0.010**	0.013
b. Steel			
1. Lockbar and welded	0.010	0.012	0.014
2. Riveted and spiral	0.013	0.016	0.017
c. Cast iron			
1. Coated	0.010	0.013	0.014
2. Uncoated	0.011	0.014	0.016
d. Wrought iron			
1. Black	0.012	0.014	0.015
2. Galvanized	0.013	0.016	0.017
e. Corrugated metal			
1. Subdrain	0.017	0.019	0.021
2. Storm drain	0.021	**0.024**	0.030
A-2. Nonmetal			
a. Lucite	0.008	0.009	0.010
b. Glass	0.009	**0.010**	0.013
c. Cement			
1. Neat, surface	0.010	0.011	0.013
2. Mortar	0.011	0.013	0.015
d. Concrete			
1. Culvert, straight and free of debris	0.010	0.011	0.013
2. Culvert with bends, connections, and some debris	0.011	**0.013**	0.014
3. Finished	0.011	0.012	0.014
4. Sewer with manholes, inlet, etc., straight	0.013	0.015	0.017
5. Unfinished, steel form	0.012	0.013	0.014
6. Unfinished, smooth wood form	0.012	**0.014**	0.016
7. Unfinished, rough wood form	0.015	0.017	0.020
e. Wood			
1. Stave	0.010	0.012	0.014
2. Laminated, treated	0.015	0.017	0.020
f. Clay			
1. Common drainage tile	0.011	**0.013**	0.017
2. Vitrified sewer	0.011	0.014	0.017
3. Vitrified sewer with manholes, inlet, etc.	0.013	0.015	**0.017**
4. Vitrified subdrain with open joint	0.014	**0.016**	**0.018**
g. Brickwork			
1. Glazed	0.011	0.013	0.015
2. Lined with cement mortar	0.012	0.015	0.017
h. Sanitary sewers coated with sewage slimes, with bends and connections	0.012	0.013	0.016
i. Paved invert, sewer, smooth bottom	0.016	0.019	0.020
j. Rubble masonry, cemented	0.018	0.025	0.030

VALUES OF THE ROUGHNESS COEFFICIENT n (*continued*)

Type of channel and description	Minimum	Normal	Maximum
B. LINED OR BUILT-UP CHANNELS			
B-1. Metal			
a. Smooth steel surface			
1. Unpainted	0.011	**0.012**	0.014
2. Painted	0.012	0.013	0.017
b. Corrugated	0.021	0.025	0.030
B-2. Nonmetal			
a. Cement			
1. Neat, surface	0.010	0.011	0.013
2. Mortar	0.011	0.013	0.015
b. Wood			
1. Planed, untreated	0.010	0.012	0.014
2. Planed, creosoted	0.011	0.012	0.015
3. Unplaned	0.011	0.013	0.015
4. Plank with battens	0.012	0.015	0.018
5. Lined with roofing paper	0.010	0.014	0.017
c. Concrete			
1. Trowel finish	0.011	**0.013**	0.015
2. Float finish	0.013	0.015	0.016
3. Finished, with gravel on bottom	0.015	0.017	0.020
4. Unfinished	0.014	0.017	0.020
5. Gunite, good section	0.016	0.019	0.023
6. Gunite, wavy section	0.018	0.022	0.025
7. On good excavated rock	0.017	0.020	
8. On irregular excavated rock	0.022	0.027	
d. Concrete bottom float finished with sides of			
1. Dressed stone in mortar	0.015	0.017	0.020
2. Random stone in mortar	0.017	0.020	0.024
3. Cement rubble masonry, plastered	0.016	0.020	0.024
4. Cement rubble masonry	0.020	0.025	0.030
5. Dry rubble or riprap	0.020	0.030	0.035
e. Gravel bottom with sides of			
1. Formed concrete	0.017	0.020	0.025
2. Random stone in mortar	0.020	0.023	0.026
3. Dry rubble or riprap	0.023	0.033	0.036
f. Brick			
1. Glazed	0.011	**0.013**	0.015
2. In cement mortar	0.012	**0.015**	0.018
g. Masonry			
1. Cemented rubble	0.017	0.025	0.030
2. Dry rubble	0.023	0.032	0.035
h. Dressed ashlar	0.013	0.015	0.017
i. Asphalt			
1. Smooth	0.013	0.013	
2. Rough	0.016	0.016	
j. Vegetal lining	0.030	.....	0.500

VALUES OF THE ROUGHNESS COEFFICIENT n (*continued*)

Type of channel and description	Minimum	Normal	Maximum
C. EXCAVATED OR DREDGED			
a. Earth, straight and uniform			
1. Clean, recently completed	0.016	0.018	0.020
2. Clean, after weathering	0.018	0.022	0.025
3. Gravel, uniform section, clean	0.022	0.025	0.030
4. With short grass, few weeds	0.022	0.027	0.033
b. Earth, winding and sluggish			
1. No vegetation	0.023	0.025	0.030
2. Grass, some weeds	0.025	0.030	0.033
3. Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
4. Earth bottom and rubble sides	0.028	0.030	0.035
5. Stony bottom and weedy banks	0.025	0.035	0.040
6. Cobble bottom and clean sides	0.030	0.040	0.050
c. Dragline-excavated or dredged			
1. No vegetation	0.025	0.028	0.033
2. Light brush on banks	0.035	0.050	0.060
d. Rock cuts			
1. Smooth and uniform	0.025	0.035	0.040
2. Jagged and irregular	0.035	0.040	0.050
e. Channels not maintained, weeds and brush uncut			
1. Dense weeds, high as flow depth	0.050	0.080	0.120
2. Clean bottom, brush on sides	0.040	0.050	0.080
3. Same, highest stage of flow	0.045	0.070	0.110
4. Dense brush, high stage	0.080	0.100	0.140
D. NATURAL STREAMS			
D-1. Minor streams (top width at flood stage <100 ft)			
a. Streams on plain			
1. Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033
2. Same as above, but more stones and weeds	0.030	0.035	0.040
3. Clean, winding, some pools and shoals	0.033	0.040	0.045
4. Same as above, but some weeds and stones	0.035	0.045	0.050
5. Same as above, lower stages, more ineffective slopes and sections	0.040	0.048	0.055
6. Same as 4, but more stones	0.045	0.050	0.060
7. Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
8. Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

VALUES OF THE ROUGHNESS COEFFICIENT n (*continued*)

Type of channel and description	Minimum	Normal	Maximum
b. Mountain streams, no vegetation in channel, banks usually steep, trees and brush along banks submerged at high stages			
1. Bottom: gravels, cobbles, and few boulders	0.030	0.040	0.050
2. Bottom: cobbles with large boulders	0.040	0.050	0.070
D-2. Flood plains			
a. Pasture, no brush			
1. Short grass	0.025	0.030	0.035
2. High grass	0.030	0.035	0.050
b. Cultivated areas			
1. No crop	0.020	0.030	0.040
2. Mature row crops	0.025	0.035	0.045
3. Mature field crops	0.030	0.040	0.050
c. Brush			
1. Scattered brush, heavy weeds	0.035	0.050	0.070
2. Light brush and trees, in winter	0.035	0.050	0.060
3. Light brush and trees, in summer	0.040	0.060	0.080
4. Medium to dense brush, in winter	0.045	0.070	0.110
5. Medium to dense brush, in summer	0.070	0.100	0.160
d. Trees			
1. Dense willows, summer, straight	0.110	0.150	0.200
2. Cleared land with tree stumps, no sprouts	0.030	0.040	0.050
3. Same as above, but with heavy growth of sprouts	0.050	0.060	0.080
4. Heavy stand of timber, a few down trees, little undergrowth, flood stage below branches	0.080	0.100	0.120
5. Same as above, but with flood stage reaching branches	0.100	0.120	0.160
D-3. Major streams (top width at flood stage >100 ft). The n value is less than that for minor streams of similar description, because banks offer less effective resistance.			
a. Regular section with no boulders or brush	0.025	.....	0.060
b. Irregular and rough section	0.035	.....	0.100

Soil Map—Middlesex County, Massachusetts

Map Scale: 1:6,600 if printed on A landscape (11" x 8.5") sheet.

N

Meters
0 50 100 200 300

Feet
0 300 600 1200 1800

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features

- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Middlesex County, Massachusetts
Survey Area Data: Version 18, Sep 7, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 10, 2014—Sep 19, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
1	Water	59.1	32.5%
51A	Swansea muck, 0 to 1 percent slopes	7.5	4.1%
52A	Freetown muck, 0 to 1 percent slopes	25.9	14.3%
603	Urban land, wet substratum	1.0	0.5%
626B	Merrimac-Urban land complex, 0 to 8 percent slopes	12.7	7.0%
629C	Canton-Charlton-Urban land complex, 3 to 15 percent slopes	2.4	1.3%
652	Udorthents, refuse substratum	6.4	3.5%
656	Udorthents-Urban land complex	66.8	36.7%
Totals for Area of Interest		**181.8**	**100.0%**

National Flood Hazard Layer FIRMette

FEMA

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN — Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 / 17.5 — Cross Sections with 1% Annual Chance / Water Surface Elevation
- Coastal Transect
- 513 — Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

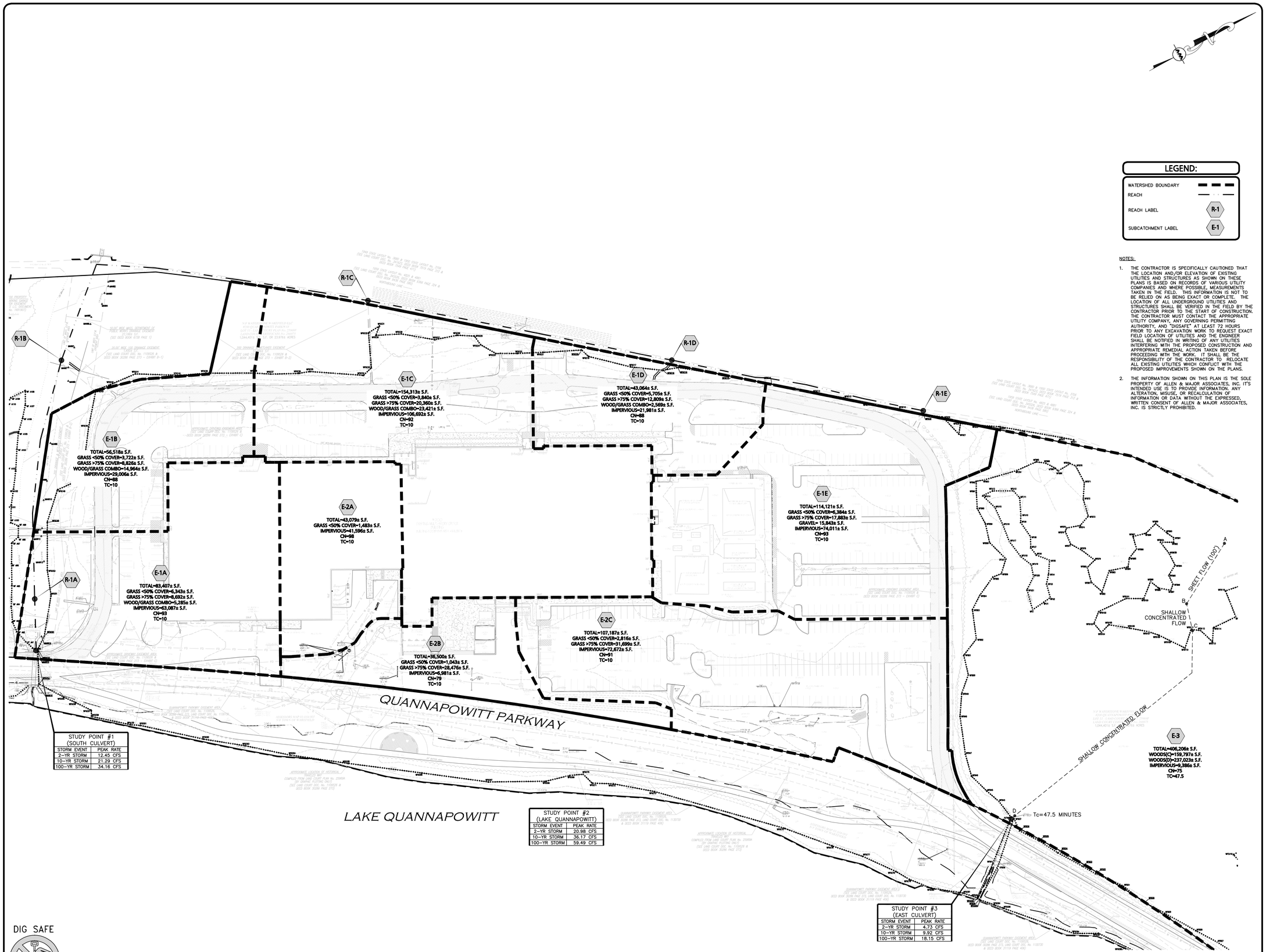
The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 10/2/2018 at 9:24:28 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

SECTION 6.0 –
WATERSHED PLANS



LEGEND:

WATERSHED BOUNDARY	
REACH	
REACH LABEL	R-1
SUBCATCHMENT LABEL	E-1

NOTES:

1. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AND STRUCTURES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF VARIOUS UTILITY COMPANIES AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THIS INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE LOCATION OF ALL UNDERGROUND UTILITIES AND STRUCTURES SHALL BE VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO THE START OF CONSTRUCTION. THE CONTRACTOR MUST CONTACT THE APPROPRIATE UTILITY COMPANY, ANY GOVERNING PERMITTING AUTHORITY, AND "DIGSAFE" AT LEAST 72 HOURS PRIOR TO ANY EXCAVATION WORK TO REQUEST EXACT FIELD LOCATION OF UTILITIES AND THE ENGINEER SHALL BE NOTIFIED IN WRITING OF ANY UTILITIES INTERFERING WITH THE PROPOSED CONSTRUCTION AND APPROPRIATE REMEDIAL ACTION TAKEN BEFORE PROCEEDING WITH THE WORK. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.
2. THE INFORMATION SHOWN ON THIS PLAN IS THE SOLE PROPERTY OF ALLEN & MAJOR ASSOCIATES, INC. IT'S INTENDED USE IS TO PROVIDE INFORMATION. ANY ALTERATION, MISUSE, OR RECALCULATION OF INFORMATION OR DATA WITHOUT THE EXPRESSED, WRITTEN CONSENT OF ALLEN & MAJOR ASSOCIATES, INC. IS STRICTLY PROHIBITED.

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1-888-344-7233

N:\PROJECTS\1623-11\CIVIL\DRAWINGS\CURRENT\C-1623-11 - EWS.DWG

RELIEF FROM THE FOLLOWING ZONING REQUIREMENTS OF THE TOWN OF WAKEFIELD ZONING ORDINANCE ARE REQUESTED:

190-32D - BUILDING HEIGHT REQUIREMENT

PROFESSIONAL ENGINEER FOR
ALLEN & MAJOR ASSOCIATES, INC.

REV	DATE	DESCRIPTION

APPLICANT:

CABOT, CABOT & FORBES
185 DARTMOUTH STREET
BOSTON, MA 02116

PROJECT:

200-400 QUANNAPOWITT PARKWAY
WAKEFIELD, MA 01880

PROJECT NO.	1623-11	DATE:	MARCH 16, 2021
SCALE:	1"=50'	DWG. NAME:	C-1623-11.dwg
DESIGNED BY:	NCD	CHECKED BY:	TJW

PREPARED BY:

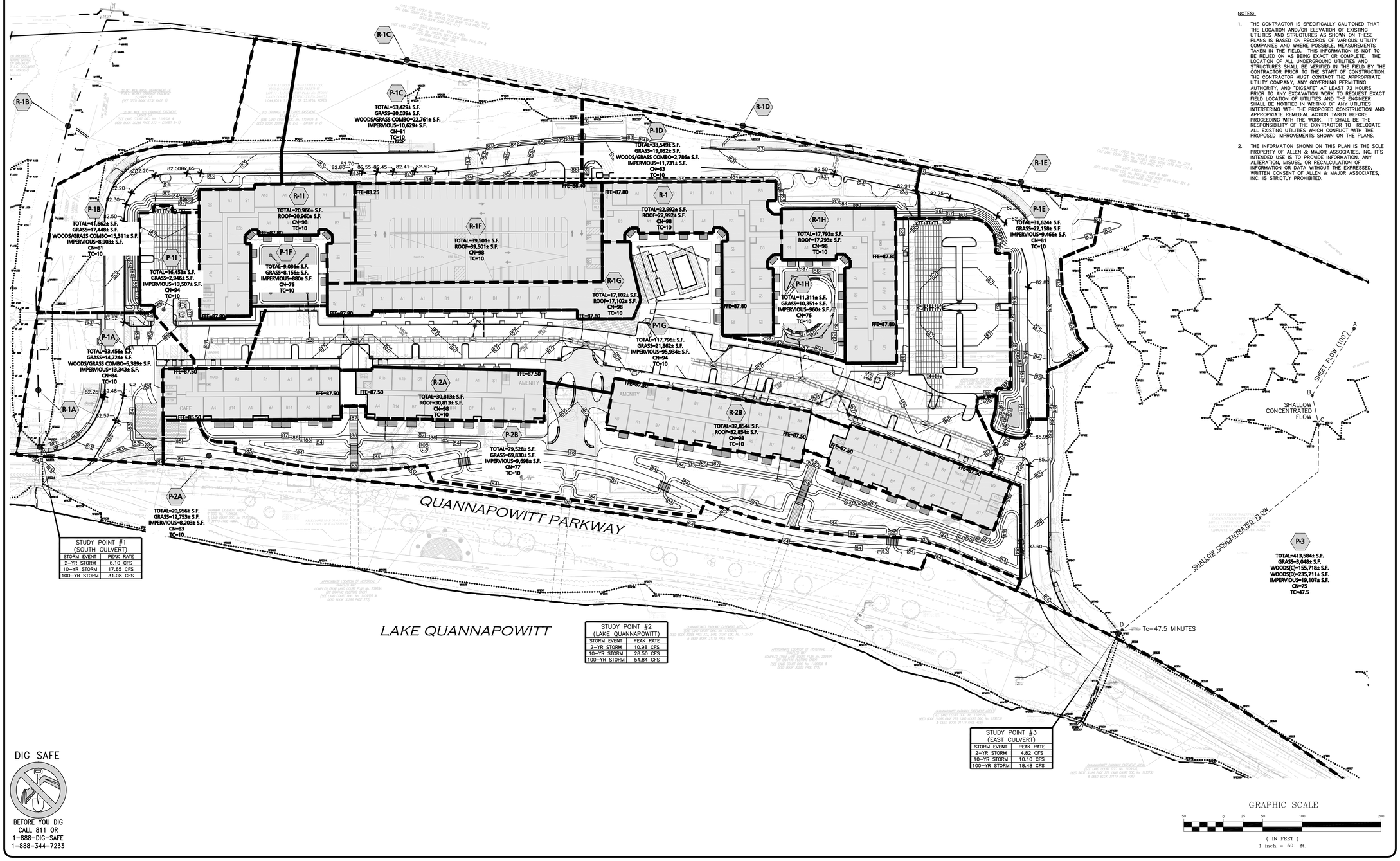
ALLEN & MAJOR ASSOCIATES, INC.
civil engineering ◆ land surveying
environmental consulting ◆ landscape architecture
www.allenmajor.com
100 COMMERCE WAY
WOBURN MA 01888-0118
TEL: (781) 935-0889
FAX: (781) 935-2896
WOBURN, MA ◆ LAKEVILLE, MA ◆ MANCHESTER, NH

DRAWING TITLE:

EXISTING WATERSHED PLAN

SHEET No.

EWS



STUDY POINT #1 (SOUTH CULVERT)	
STORM EVENT	PEAK RATE
2-YR STORM	6.10 CFS
10-YR STORM	17.65 CFS
100-YR STORM	31.08 CFS

STUDY POINT #2 (LAKE QUANNAPOWITT)	
STORM EVENT	PEAK RATE
2-YR STORM	10.98 CFS
10-YR STORM	28.50 CFS
100-YR STORM	54.84 CFS

STUDY POINT #3 (EAST CULVERT)	
STORM EVENT	PEAK RATE
2-YR STORM	4.82 CFS
10-YR STORM	10.10 CFS
100-YR STORM	18.48 CFS

LEGEND:

- WATERSHED BOUNDARY
- REACH
- REACH LABEL (R-1)
- SUBCATCHMENT LABEL (P-1)

NOTES:

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GRAPHIC SCALE

(IN FEET)
1 inch = 50 ft.

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N:\PROJECTS\1623-11\CIVIL\DRAWINGS\CURRENT\C-1623-11 - PWS.DWG

COMMONWEALTH OF MASSACHUSETTS
TIMOTHY J. WILLIAMS
CIVIL
No.43119
REGISTERED PROFESSIONAL ENGINEER

PROFESSIONAL ENGINEER FOR
ALLEN & MAJOR ASSOCIATES, INC.

REV	DATE	DESCRIPTION
1	3/31/2021	Re-Issued to ZBA

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WOBURN, MA ◆ LAKEVILLE, MA ◆ MANCHESTER, NH

DRAWING TITLE:

PROPOSED
WATERSHED PLAN

SHEET No.

PWS